

#### GEOTECHNICAL ENGINEERING INVESTIGATION REPORT

#### FOR

THE CONCORD RESORT Thompson, New York

#### PREPARED FOR:

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#### INTRODUCTION AND BACKGROUND

We have completed our engineering study of the subsurface conditions as they pertain to foundations and site grading for the proposed Concord Resort to be located at the site of the old Concord Hotel in the Town of Thompson, Sullivan County, New York. The main hotel site is bounded by Kiamesha Lake Road to the north, Concord Road to the east, a 9-hole golf course to the south and Kiamesha Lake to the west. The proposed track area is bounded by Concord Road to the west, the International golf course to the north and east and undeveloped land to the south.

Based on our review of the O.R. Mancini, Inc. report dated September 8, 1998, the original Concord Hotel was constructed in the early 1930's with numerous additions from 1950 to 1984. The hotel has recently been demolished and the foundations removed.

Based on the most recent site plans prepared for the project, the planned development will consist of: construction of a Casino with one-level of parking below; two (2) 4-story (above at-grade parking) parking structures; an indoor pool; an approximate 30-story hotel; a ballroom; a theater and event center; a harness track; a paddock; a clubhouse; a shed; retaining walls; associated utilities and at-grade paved parking and roadway areas.

Topographically, existing grades in the planned building areas vary from elevation  $1463\pm$  in the western portion of the property, to elevation  $1501\pm$  in the north corner of Parking Garage 2 and then downward to elevation  $1476\pm$  at the eastern corner of Parking Garage 1, in the eastern side of the property. There is a high point of  $1510\pm$  in Kiamesha Lake Road in the northeastern portion of the project area.

In the area of the proposed track, the site grades vary from elevation 1552± near the center of the proposed track, sloping downwards in all directions to a low elevation of 1490± in the area of the maintenance shed.

Based on the proposed grades, cuts of up to 11± feet and fills to 31± feet will be required to achieve building area subgrade elevations in the main casino and hotel area.

In the area of the proposed track, cuts of up to 53± feet and fills to 21± feet will be required to achieve subgrade elevations.

It should be noted that the exact building locations and proposed grades may still vary by a few feet, but should not affect the recommendations presented in this report.

#### FIELD AND LABORATORY INVESTIGATIONS

Our study consisted of a site reconnaissance, a review of existing soils and geologic data, and multiple field investigations consisting of test pits excavated with a rubber-tire backhoe, borings drilled using a rotary drill rig and geoprobes. The locations of the test pits, borings and geoprobes are illustrated on Figures 1 and 1B.

Our subsurface field investigations were done in September 2000, July 2002, October-November 2007, and April thru October 2008. A total of 53 borings, 44 test pits and 33 geoprobes were completed.

The fieldwork was done under the full-time observation of engineers/technicians from our firm. The individual boring logs, test pit logs and geoprobe logs, which describe the materials encountered, are presented in the attached Appendices.

Representative samples of the soils encountered in the borings and test pit excavations were obtained and brought to our laboratory for visual identification and classification testing. Classification testing consisted of water content determinations, minus No. 200 mesh sieve tests, and mechanical grain size distribution analyses. The results of the water contents and minus No. 200 sieve tests are shown on the individual boring logs. The results of the grain size analyses are shown in graphical form in the Appendix.

#### SUBSURFACE CONDITIONS

#### General

The generalized subsurface conditions consist of a miscellaneous fill varying from 1.0 to 8.0± feet thick in the planned building areas overlying dense to very dense sands and silts. Underlying the sands at depths of 2 feet to 39 feet is sandstone/shale bedrock. Rock coring was done in several of the borings. The thickness of fill and depth to bedrock at each boring and test pit location is shown on Figures 1 and 1B.

#### Casino, Event Center, Ballroom & Theater Area

Approximately 1 to 8± feet of miscellaneous fill exists in the building areas overlying medium-dense to very dense coarse to fine sand with varying amounts of silt and gravel. Occasional layers of clayey silt/silt were encountered beneath the fill and above the sand stratum in some locations.

Sandstone and shale bedrock was encountered at depths of 2.0  $\pm$  to 21  $\pm$  feet (elevation 1457 $\pm$  to 1496  $\pm$ ) below existing grade. Rock coring varying from 5

feet to 10 feet was done in 11 of the borings drilled in this area. The percent core recovery varied from 12.5 to 100 and the RQD values (rock quality designation) varied from 0 to 82 percent indicating highly variable rock quality from poor to good.

#### Hotel Tower

Beneath a thin topsoil/asphalt layer in one of the borings is 3± feet of miscellaneous mostly granular fill. The remainder of the borings did not encounter a fill layer. The soils beneath the fill/topsoil/asphalt vary from very dense coarse to fine sand to very dense silt, with varying amounts of sand and gravel.

Sandstone and shale bedrock was encountered at depths of 7.5 to 20± feet (elevation 1460± to 1475±) below existing grade. Rock coring varying from 5 to 16.5 feet was done in three of the borings drilled in this area. The percent core recovery varied from 48 to 100 and the RQD values varied from 0 to 82 percent indicating highly variable rock quality from poor to good.

#### Indoor Pool

Up to 3± feet of miscellaneous granular fill exists in this area. Beneath the fill, where encountered, are medium dense coarse to fine sands with varying amounts of gravel and silt to depths of 4± to 7± feet. Underlying the sand to the top of the rock is a dense to very dense silt with varying amounts of sand and gravel. Occasional thin layers of clayey silt were found in the silt stratum.

Sandstone and shale bedrock was encountered at depths of  $12\pm$  to  $23\pm$  feet (elevation  $1440\pm$  to  $1466\pm$ ), below existing grade. Sixteen and one half feet of rock coring was done in SB-23 with the percent core recovery of 48 to 100 and an RQD of 0 to 58 percent, indicating highly variable rock quality from poor to good.

#### Parking Garage 1

Approximately  $1\pm$  to  $5.5\pm$  feet of miscellaneous fill exists in this area. The fill varies from granular soil to solid waste type materials. Beneath the fill is a medium-dense to very dense coarse to fine sand with varying amounts of silt and gravel that extends to the top of rock

Sandstone and shale bedrock was encountered at depths of 5.5 to 21± feet (elevation 1463± to 1494) below existing grade. Five feet of rock coring was done in Boring SB-7 with the percent core recovery of 100 and an RQD of 0 percent indicating highly fractured rock.

#### Parking Garage 2

Subsurface data was obtained only in the western side of Parking Garage 2, approximately 2± to 4± feet of existing miscellaneous fill was encountered in the westernmost area of Parking Garage 2, which is underlain by dense to very dense coarse to fine sand with variable amounts of silt, gravel and occasional clayey silt.

Sandstone and shale bedrock was encountered at depths of 3.5 to 14.8± feet (elevation 1473± to 1495±) below existing grade in the westernmost building area.

Five to 10 feet of rock coring was done in B-5 and SB-22 with the percent core recovery varying from 70 to 96 and the RQD values from 0 to 48 percent indicating mostly highly fractured rock.

#### Retaining Walls/ Concord Road Relocation

The proposed construction will consist of the relocation of approximately 1,700 ± L.F. of Concord Road with approximately 320 feet adjacent to the existing dump as shown on Figure 1. The approximate length of the retaining walls in this area is 830± feet.

Borings B-1 thru B-6, B-14 and B-15 were drilled in the planned retaining wall area. A miscellaneous fill was encountered in all of the borings with the thickness varying from 2± to 19.5± feet. The greatest fill thickness is in the borings done in the old dump area. Below the fill is a mostly medium-dense to dense coarse to fine sand with varying amounts of silt and gravel. Sandstone/shale rock was encountered in 5 of the 8 borings at depths of 25± to 29± feet below existing grade. Rock coring was done in two of the borings with the percent core recovery varying from 16.7 to 90 and the RQD values from 6.7 to 35.8 percent indicating poor rock quality.

#### Dump Area

A review of the "Main Parking Dump" plan provided by JM Associates indicates the thickness of the dump to be up to 16± feet with the material consisting of a mixture of sand, silt, gravel, cans, bottles, cloth, wood, asphalt, tires, metal, carpet, refuse, concrete, etc. Several of the test pits shown on the sketch indicate that no waste was found.

Our investigation of the dump area consisted of the drilling of nine borings, three at the proposed road centerline every 100 feet, three at the right of way line every 100 feet offset from the centerline borings by 50 feet, and three borings 50 feet east of the right of way line in the middle of the dump area.

The proposed road centerline borings (B-11, B-12 and B-13) encountered a dense to very dense 2±-foot thick fill layer consisting of a brown coarse to fine sand, little silt and trace gravel. Beneath the fill, to the completion depths of the borings at 15 to 22 feet below existing grade is generally a brown coarse to fine sand, trace to some silt, trace to little gravel with occasional cobbles. Auger refusal was encountered in boring B-11 at a depth of 15 feet.

The proposed road right of way borings (B-8, B-9 and B-10) encountered a 13 to 18 foot thick fill layer consisting of a brown sand, silt and gravel with wood, ash, concrete, brick and asphalt. This fill appears to be part of the dump area. Beneath the fill, to the completion depths of the borings at 22 to 27 feet below existing grade is generally a brown coarse to fine sand, little to some silt, trace gravel with occasional cobbles and boulders.

The dump area borings (B-3, B-14 and B-15) encountered a 10 to 19± foot thick fill layer consisting of a brown sand, silt and gravel with wood, ash, concrete, brick and asphalt. Beneath the fill, to the completion depths of the borings at 17 to 29 feet below existing grade is generally a brown coarse to fine sand, little to some silt, trace gravel with occasional cobbles and boulders. Auger refusal was encountered at 29 feet in boring B-3 and two 5-foot rock cores were taken. The rock cored is gray sandstone and had core recoveries of 16.7 and 16.7 percent and RQD's of 10.4 and 6.7 percent. The low rock core recovery and RQD values indicate a highly weathered/fractured rock.

Groundwater was encountered in most of the borings in the area of the dump at depths ranging from 14 to 21.5 feet below existing grade. Some water seepage should be anticipated at higher elevations, after periods of heavy rain particularly at the soil/rock interface.

#### Track Area

The geoprobes, borings and test pits in this area did not encounter any existing fill except for OU-1B-17, which had 2.4 feet of fill at the ground surface and OU-1B-24 which had approximately 10 to 12± feet of fill associated with the existing UST's.

Beneath the topsoil/asphalt, where encountered is a natural medium-dense to very dense medium to fine sand, with varying amounts of silt and gravel and cobbles and boulders. This stratum extends to the completion depths of up to 50± feet at the geoprobes/test pit locations. Rock was encountered in TP-T5 and TP-T6 (at depths of 8± to 9± feet), which were excavated in the proposed detention pond areas and in the deep monitoring wells at depths ranging from 30 to 50± feet bgs.

#### **EVALUATION AND RECOMMENDATIONS**

#### General

Based on the borings, test pit excavations and geoprobes, this site can be considered good to excellent (except a few isolated areas) with respect to providing satisfactory support of the planned structures. No organic material other than surface vegetation, roots, and topsoil was encountered and the uncontrolled fills that were found appear to be relatively shallow, except in the old dump area and the majority of these existing fills will be excavated as part of the proposed construction.

The natural granular soils are in a mostly dense to very dense condition and will provide suitable support for conventional shallow foundations with moderate allowable bearing capacities.

Higher bearing capacities can be realized if the foundations are constructed on the underlying sandstone/shale bedrock.

The existing fills in the planned building areas may be left in place to support floor slabs and the at-grade parking areas and will provide suitable floor and pavement support after proofrolling and remediating any soft areas disclosed by the proofrolling.

#### General Site Preparation Procedures

In general, the site preparation procedures will consist of stripping the surface vegetation, topsoil, asphalt, and any organic material, if encountered, from within building and parking areas, and then cutting and filling the site to grade. Prior to placing any fill, the existing uncontrolled fill and natural soils should be proofrolled with a heavy vibratory roller. The proofrolling should consist of making 4 complete coverages of the area. Any soft areas disclosed should be excavated to stable material and backfilled in compacted lifts to achieve 95 percent of Modified Proctor Density (ASTM D 1557).

In areas where a minimum of 3 feet of fill is required to reach finished subgrade elevation, the existing asphalt/concrete need not be removed; however, it should be broken-up in place (maximum size 18" to 24") to allow for water drainage prior to the placement of fill. This is currently being done in the existing tennis courts on the west end of the project site.

The concrete in the existing tennis courts should be broken up and then be covered with 12"± to 18"± of fill and the area proofrolled, prior to placing additional fill.

The existing retaining wall located in the northwest corner of the planned future construction area may be left in place.

Following is a guide for the reuse of onsite materials:

Onsite Soils - The inorganic soils beneath the topsoil in cut areas may be used as structural fill; however, if these materials possess a high silt/clay content, they cannot be worked or compacted when significantly over optimum water content and, once wet, will required a long period of time to dry. The ease with which soil fills can be constructed on this site will, to a high degree, depend on the time of year in which construction takes place and the construction procedures utilized by the earthwork contractor. In order to reuse these soils, they may need to be spread out to let dry or treated with lime/cement to reduce the moisture content and make them workable.

It is the intent to reuse the existing site soils in compacted fills throughout the project. Acceptable uses for the onsite inorganic soils are as follows:

- Structural fill all areas
- · General Fill in landscape areas

Visual observations and density tests should be done throughout the placement of the onsite soils to determine that they are being placed in accordance with our recommendations.

Onsite Rock - Sandstone/shale may be encountered during some excavations and will likely require a large track backhoe or hoe-ram to excavate the rock below the top one to two feet. The rock may be used in structural fills as long as it is sufficiently broken down such that a dense mass without any large or significant voids is achieved. Compaction of rock fills, if any, should be accomplished by placing the rock as thinly as possible and working it into the underlying material with a large piece of equipment such as a D-8 buildozer. Rock fills should be constructed so that all large voids between the rock pieces are eliminated in the placement and working of the material.

The existing rock is currently being crushed onsite for reuse. Acceptable uses for the crushed rock are as follows:

- Backfill under footings
- · Structural fill in all areas
- · Pipe bedding and/or backfill

This material does not require density testing. After placement of the crushed rock, it should be compacted under our visual observation.

Recycled Concrete Aggregate (RCA) - We have completed geotechnical laboratory testing of the RCA samples from the stockpiles onsite. The RCA is broken into coarse grained and fine-grained RCA. The results of the laboratory testing are attached. We have also approved the use of recycled concrete that has a maximum particle size of 12± inches.

The existing RCA is currently being crushed onsite for reuse. Acceptable uses for the RCA are as follows:

- · Backfill under footings
- · Structural fill in all areas
- Pipe bedding and/or backfill
- Roadbase (onsite roads only)
- Slab-on-grade subbase

Visual observations and density tests should be done throughout the placement of the RCA to determine that it is being placed in accordance with our recommendations.

#### Fill Procedures:

Where structural fill is being placed in steep slope areas, benching into the existing slope should be done.

The fill should be placed in maximum 12-inch thick lifts, with each layer compacted to the required density using a large vibratory roller (minimum 10-ton static drum weight). Building area fills should be compacted to a minimum of 92 percent and average of 95 percent of the maximum Modified Proctor Density (ASTM D 1557).

Offsite borrow material, if required, should be well graded granular material and have a maximum particle size of 6 inches and the maximum amount of fines (percentage passing a No. 200 mesh sieve) should be 15% to help facilitate construction during wet weather. The "fines" should be non-plastic. The granular fill should be compacted using a large vibratory roller (Dynapac CA-15 or equivalent) to achieve the same density requirements as above.

Backfill in confined areas such as utility trenches and foundations within load bearing or paved areas should be placed in maximum 6-inch thick layers and compacted to a minimum of 92 percent and average of 95 percent density as described above.

As previously indicated, some of the onsite soils may contain significant percentages of silt and some clay and will readily soften during wet weather and from construction activity. Wetting or drying of the fill material should be accomplished as necessary to achieve the required density. The subgrade

should be graded to drain and tight-rolled at the end of the day, if wet weather is anticipated.

Rock fill should be placed in maximum 12-inch thick lifts, with each layer compacted to the required density using a large vibratory roller (minimum 10-ton static drum weight).

#### Earthwork Construction During Cold Weather Conditions

We are providing recommendations for earthwork construction during the upcoming cold weather conditions that will exist at the above referenced site during the winter months. The primary concern in dealing with frozen soils is the volume change (expansion) that occurs upon freezing and the subsequent softening and consolidation of the soils upon thawing.

In order to have a significant amount of expansion of soils during freezing conditions, it requires that the soils have more than 5 to 8± percent "fines" (material passing a No. 200 mesh sieve) and a source of water such as groundwater, wet weather or a high natural water content. The largest soil expansion during freezing occurs when there are sufficient "fines" and shallow groundwater that rises as a result of capillary action, which creates ice lenses and thus large volume changes from the freezing and thawing cycle. Natural granular soils and granular compacted fills that contain less than 5 to 8± percent "fines" do not experience a significant expansion upon freezing.

The two major types of material that will be used in construction of the compacted fills at this site will be the recycled crushed concrete and the natural silty granular soils. The recycled concrete should be relatively non-frost susceptible while the natural soil will be susceptible to significant volume changes due to freeze/thaw cycles. The following are our recommendations for earthwork construction during freezing weather conditions using these materials:

Recycled Crushed Concrete - This material should not be frost susceptible and can therefore be taken from the stockpiles, spread in 12± inch thick lifts and compacted to 95 percent of Modified Proctor Density (ASTM D 1557). There may be some clumping of particles during cold periods but these will readily break down during placement and compaction.

We also recommend that any areas to receive fill after a night of below freezing temperatures be "tracked" with a dozer prior to placing the initial lift of fill for the day.

Natural Silty Granular Soils - The natural site soils are frost susceptible and will likely experience a significant amount of frost "heave" during periods of

cold weather. The following procedures are recommended to minimize the effects of the cold weather on the construction activity:

- If there is more than 2 to 3 inches of frost in the borrow area, the frost should be stripped prior to loading the fill for use in the compacted fill construction. The stripped frozen soils can either be stockpiled for later use after thawing or spread in the deeper parking area fills (greater than 10 feet below finished grade) in a maximum layer of 6 inches. The frozen soil can also be used in planned landscape areas.
- Fill areas that contain 2 inches of frost or less need not have the frost removed prior to placing additional fill; however, the area should be "tracked" with the dozer prior to placing additional fill. If more than 2 inches of frost is present, the frozen soil should be stripped, prior to placing additional fill. The stripped frozen soils can be used as indicated previously.
- In order to reduce the amount of frost that will have to be stripped, we recommend that smaller areas of fill be done at a time (i.e., several lifts of fill in a small area each day rather than one fill lift over a large area). Additionally, if extremely cold weather is anticipated (particularly over a weekend) a layer of salt hay or mulch could be spread in the compacted fill and cut areas and then stripped prior to cutting/filling. Other alternates would include the use of insulation blankets placed on the subgrade to limit the amount of frost that develops or heated blankets/heating systems that can thaw the ground.

#### **PAVEMENT AREAS**

Prior to placing additional fill, proposed paved areas should be stripped of topsoil/asphalt and proofrolled as discussed earlier in this report.

The compaction criteria for fills in parking areas may consist of 92 percent, except in the uppermost 2 feet where 95 percent should be achieved to provide for good pavement support. Visual observations and in-place field density tests should be made to determine the adequacy of the compaction.

In on-grade parking areas where a minimum of 3 feet of fill is required to reach finished subgrade elevation, the topsoil/asphalt need not be excavated. We should inspect the subgrade prior to paving, to confirm that the estimated CBR and design pavement sections are appropriate. The site soils will have a moderate CBR value (California Bearing Ratio) on the order of 10 to 20 percent.

#### **GENERAL FOUNDATION DESIGN CRITERIA**

All footings may be placed on compacted structural fill, the dense natural granular soils and/or on the sandstone/shale bedrock. Footings founded on the compacted fill or natural granular soils may be designed for a maximum net allowable average bearing pressure of 4.5 tsf (9,000 psf). The allowable net bearing pressure can be increased to 10.0 tsf (20,000 psf) if the footings are founded on the sound sandstone/shale bedrock.

If the bottom of footing excavations becomes softened as the result of construction activity or wet weather, the soft material should be excavated and replaced with clean ¾ inch crushed stone.

All excavations greater than 5 feet in depth should have the sides sloped back to a maximum slope of 1 horizontal to 1 vertical and/or be sheeted and braced in accordance with applicable codes.

Exterior footings and those footings potentially exposed to frost action should be founded a minimum of 4.0 feet below adjacent exterior grade. Interior footings within heated areas may be founded at conventional depths below the floor slab. Footings founded on sound rock are not required to extend 4.0 feet below grade for frost protection.

The floor slab should be designed using a subgrade modulus of 175 pci, assuming that a 6-inch thick layer of granular material with a maximum particle size of 1.5 inches and a maximum percent passing the No. 200 mesh sieve of 12 percent is placed beneath the floor slab.

All retaining walls should be provided with positive drainage behind the wall to preclude hydrostatic pressures from developing.

The site soils have been classified as Site Class B and C, as indicated elsewhere in this report, for seismic design purposes in accordance with the Building Code of NY State, 2007. See the attached Table 1 for a summary of soil design parameters. Site soils classified as Site Class B will be rock or where there is less than 10 feet of soil cover over rock. Site soils classified as Site Class C will be areas that have greater than 10 feet of soil cover over rock.

After satisfactory completion of the outlined building area preparation procedures, footings and floor slabs founded on the compacted structural fill/natural soils/rock should have post-construction total settlements of less than 1-inch and maximum differential settlements in 50 feet of less than ½ inch.

#### AREA SPECIFIC FOUNDATION RECOMMENDATIONS

#### Casino, Event Center, Ballroom & Theater Areas

The proposed finished floor grade in these areas is elevation 1495.0. Existing grades vary from a low of 1463 in the northwest portion of the building area (proposed Ballroom), to elevation 1500± in the south portion of the planned Casino area. Based on the finished floor elevation of 1495, cuts of up to 6.0± feet and fills of up to 31.0± feet will be required to achieve finished subgrade elevation.

The required cuts and fills and depth to rock are variable over the building area, but it appears that the majority of the proposed footings will be founded on compacted structural fill or the natural soils and may be designed for a net allowable average soil bearing pressure of 4.5 tsf (9,000 psf). Footing excavations in the higher-elevation areas may extend close to or into the sandstone/shale bedrock. Footings that are founded on sound bedrock may be designed for a maximum net allowable bearing pressure of 10.0 tsf (20,000 psf).

The seismic design Site Class for this area varies from B in the eastern half of the planned Casino area to C in the remainder of the building area.

In areas that require fill, the existing fill should be proofrolled, any soft areas removed and a controlled compacted structural fill placed to subgrade elevation.

#### Hotel Tower

The proposed finished floor grade in the hotel tower is elevation 1495.0. Existing grades vary from elevation 1470 at the western end of the building area to elevation 1486 at the eastern side of the planned hotel tower. Based on the finished floor elevation of 1495, fills of 8± to 24± feet will be required to achieve finished subgrade elevation.

The significant depth of fill and depth to bedrock makes founding the footings on rock uneconomical. Therefore, either spread footings or a mat foundation constructed on a compacted structural fill consisting of recycled crushed concrete, are the recommended foundation support systems. The spread footings/mat may be designed for a maximum average net allowable soil bearing pressure of 4.5 tsf (9,000 psf).

Any existing fill in the Hotel Tower area should be excavated to dense natural soils, prior to placing any compacted structural fill,

The seismic design Site Class for the Hotel Tower is C.

#### Future Hotel Tower

Existing grades in the future hotel area vary from elevation 1463± to elevation 1482 and with an assumed finished grade of elevation 1495 will require 12± to 31± feet of fill to attain finished subfloor grade. The recommendations for the presently planned hotel are appropriate for the future hotel.

Because of the close proximity to adjacent planned structures, we recommend that the building area fills be placed as part of the present phase of construction.

#### Indoor Pool

The proposed finished grade in the indoor pool area is 1495. Existing grades vary from elevation 1463± on the north side to elevation 1478± on the south side of the planned pool area.

Based on the finished floor elevation of 1495, fills of 16± to 31± feet will be required to attain finished subgrade elevation.

We recommend that the pool area be founded on conventional spread/strip footings on a compacted structural fill. The footings may be designed for a maximum net allowable soil bearing pressure of 4.5 tsf (9,000 psf).

The seismic design Site Class for the pool area is C.

#### Parking Garage 1

The proposed lowest floor grade in this building area is elevation 1488.0. Existing grades vary from elevation 1476± in the eastern building corner to elevation 1500± in the northwest building corner.

Based on the finished lowest level floor elevation of 1488, cuts of up to 13± feet and fills of up to 11± feet will be required to attain finished subgrade elevation.

The area of deeper cuts in the northern portion of the Parking Garage 1 will encounter rock above finished grade; therefore, a portion of this structure will be founded on rock and a portion on natural soils/compacted structural fill.

We recommend that Parking Garage 1 be founded on spread/strip footings founded on either rock or soil and be designed for a net allowable bearing pressure of 10.0 tsf (20,000 psf) on rock and 4.5 tsf (9,000 psf) on the natural soils/compacted structural fill.

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#### Parking Garage 2

The proposed finished lowest floor grade in this building area is elevation 1499.5. Existing grades vary from elevation 1488± at the southeast corner to elevation 1501± at the northeast corner of the building.

Based on the finished lowest level floor elevation of 1499.5, cuts of up to 2.5± feet and fills of up to 10.5± feet will be required to reach finished subgrade elevation.

We recommend that Parking Garage 2 be founded on spread/strip footings founded on the natural soils/compacted structural fill. The footings may be designed for a maximum average net allowable soil bearing pressure of 4.5 tsf (9,000 psf). Any existing fill should be excavated and/or proofrolled, prior to placing the compacted structural fill.

The seismic design Site Class for Parking Garage 2 is C.

#### Racetrack Support Buildings

1

The proposed Clubhouse will be in a deep cut area and therefore suitable soils will be encountered at footing subgrade elevation. The planned Paddock and Maintenance Shed will be located in primarily a fill area.

We recommend that the above structures be founded on spread/strip footings founded on the natural soils/compacted structural fill and designed for a maximum net allowable soil bearing of 4.5 tsf (9,000 psf).

The seismic design Site Class for this area is C.

#### **RETAINING WALLS/ ROADWAY RELOCATION**

Based on the borings and historical information we have reviewed, the subsurface conditions in the proposed roadway relocation area can be considered fair to good with respect to providing satisfactory support of the planned roadway and retaining walls. The existing fill encountered in the dump area does not contain a significant amount of organic material and has been in place for a long period of time, resulting in a mostly medium-dense condition.

Site preparation procedures in the proposed roadway and Retaining Wall area should consist of stripping any surface vegetation, topsoil and miscellaneous surficial debris from within the roadway and retaining wall areas. Prior to placing any new fill, the existing uncontrolled fill should be proofrolled with a heavy vibratory roller such as an Ingersoll-Rand SP-100 or equivalent. The

#### **TABLE I**

#### **SUMMARY OF SOIL DESIGN PARAMETERS**

	PARAMETER	VALUE
1.	Allowable Average Bearing Capacity (net) Natural Granular Soils/Compacted Fill Sandstone/Shale Bedrock	4.5 tsf 10.0 tsf
2.	Total Unit Weight	125 pcf
3.	Angle of Internal Friction - Backfill against Structures	32 degrees
4.	Earth Pressure Coefficient (See Note 1) Active Earth Pressure (Ka) Earth Pressure @ Rest (Ko) Passive Earth Pressure (Kp)	0.32 0.50 3.26
5.	Coefficient of Sliding (concrete over soil)	0.40
6.	Subgrade Modulus for Floor Slab Design (Granular Fill)	175 pci
7.	Slopes (Above groundwater and maximum height of 15 ft) Maximum Cut Slope in Soil Maximum Fill Slope in Soil	2.0 H:IV 2.0 H:IV
8. Notes	Seismic Design Criteria- Site Class (see report for specific areas) s:	B or C

- 1.) A drainage medium should be installed along all retaining walls to avoid hydrostatic pressures from developing.
- 2.) Compaction equipment used within 5± feet of permanent walls should not weight more than 5,000 pounds.

### **APPENDIX**



# BORING LOGS AND TEST PIT LOGS FROM SUBSURFACE INVESTIGATION 10/13/00

PROJECT	NO.	N- 5485	INSPECTE	DBY:	WSP	BORING I	۷o. [	D-T
LOCATION	l se	E FIGURE 1	APPROX. E	ELEV.	98.8'	DATE	9/15/00	-
D S A P M P L E S T	* SAMPL!NG	D E P T H			DESCRIPTIO	N		
	20		Dark Brown coarse t	to fine SANE	), little Silt, trac	e Gravel		٠.
5—	26	·	Brown coarse to fine	e SAND, son	ne Silt, little Gra	wel with Rock	c Fragments	
10	55/2"		Gray and brown dec	composed SA	NDSTONE		W.C. = 6 (-200) =	
			Gray Weathered SA	NDSTONE		<del>.</del>		
15	50/1" 5 min. 4 min. 1 min. 1 min. 4 min.		Rock Core: Run 1 (; Recovery = 40"/60"; RQD = 5"/60" = 12"	= 66.7%				
25 —	- F.		BORING COMPLE	ETB AT 20 F	EET	٠.		
30 —								
			PLIT BARREL DROP * Blows/Ft,		TO WATER: KS: AT COMI	10.5±		9/15/00

PROJECT NO.	ROJECT NO. N- 5485 IN		DBY:	WSP	BORING N	10.	B-2
LOCATION	SEE FIGURE 1	APPROX, E	LEV.	98.1'	DATE	9/15/00	
D S R R S E P M A S I P P T A N C G E	D E P T H			DESCRIPTION	DN		
0-		Asphalt 2.5" Brown coarse to fine	SAND, li	ttle to some Grav	el, little Silt	· · · · · · · · · · · · · · · · · · ·	
5		Augered 6" into Gra BORING COMPLET	y Sandsto IE AT 2 F	ne EET			
	HNCH O.D. SPLI MER 30 INCH DR			H TO WATER: .RKS: AT COM	Dry PLETION OF	DATE: BORING	9/15/00

3

PROJECT NO.	N- 5485	INSPECTED BY:	WSP	BORING N	ю.	В-3
LOCATION	SEE FIGURE 1	APPROX. ELEV.	98.3'	DATE	9/15/00	
D S + R S E P M A S T P M I P S T L A N C G E	E P T H		DESCRIPTIO	М		
0		Asphalt 2.5" Brown coarse to fine SAND, little	e to some Grave	l, little clayey s	Silt	
5—		Brown clayey SILT and coarse to	o fine Sand, littl	e Gravel	W.C. = 1 (-200) = 4	4.7% 6.5%
10 - 65/1		Weathered SANDSTONE  Split Spoon Refusal at 10.1 Feet BORING COMPLETE AT 10.1	FEET			•
15—						
20 —						
30						
35—					٠	
	-INCH O.D. SPLIT MER 30 INCH DRO	1	TO WATER: KS: AT COMP	Dry LETION OF B	DATE: ORING	9/15/00

PROJI	ROJECT NO.		N- 5485	INSPECTE	DBY:	WSP	BORING !	<b>VO.</b>	B-4					
LOCA	TIOIT	N s	EE FIGURE 1	APPROX. E	ELEV.	99.3'	DATE	9/15/00						
D E P T H F T	S A M P L E S	* SAISTANCE	D E P T H			DESCRIPTIC	PN .		·					
0		12		Asphalt 2.5" Probable Fill: Brown	Probable Fill: Brown coarse to fine SAND, some Gravel, little Silt									
				Weathered and Frac	tured SAND	STONE	• • • • • • • • • • • • • • • • • • • •							
10-		65/2"		Split Spoon Refusal BORING COMPLE		EET	-							
15-														
20 —														
35 —	SAMI	PLER: 2-	INCH O.D. SF	PLIT BARREL DROP * Blows/Ft.		TO WATER:	Dry		9/15/00					

PROJE	PROJECT NO. N- 5485 INSPECT				D BY:	WSP	BORING NO		B-5	
LOCAT	ΓΙΟΝ	SEE FIG	URE 1	APPROX.	ELEV.	99.0'	DATE	9/15/00	_	
l E.	A S M A P M	R D E S P T T A N F E T		,	•	DESCRIPTIO	'n			
0-	1			Asphalt 2.5" FILL: Brown Sand,	Silt, Gravel, t	тасе Clay		· · · · · · · · · · · · · · · · · · ·		
				Brown clayey SILT and coarse to fine Sand, little Gravel						
15—				Weathered SANDS: Auger Refusal at 4.6 BORING COMPLE	6 Feet	EET				
35 —										
SAMPLER: 2-INCH O.D. SPLIT BARREL DEPTH TO WATER: DIY DAT 140 LB. HAMMER 30 INCH DROP * Blows/Ft. REMARKS: AT COMPLETION OF BORING									9/15/00	

PROJECT NO.	N- 5485	INSPECTED BY:	WSP	BORING	NO.	B-6
LOCATION	SEE FIGURE 1	APPROX. ELEV.	79.0'	DATE	9/15/00	
S * REP M ASIPE P M P T P P L L I N C G E	D E P T H		DESCRIPTIO	ЭМ		,
50		FILL: Brown Sand, Silt, Gravel, O	Cobble and Occ	casional Const	ruction Debris	
37		Gray and Brown coarse to fine SA	AND, some Gra	avel, little clay	ey Silt	
10— 88/7"		Weathered SANDSTONE  Split Spoon Refusal at 10.7 Feet BORING COMPLETE AT 10.7 I	FEET			
20 —						
30 —			·		·	
35—						•
	INCH O.D. SPLITIER 30 INCH DR		O WATER:		DATE: BORING BENGINEERS	9/15/00

PROJ	ECT NO. N- 5485 INSPECTED BY WSP	TEST PIT NO	TP-1	1
LOCA	ATION See Figure 1 APPROX. ELEV. 99.0+			
WATE	R OBSERVATION NOT ENCOUNTERED	DATE EXCAV	ATED 9/15/00	_
DEPTH FT.	DESCRIPTION / SOIL CLASSIFICATION	RE	LATIVE DENSITY O	)R
0	Asphalt 2.5"- 3.0"		······································	-
1	<b>1</b>			
'	Brown coarse to fine SAND, some Gravel, little Silt			
2			•	
	Rock at 2.0 to 3.0 Feet			
з	TEST PIT COMPLETE AT 2.0 TO 3.0 FEET			
-				
4	•			
5				
6				
7		-		
8				
9				
	•			
10—	·		•	
11				
' '			•	
12				
13	•			
<u></u>				
14				
NOTE:	SES	CONSULTING	ENGINEERS, F	$\overline{z}$

NOTE:

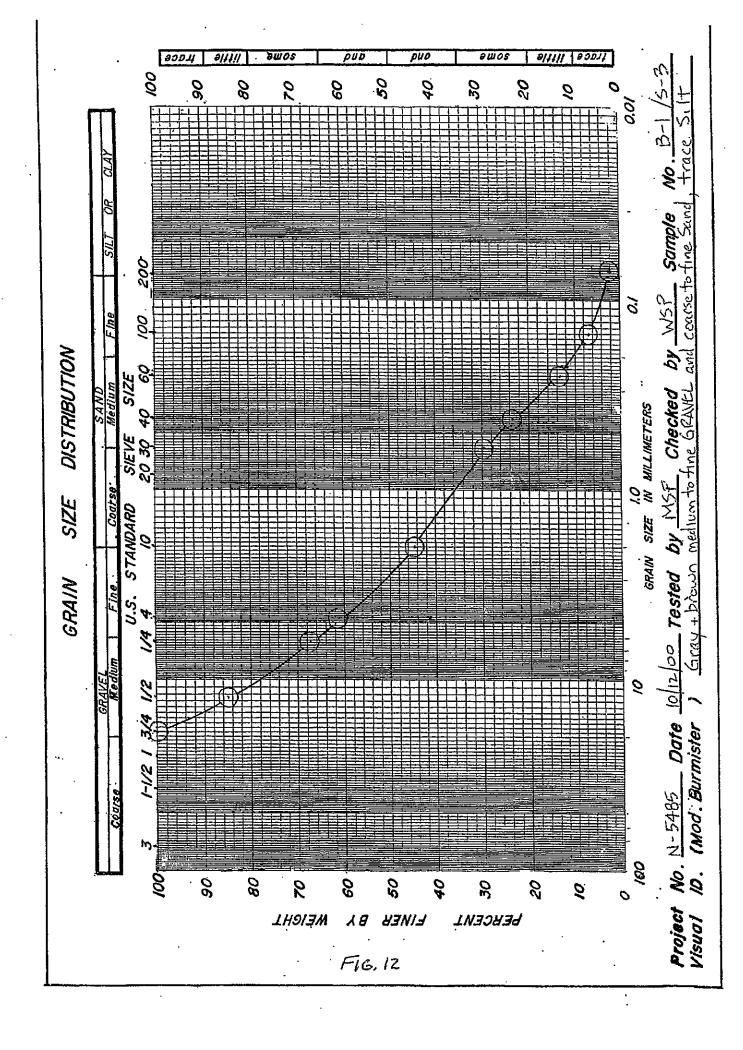
PROJECT NO. N- 5485		N- 5485	INSPECTED BY	WSP	TEST PIT	NO.	TP-2
LOCA	NOITA	See Figure 1	APPROX. ELEV.	89,0	_		
WATE	ER OBSEF	RVATION	NOT ENCOUNTERED		DATE EX	CAVATED	9/15/00
DEPTH FT.		DES	CRIPTION / SOIL CLASSIFIC	CATION		RELATIVI CON:	E DENSITY OR
0	Aspl	nait 2.5"		· · · · · · · · · · · · · · · · · · ·			
1	ा गर्न	: Brown coars					
	l" la						
2	·						
_				<b> </b> 			
3	Gray	- brown coarse	1	lium Dense			
4			to	Dense			
				·			:
5—							:
6			•				
7							
_							
8							£
9							
,							
10					· 	 	
	TES	T PIT COMPL	ETE AT 9.83 FEET		•		
11							
12							
12		_					
13		·					
14							
NOTE:				SES	SI CONSULT	TING ENG	INEERS, PC

NOTE:

PROJECT NO. N- 5485		N- 5485	INSPECTED BY WSP		TEST PIT NO.		TP-3
LOC	ATION	See Figure 1	APPROX. ELEV.	93.0	•		
WAT	ER OBSEF	RVATION	NOT ENCOUNTERED		DATE EX	CAVATED	9/15/00
DEPTH FT.		DES	CRIPTION / SOIL CLASSIFIC	ATION			E DENSITY OR
o—	TOP	SOIL 8"			· · · · ·		
1	Prob	able FILL: Gra	ay- brown coarse to fine SAN	D, little Silt, (	race Gravel		
2		7.		***************************************			
3	Grav	- brown coarse	to fine SAND, some Gravel,	linda Gila			
		- DIGMI COMP			ium Dense Dense		
4		,				ıc	Dense
		•					
5							
_							
6							
7—							
8_		•					:
	<b>1</b> 1 1				•		
9	G	ray SANDSTO	ONE				
10	TES'	T PIT COMPL	ETE AT 9.33 FEET				
11							٠,
12			•				
· · · · ·							
13							
	ļ.		•				•
14		-					
NOTE:				SE	SI CONSULT	ING EN	INEERS, PC

NOTE:

PROJ	PROJECT NO. N- 5485		INSPECTED BY WSP 1		TEST PIT NO.	TP-4
LOCA	ATION	See Figure 1	APPROX. ELEV.	78.0		
WATE	R OBSER	RVATION	NOT ENCOUNTERED		DATE EXCAVATE	D 9/15/00
DEPTH FT.		DES	CRIPTION / SOIL CLASSIFIC	PATION		VE DENSITY OR NSISTENCY
0—	· -	-				
1		.: Gray- brown	t			
	with	occasional Co	bbles- boulders, concrete, met	al, brick		
3			•			
4			•		-	
5		·				
_						
6	Gray	- brown coars	e to fine SAND, some Gravel,	little Silt	M	edium Dense
7		Sansa Cl A NTTS COTTO	ONTE	•		to Dense
		Gray SANDST	ONE			
8						
	TES'	T PIT COMPI	LETE AT 7.5 FEET			
9.—						
10					·	
71						
12	•				1	
5						
13					}	
14						
NOTE:				SES	CONSULTING EN	IGINEERS PC





## BORING LOGS FROM SUBSURFACE INVESTIGATION 2/10/2003

PROJECT NO. N-5485			N-5485	INSPECTED BY: MM BORING NO.					
LOCA	TIO	N	SEE FIGURE 1	APPROX.	ELEV.	1476 <u>+</u>	DATE	7/16/2002	
D E P T H F T	S A M P L E S	* R S E A S I P S L I A N C G E	D E P T H			DESCRIPTIO	ЭМ		
		26		FILL: Brown coars	se to fine SAI	ND, little Silt, tra	ace Gravel	W.C.=3.6%	
5 <del></del>		13		Brown coarse to fin	e SAND, litt	le Silt, trace Gra	vel		
10-		26							
15 —		20		Brown coarse to fin	e SAND, son	ne Silt, trace Gra	ivel		
20 —	**	50/0"		with Cobbles and B	loulders				
25 — —		48							
30 —		82							
35 <del></del>				BORING COMPLE	STE AT 32 F	EET			;
			NCHO.D. SPL ER 30 INCH DI	IT BARREL ROP * Blows/Ft.		TO WATER: KS: AT COMP	20 ft. <u>+</u> LETION OF F	DATE:	7/16/2002

PROJ	PROJECT NO.		N-5485 INSPE		TED BY:	MM	1 BORING NO.		B-2
LOCA	TION	1 8	SEE FIGURE	E 1 APPRO	X. ELEV.	1474 <u>+</u>	DATE	7/16/2002	
D E P T H	SAMPLES	* R S S S S S S S S S S S S S S S S S S S	D E P T H F T			DESCRIPTIC	PN .		
0		12		FILL: Brown coa		ND, little Silt, tra	ce Gravel		
		50/0"							
10		9		Brown coarse to fi with occasional Co	ine SAND, littl obbles and Bot	ie to some Silt, tra ilders	ace Gravel		
15		30							
25 —		45		Brown coarse to fi	ne SAND, son	ne Silt, little Grav	el		
		50/0"	 	with Cobbles and I	Boulders	·			i
30 — 35 —		D. a Mi		AUGER REFUSA BORING COMPL	ETE AT 26.5	FEET			
140 L	B. H	:H: 2-IN IAMMEI	CH O.D. SPI R 30 INCH D	LIT BARREL PROP * Blows/Ft.	,	O WATER: S: AT COMPLI	20 ft. <u>+</u>	DATE:	7/16/2002

PROJECT NO.		N-5485	INSPEC	TED BY:	MM	BORING N	o.	B-3	
LOCA	TION		EE FIGURE 1	APPRO	X. ELEV.	1467 <u>+</u>	DATE	7/15/2002	
DEPTH FT.	P L E S	* SAM PLANCE	D E P T H			DESCRIPTIO	N	- , _	
0		58		FILL: Brown me	dium to fine G	RAVEL, little co	erse to fine Sand	d, trace Silt	
5		6		FILL: Brown con	rse to fine SAN	ND, little to some	Silt, trace Grav	el	
10		15		with Cobbles					
15		14		FILL: Brown coar	rse to fine SAN	D, some Silt, trac	ce Gravel with (	Cinders and A	rsh
20		8		Dark Brown coarse Brown coarse to fin	e to fine SAND ne SAND, som	, some Silt with one Silt, trace Grav	organic el with organic		
25		61		Brown coarse to fin	ne SAND, som	e Silt, trace Grav	el with Cobble	<del></del>	
30 -	4 1 4 1 3 1	min. min. min. min. min.		Rock Core #1 Recovery = 10"/60" RQD = 6.25"/60" =	" = 16.7% = 10.4%			·	
SAM 140 L	IPLER B. HA	: 2-INC MMEF	CHO,D. SPLIT R 30 INCH DRO	BARREL P * Blows/Ft.		O WATER: S: AT COMPLE	19.5 ft. <u>+</u>	DATE:	7/15/2002

Fig. 4 SESI CONSULTING ENGINEERS PC

PROJECT NO.		N-5485	. INSPECTI	ED BY:	MM	BORING	NO.	B-3(cont.)	
LOCATION S		SEE FIGURE	APPROX.	ELEV.	1467 <u>+</u>	DATE	7/15/2002		
DEPTH FT.	SAMPLES	REASIPSTIANGE	D E P T H			DESCRIPTIO	i <b>N</b>		
35	***************************************	4 min. 6 min. 6 min. 5 min. 5 min.		Rock Core #2  Recovery = 10"/60"  RQD = 4"/60" = 6.	7% 				
45				BORING COMPLE	ETE AT 39 FE.	ET			
55 — 60 — 65 —									
				PLIT BARREL DROP * Blows/Ft.		O WATER:	19.5 ft. ±	DATE:	7/15/2002

PROJE	ECT NO.	N-5485	INSPEC	MM	MM BORING NO.			
LOCA	TION	SEE FIGURE	1 APPROX	X. ELEV.	1468 <u>+</u>	DATE	7/16/2002	
D E P T H F T	S A S E S A S E P P S E S N N G C E	E P T H			DESCRIPTIC	N		
0 —	15		FILL: Brown coa	rse to fine SAI	ND, little Silt, tra	ce Gravel V	V.C.=1.5%	
5	14		Brown coarse to fi	ine SAND, littl	le Silt, trace Grav	el, trace wood	l	!
15	8							
20	13		Brown coarse to fin	ne SAND, liul	e to some Silt, tra	ace Gravel	<del>-</del>	
25	28		Brown coarse to fir	ne SAND, som	ne Silt, little Grav	el .		
30			AUGER REFUSAI BORING COMPLI	L AT 25.5 FEE ETE AT 25.5 I	3T FEET			
35 — SAM	IPLER: 2-II	NCH O.D. SPLI	IT BARREL ROP * Blows/Ft.		O WATER:	18.5 ft. <u>+</u>	DATE;	7/16/2002
		ELLON MACH DE	TOF Blows/Ft.	REMARK	S: AT COMPLE	TION OF BO	RING	

PROJECT NO.		NO.	N-5485	INSPECTE	ED BY:	MM	BORING	NO.	B-5
LOCA	10IT.	N s	EE FIGURE	1 APPROX.	ELEV.	1471 <u>+</u>	DATE	7/10/2002	•
D E P T H F T	SAMPLES	· R S E S S I P S T I N C E	D E P T H			DESCRIPTIO	)N	,	
0		70		FILL: Brown coars	se to fine SAI	ND, little Silt, tra	ace Gravel with	h occasional Co	bbles
5—		7		Brown coarse to fin	e SAND, litt	le to some Silt, t	race Gravel		
10—		52		with occasional Co	bbles				
15 —		50/4"		with Boulders					
20 —		52		Brown coarse to fin	ne SAND, litt	lle Silt, trace Gra	vel with Cobb	ie	
30 —		50/2" 5 min 5 min 6 min 5 min 6 min		with Shale  Rock Core  Recovery = 54"/60"  RQD = 21.5"/60" =  AUGER REFUSAL  BORING COMPLE	= 35.8% L AT 25.5 FE		ij		
11				PLIT BARREL DROP * Blows/Ft.	1	TO WATER:	18 ft. <u>+</u> LETION OF	DATE: BORING	7/10/2002

PROJECT NO.	N-5485	· INSPECT	ED BY:	MM	BORING	NO.	B-6
LOCATION	SEE FIGURE 1	APPROX	. ELEV.	1475 <u>+</u>	DATE	7/10/2002	
D S + R S E S E S E S E S E S E S E S E S E S	D E P T H			DESCRIPTIC	N .		
19		FILL: Brown coars	se to fine SAND,	some Silt, lit	tle Gravel		
5 7		Brown coarse to fir	ne SAND, some S	ilt, little Gra	vel		
10 22			·				
15 56		Red/Brown coarse	to fine SAND, so	me Silt, trace	Gravel with	Weatherd Rock	:
50/3"		Brown coarse to fin	ie SAND, some S	ilt, little Grav	el with Weat	hered Rock	
25 50/4"		with occasional Co AUGER REFUSAI BORING COMPLE	L AT 26 FEET				
30						•	
	NCH O.D. SPLIT ER 30 INCH DRO		DEPTH TO		18 ft. <u>+</u> ETION OF B	DATE:	7/10/2002

PROJE	CT NO	).	N-5485		INSPECTED	BY:	MM	BORING I	vo	15-7	
LOCATI	ION	SI	EE FIGURE	1	APPROX. EI	LEV.	1475 <u>+</u>	DATE	7/10/2002		
P I	E L S !	S E A S	D E P T H F T				DESCRIPTIO	DN			
0		35		FILL:	Brown coarse I	oarse to fine SAND, little Silt, trace Gravel					
5		119			rown coarse to t		liule Silt, trace	Gravel			
10 —		83									
15		23		with o	occasional Cobb	oles					
20 —		73		Red/F	Brown coarse to	fine SAND,	some Silt, littl	e Gravel with	Weathered Rock		
25 —	6	f min. 5 min. 5 min. 7 min. 7 min.			Core Recovery = 35.75*/60" =		66.7%		·		
35 —				BOR	ING COMPLET	TE AT 30 FF	eet .				
			NCHO.D. S ER 30 INCH				O WATER:	19 ft. ± PLETION OF	DATE: BORING	7/10/2002	

PROJ	ECT	NO.	N-5485	INSPECTE	D BY:	RH	BORING I	۷0.	B-8
LOCA	TIOI	N SI	EE FIGURI	E1 APPROX.	ELEV.	1471 <u>±</u>	DATE	7/12/2002	
D E P T H F T .	SAMPLES	· SESISTANCE	D E P T H F			DESCRIPTIO	ON.		
0—		53		FILL: Brown coars	e to fine SAN	D, trace Silt, tr	ace Gravel		
5—		20							
10-		18							
15—		16		with Brick and Asp	halt			11	
20 —		12		Brown coarse to fin	ne SAND and	Silt, trace Grav	/el		
25 — —				BORING COMPLE	ETE AT 22 FI	EET			
30				·					
				SPLIT BARREL I DROP * Blows/Ft.		TO WATER: KS: AT COM	17 ft ± PLETION OF	DATE: BORING	7/12/2002

PROJECT NO	). _	N-5485	INSPECTED	BY:	RH	BORING	NO.	D*7
_OCATION	SI	E FIGURE 1	APPROX. EL	.EV.	1471 <u>+</u>	_ DATE	7/12/2002	
E A S P M T P I L E F S T	· FESISAISTANCE	D E : P H F			DESCRIPTI	ON		
0	22		FILL: Brown coarse t	lo fine SAI	ND, little Silt, t	race Gravel		
5	12							
10	<b>i</b> 1		with Decomposed Wo	ood				
15	41		Gray/Brown coarse to	o fine SAN	D, liule Silt, tr	ace Gravel wit	h Weathered Sha	ale
20 —	15		Brown coarse to fine	SAND an	d clayey Silt			
25 —			BORING COMPLE	TE AT 22	FEET			
			PLIT BARREL DROP * Blows/Ft.		H TO WATER		DATE:	7/12/20

ROJECT NO.	N-5485	INSPECTED BY:	MM	BORING 1	NO.	
OCATION	SEE FIGURE 1	APPROX. ELEV.	<u>1469±</u>	DATE	7/11/2002	
D S R E A S E P M A S T P M 1 H L P S I A T S I A T S C C	F T	·	DESCRIPTIO	ON		
57		FILL: Brown coarse to fine S	AND, little Silt, tr	race Gravel		
105		with Wood, Ash, and Concre	ste			
15 40	,	Brown/Black coarse to fine	SAND, some Silt, t	trace Gravel, t	race wood	
20 50/	2"	with Boulders		·		
25	9	Brown coarse to fine SANE	), little Silt, trace C	Gravel		
30		BORING COMPLETE AT	27 FEET			
SAMPLER	: 2-INCH O.D. SPL MMER 30 INCH D	IT BARREL DE	PTH TO WATER	14 ft. ±		7/11/29

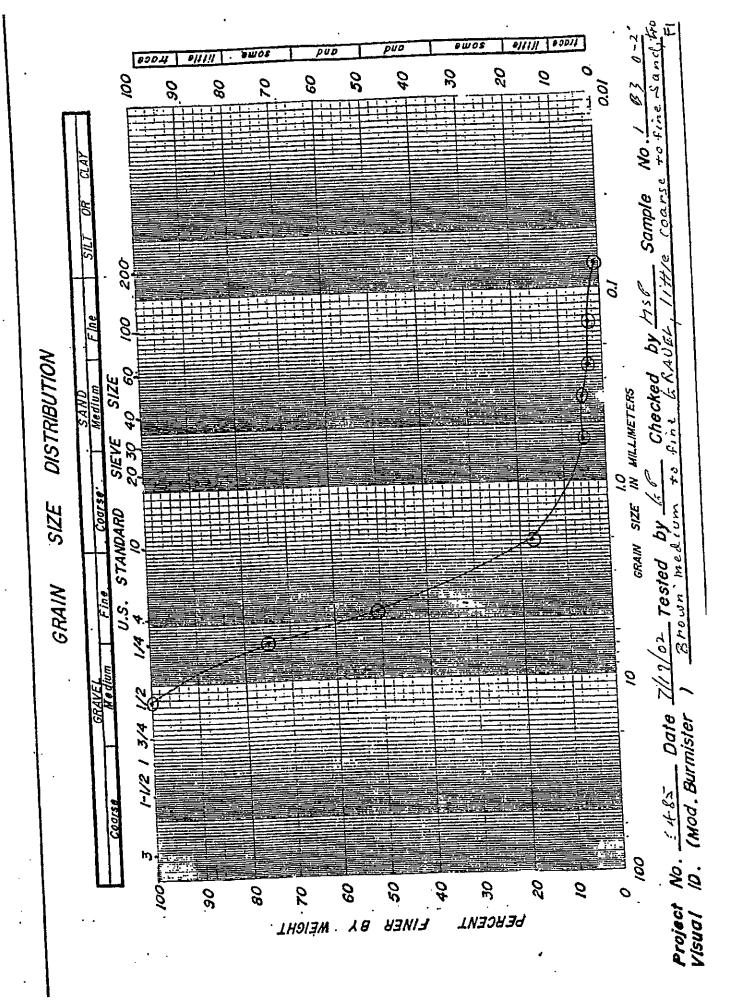
PROJE	CT NO.	N-5485	INSPECTED	BY:	RH	BORING I 	۷o	571
LOCAT	TON S	SEE FIGURE 1	APPROX. EI	LEV.	1471 <u>+</u>	DATE	7/12/2002	
E P T	S R A S E M A S P M I P S E L T S I A N C G C	D. E P T H			DESCRIPTIO	ON		
0	30		FILL: Brown coarse	to fine SAN	D, little Silt, tr	ace Gravel		
5	15		Brown medium to fin with occasional Cobb		ome Silt, trace c	organic clayey	Silt	
10 —	10							
20			With Grey Shale  AUGER REFUSAL A BORING COMPLET					
		-INCH O.D. SPL MER 30 INCH D	IT BARREL ROP * Blows/Ft.		TO WATER:	PLETION OF	DATE: BORING	7/12/2002

ROJECT NO.	N-5485	INSPECTED BY	; R	H BORING	NO.	时人
OCATION	SEE FIGURE 1	APPROX. ELEV	'. <u>1471+</u>	DATE	7/12/2002	
E	R D E S I S I T A R C E		DESCR	IPTION		
0 40	0	FILL: Brown coarse to fin	ne SAND, little G	ravel, little Silt	W.C.=9.2%	
52	1	Red/Brown coarse to fine	e SAND, little Silt	, trace Gravel		
10	7	with occational Cobbles				
15	3					
20	22	Brown coarse to fine SA	AND, some clayey	Silt, trace Gravel		
25 — 30 — 35 — 35 — 35 —		BORING COMPLETE	AT 22 FEET			
SAMPLE	R: 2-INCHO.D. SP	·-· ·	DEPTH TO WA	TER: 15.5 (		7/12/20

LOCATION SEE FIGURE	APPROX. EL	LEV.	1473 <u>+</u>	DAŢE	m /4 4/50.04	
<del></del>					7/12/2002	
D S F R D E P M A S P T P M I T P S L T H E L T F S I A N N F T E C E E P T E E E E E E E E E E E E E E E E			DESCRIPTIO	N ·		
52	FILL: Brown coarse	to fine SAI	ND, trace Silt, tra	ace Gravel		
19	Brown coarse to fine	SAND, tra	ce Silt, trace Gra	vel		
10	with occasional Cobb	bles				
15 22						
32	Brown medium to fi	ne SAND,	some clayey Silt			
25 —	BORING COMPLE	TE AT 22	FEET			
SAMPLER: 2-INCHO.D.			H TO WATER:	21.5 ft. <u>+</u>		7/12/2002

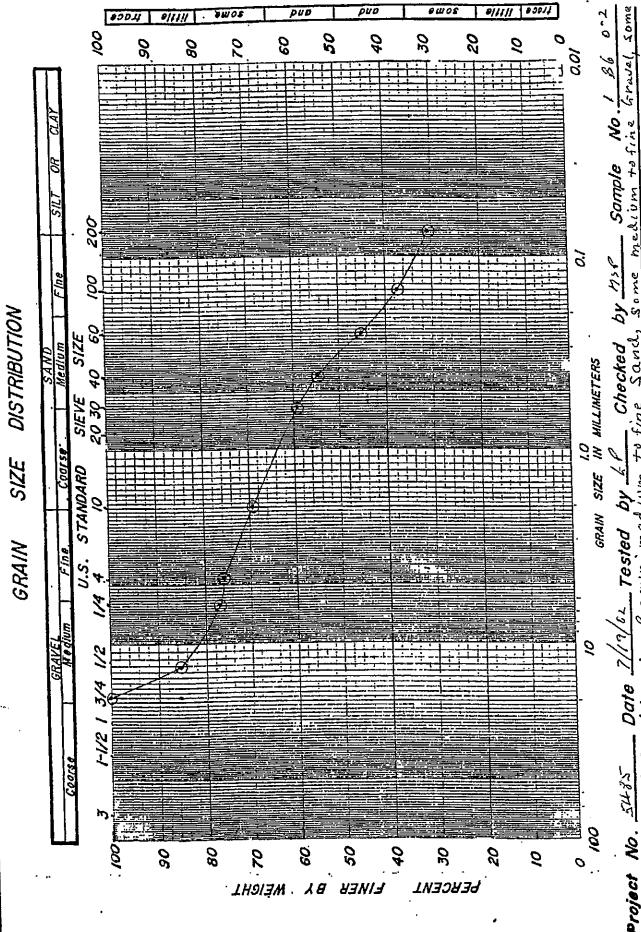
ROJECT NO.	N-5485	INSPECTED	BY:	MM	BORING I	<b>NO.</b>	D-14
OCATION	SEE FIGURE 1	APPROX. EL	LEV.	1466 <u>+</u>	DATE	7/16/2002	
D S R E A SE P M AS T P M I P S I A N N G C E	D E P T H F			DESCRIPTIO	ON		
28		FILL: Brown coarse	to fine SAN	D, little Silt, li	ttle Gravel		
33		FILL: Brown coarse with Cobbles, Boulde			race Gravel, tr	ace Ash	· · · · · · · · · · · · · · · · · · ·
29							
115 11							
20 16		Brown coarse to fine	s SAND, litt	ile Silt, little Gr	ravel		
30		BORING COMPLE	TE AT 22 F	FEET		·	
	2-INCH O.D. SPL MMER 30 INCH D			TO WATER		DATE:	7/16/2

ROJECT	NO.	N-5485	INSPECTED I	BY:	MM	BORING I	NO.	B-15			
LOCATIO	N SE	E FIGURE	1 APPROX. ELI	EV.	1466 <u>+</u>	DATE	7/16/2002				
D S A M P L E S T	* RESISTANCE	D E P T H		DESCRIPTION							
	77/8"		FILL: Brown coarse to	FILL: Brown coarse to fine SAND, little Silt, trace Gravel							
5	21		with occasional Cobble	with occasional Cobbles							
10-	23		Brown coarse to fine S	<u>.</u>							
25	16		Brown coarse to fine		<del>-</del> -	ravel with occ	asional Cobble				
SA 140	MPLER: 2 LB. HAM	-INCHO.D. VIER 30 INC	SPLIT BARREL H DROP * Blows/Ft.	DEPTH REMAI	TO WATER	: MPLETION	DATE: OF BORING	7/16/200			

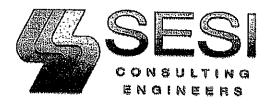


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Sand  $\infty$ No. 54435 Date 10. (Mod. Burmister Project Visual



# TEST PIT LOGS FOR PROPOSED TRACK AREA (FOR STORMWATER PURPOSES) 4/14/08

PROJ	JECT NO.	N-5500	PROJECT _	М	onticello, NY	TEST PIT	NO.	TP-T1
LOC	ATION	SEE FIGURE 1	APPROX. EL	EV.	1480' <u>+</u>	INSPECTE	D BY	JZ
WAT	ER OBSEI	RVATION	Seepage @ 4.0 F	eet		DATE EXC	CAVATED	4/14/2008
DEPTH FT.		DES	CRIPTION / SOIL	. CLA	SSIFICATION			DENSITY OR
0	6" T	opsoil/ Grass/ I	Roots					
1				~				
2_	Red-	-brown coarse t	o fine SAND, son	ne Silt,	, little coarse to f	ine Gravel	Medi	ium Dense
3	r	arge Boulders						•
_		ango Donidoro						1
4								
	g.					·		
5	1				•			to
	1							
6								
7_								
/—								
8								
	Ì							
9	,	vith Cobble and	1 Boulders					Dense
10								
_	TES	ST PIT COMPI	LETE AT 9.5 FEE	T				
11								
12								
-								
13								
14								

Fig. 2

NOTE:

SESI CONSULTING ENGINEERS, PC

PROJ	IECT NO.	N-5500	TEST PIT	NO.	TP-T2			
LOCA	ATION	SEE FIGURE 1	APPROX. EL	EV.	1502' <u>+</u>	INSPECTE	ED BY	JZ
WATI	ER OBSEF	NOITAV	Seepage @ 3.0 F	eet		DATE EXC	CAVATED	4/14/2008
DEPTH FT.		DES	SCRIPTION / SOIL	- CLAS	SSIFICATION			DENSITY OR
0	Tops	oil/ Grass/ Ro	ots					
1	FILL	: Brown medi	um to fine SAND,	little n	aedium to fine Gra	vel, little Silt	1	Loose
3	FILL	:Red-brown co	oarse to fine SANI	D and i	Silt, little medium	to fine Gravel	]	Loose
45		brown coarse t	to fine SAND, son	ne Silt,	little medium to fi	ine Gravel	Medi	um Dense
6 7								to
8 — 8	wi	ith Cobbles an	d Boulders			·		Dense
10  11	TEST	PIT COMPI	ETE AT 9.5 FEE	T				
12 <u>-</u>								:
13								
NOTE:	L				S	ESI CONSUL	TING EN	INEERS, PC

PROJ	ECT NO.	N-5500	PROJECT	Moi	nticello, NY	TEST PIT	NO.	TP-T3			
LOCA	ATION	SEE FIGURE 1	APPROX. EL	EV.	1507' ±	INSPECTE	ED BY	JZ			
WATE	ER OBSE	RVATION	Seepage @ 3.0 F	eet		DATE EXC	CAVATED	4/14/2008			
DEPTH FT.		DES	CRIPTION / SOIL	_CLAS	SIFICATION			DENSITY OR			
0	6" To	opsoil/ Grass/ F	Loots	-		, <u></u> .					
			<del></del>								
1											
2	Red-	brown medium	to fine SAND, so	ome Silt	, little medium t	o fine Gravel	Medi	um Dense			
· —											
3											
4					to						
5	w	ith Cobbles									
6	v	vith Cobbles an	d Boulders					Dense			
7											
'											
8											
9					~						
10											
_	TES	T PIT COMPL	ETE AT 9.0 FEE	ET							
11			•								
<u></u> -											
' <u>-                                   </u>											
13											
	_										
14											
NOTE:	<u> </u>					SESI CONSUL	TING EN	SINEERS, PC			

PROJECT NO. N-5500			PROJECT _	M	onticello, NY	TEST PIT	NO.	TP-T4
LOCA	ATION	SEE FIGURE 1	APPROX. EL	EV.	1513' <u>+</u>	INSPECT	ED BY	JZ
WAT	ER OBSEI	RVATION	Seepage @ 1.5 F	eet		DATE EX	CAVATED	4/14/2008
DEPTH FT.		DES	CRIPTION / SOIL	. CLA	SSIFICATION			DENSITY OR SISTENCY
0	6" To	opsoil/ Grass/ F	Roots	<b>-</b>				
1								
	Red-	brown medium	to fine SAND, so	ome Si	ilt, little medium	a to fine Gravel	]	Dense
2					ŕ			
3								
<u> </u>	w	ith Cobbles			to			
4								
_								
5	w	ith Cobbles and	d Boulders				Ve	ry Dense
6								
- 7	[							
7								
88				•				
9							-	
10								
	TES	T PIT COMPL	ETE AT 9.5 FEE	T				
11								
12								
·	:					·		:
13								
14					•			
NOTE:						SESI CONSUL	 TING ENG	INEERS PC

Fig. 5

:

NOTE:

PROJ	ECT NO.	N-5500	PROJECT	Mo	nticello, NY	TEST PIT	NO.	TP-T5
LOCA	ATION	SEE FIGURE 1	APPROX. EL	EV.	1510' <u>+</u>	INSPECTI	ED BY	JZ
WATI	ER OBSE	RVATION	Seepage @ 1.5 Fo	eet		DATE EX	CAVATED	4/14/2008
DEPTH FT.		DES	SCRIPTION / SOIL	CLAS	SSIFICATION			DENSITY OR
0	Tops	oil/ Grass/ Ro	ots			· <u>.</u>	-	Soft
-							:	
1—								
2				:				
<u>د</u>	Dod	h						
3	Keu-	brown mediun	io nne Gravei	1	Dense			
_								
4	:							
5								
								•
6								
7		ide Caldalaa an	4 D14					
	W	ith Cobbles an	g Boulders					
8_				-a				
_	Refu	sal on Rock						
9	TES	T PIT COMPI	ETE AT 8.0 FEE	Т				
· —								
10 —								
							ļ	
11								
12								
'  								
13								
14								
NOTE:		<del></del>				SESI CONSUL	TING ENG	INEERS, PC

PROJ	ECT NO. N-5500	PROJECT	Monticello, NY	TEST PIT	NO. TP-T6		
LOCA	TION SEE FIGURE 1	APPROX. ELEV	1489° <u>+</u>	INSPECTE	D BY	JZ	
WATI	ER OBSERVATION S	eepage @ 2.0 Feet		DATE EXC	AVATED 4/14/2008		
DEPTH FT.	DESC	RIPTION / SOIL C	LASSIFICATION		RELATIVE DENSITY OR CONSISTENCY		
0	Topsoil/ Grass/ Roots			Soft			
1							
2—	D. M	Eng SAND	014 1141 3in - 4 - E-	on Creasel		D	
3	Ked-blown coarse to	ime sand, some	Silt, little medium to fir	ic Gravei		Dense	
_							
4—	with Cobbles and	Boulders					
5					to		
_							
6							
7							
_							
8—					Ve	ery Dense	
9		<del>,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,</del>					
10	Refusal on Rock TEST PIT COMPLE	ጉፑ ልጥ ዐ በ ፑፑፑጥ					
11	1231111 COWIFE	LIMIL YOURDE					
12							
13	:						
						•	
14							

SESI CONSULTING ENGINEERS, PC

NOTE:

PROJECT NO		N-5500	PROJECT	Мо	nticello, NY	T	EST PIT	NO.	TP-T7			
LOCA	NOITA	SEE FIGURE 1	APPROX. EI	_EV.	1492' <u>+</u>	. [	NSPECTE	ED BY	JZ			
WATI	ER OBSEI	RVATION	Seepage @ 3.0 l	Feet			OATE EX	CAVATED	4/14/2008			
DEPTH FT.		DES	SCRIPTION / SOI	L CLAS	SIFICATION				DENSITY OR BISTENCY			
0	Торя	soil/ Grass/ Ro	ots						Soft			
1_				<del></del>								
_												
2_	  -											
	Red-brown medium to fine SAND, some Silt, some medium to fine Gravel Medium Dense											
3												
4												
5									4-			
6									to			
II —												
7												
∥		with Cobbles a	nd Boulders						Dense			
8_												
9				<del></del>			<del></del>					
10		fusal on Rock	27 27 DE 400 0 0 101	D D OD			•					
11_	TE	ST PIT COMP	PLETE AT 9.0 FI	2E I								
12												
	-					٠						
13_	·											
14	-											
NOTE:	<del></del>				. <del></del>	SESI	CONSU	LTING EN	IGINEERS, PO			



 TEST PIT NO.:
 RT-TP-1

 SHEET
 1
 OF
 1

 DATE: START:
 10/3/08
 END:
 10/3/08

	PROJ	ECT NAME:	Concord	d LOCATION: Race Track	END: 10/3/08
	JOB N	IO.: <u>5485</u>	4	CLIENT: Cappelli-Concord	O.G. ELEV.: Approx. 1545
		ECTOR:		CONTRACTOR: Nacirema	
		PMENT: E			
		R LEVEL DE		N/A NOT ENCOUNTERED: X	
			. –		
0	рертн (ғт)	SAMPLE NO./ TYPE	SOIL	DESCRIPTION	REMARKS
				Red-Brown medium to fine Sand, some Silt, little coarse to fine Gravel	
				With Gray coarse grained sandstone cobbles from 5-ft. to 20-ft.	
				(Mottled with Orange medium to fine Sand from 1-ft. to 4-ft.)	
10					
20					
		!		End of Test plt at 24-ft. (no bedrock encountered)	•
		~~.			
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		!			
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	PROJ	ECT NAME:	Concor	LOCATION: Race Track	END: <u>10/3/08</u>
	JOB N	NO.: 5485		CLIENT: Cappelli-Concord	O.G. ELEV.: Approx. 1548
	INSPE	ECTOR: _C	DDM	CONTRACTOR: Nacirema	
	EQUI	PMENT: E	Backhoe-365	ic	
	WATE	R LEVEL DI	EPTH: _	N/A NOT ENCOUNTERED: X	
0	<b>DEPTH (FT)</b>	SAMPLE NO./ TYPE	SOIL	DESCRIPTION	REMARKS
				Red-Brown medium to fine Sand, some Silt, little coarse to fine Gravel	
				With Gray coarse grained sandstone cobbles from 4-ft. to 24-ft.	
				With Gray Godined Sandstone Cobbies non 4-16. to 24-16	
10					
					•
	-				
20					
	-			·	
				End of Test pit at 24-ft. (no bedrock encountered)	
				, , , , , , , , , , , , , , , , , , , ,	,
		٠			
		<b>`*</b> .			
30		•			
40					



TEST PIT NO.: RT-TP-3
SHEET 1 OF 1
DATE: START: 10/3/08
END: 10/3/08

	PROJ	ECT NAME:	Concord	LOCATION: Race Track	END: 10/3/08
	JOB N	NO.: _5485		CLIENT: Cappelli-Concord	O.G. ELEV.: Approx. 1550
		ECTOR:		CONTRACTOR: Nacirema	
			Backhoe-365		
		ER LEVEL D		N/A NOT ENCOUNTERED: X	
0	ОЕРТН (FT)	SAMPLE NO./ TYPE	SOIL	DESCRIPTION	REMARKS
				Red-Brown medium to fine Sand, some Silt, trace Gravel	
				Mottled veins of Orange medium to fine sand from 0-ft. to 2-ft.	
				Red-Brown medium to fine Sand, some Silt, little coarse to fine Gravel	
				With Gray coarse sandstone cobbles from 4-ft. to 23-ft.	
0					
	-				
	-				
20					
				End of Test pit at 23-ft. (no bedrock encountered)	•
		<b>~</b> .			
		-			
30					



PROJECT NAME: Concord · LOCATION: Race Track

TEST PIT NO.: RT-TP-4
SHEET 1 OF 1
DATE: START: 10/3/08
END: 10/3/08

	JOB N	NO.: 5485	5	CLIENT: Cappelli-Concord	O.G. ELEV.: Approx. 1548
		ECTOR:		CONTRACTOR: Nacirema	
	EQUI	PMENT: _	Backhoe-36	95C	
	WATE	ER LEVEL (	DEPTH:	N/A NOT ENCOUNTERED: X	
0	ОЕРТН (FT)	SAMPLE NO./ TYPE	SOIL	. DESCRIPTION	REMARKS
			,	Red-Brown medium to fine Sand, some Slit, little medium to fine Gravel	
				Red-Brown medium to fine Sand, some Slit, little coarse to fine Gravel	
				With Gray coarse sandstone cobbles from 4-ft. to 24-ft.	
0					
20					
				End of Test pit at 24-ft. (no bedrock encountered)	
		**.			
30					



TEST PIT NO.: RT-TP-5
SHEET 1 OF 1
DATE: START: 10/3/08
END: 10/3/08

	PROJ	ECT NAME:	Concord	LOCATION: Race Track	END: 10/3/08
	JOB N	1O.: 5485		CLIENT: Cappelli-Concord	O.G. ELEV.: Approx. 1538
		ECTOR: _C		CONTRACTOR: Nacirema	
		PMENT: _			
		R LEVEL DI		N/A NOT ENCOUNTERED: X	
0	ОЕРТН (FT)	SAMPLE NO./ TYPE	SOIL CLASSIFICATION	DESCRIPTION	REMARKS
				Topsoil	
				Red-Brown medium to fine Sand, some Silt, little medium to fine Gravel	
				Red-Brown medium to fine Sand, some Silt, little coarse to fine Gravel	
				With Gray coarse sandstone cobbles from 4-ft. to 24-ft.	
^					
0.					
20					
				End of Test pit at 24-ft. (no bedrock encountered)	
		4.			
30	_				
-					
		Ī			



## BORING LOGS, TEST PIT LOGS, AND MONITORING WELL LOGS FROM REMEDIAL INVESTIGATION 10/29/07 - 11/2/07 AND 5/5/08 - 5/14/08

	$U \subseteq \mathbb{C}$				LOCA	N NOITA	AME: Co	oncord F	Resort	BORING NO.				MW-1 OU-1cxis	
W. Taraba		OCNEUL	TING	l				nticello,			JOB NO.	J			1/00-1
BOD!		ENGINE			<u> </u>							I ENZATE	ON!		7180
POKI	NG BY: G	Bi / Jim C	. & Da	ve C.		START		10/2	9/2007	GROUND ELEVATION: GROUNDWATER TABLE DEPTH					
	ECTOR: J	<u> Z</u>			DATE	COMPL	ETED	10/3	0/2007	0 Hr. N/A Date					
DEPTH (ft)	PID (ppm)	SAMPLE		EPTH	_	Blower	on Spoo	,	Γ		IVA Z	+ rzr.	N/A	Date 1	N/A
0	PIU (ppm)	No.	FROM		0/6				REC	SOIL DESCRI	PTION AND	STRATI	FICATIO	N	SYMB
	45	1	0	2	8	6/12 17	12/18 15	18/24 20						•	O TREES
				<u> </u>	+	╁┷	1-13	20	18	(Top 2") Asphalt/ 3" of Gr	avel Base coar	rse below	Asphalt		┪
	0	2	2	4	31	27	30	33	24	(Bottom 13.) Ked/Brown	c-f SAND, littl	le m-f Gr	avel little	Silt	
					_	<del> </del>			24	Same					
5						†	<b></b>			-		•			
	0	3	5	7	17	35	37	45	24	0-					
					$\vdash$	<u> </u>	-			Same				_	<del> </del>
	0	4	7	9	37	38	41	40	24	Same					
								<del></del>		Paine					
10	0	5	9	11	51	30	33	45	20	Same					
ļ	<del>  </del>													_	
ļ	0	6	11	13	50/3				3	Same (rock in tip)					
				ļ						(rook at rib)					L_
16	0	7	13	15	20	31	18	40	18	Same					<u> </u>
15	<del></del>			<b></b>											<u> </u>
}	0	- 8	15	17	30	44	47	44	15	Same					<u> </u>
-	0	<del></del> -		<u> </u>						1			•		
<b>-</b>	<del></del>	_9	17	19	47	60	50	64	24	Same					<u> </u>
20	0	10	10												<b> </b> -
+	<del></del> -	-10	19	21	22	48	45	64	24	Same					
上	0	11	21	72	50/2		.	_							<del> </del>
	<del></del>	<del></del> +	21	23	50/3				_3	Same (rock in tip)					
	0	12	23	25	48	500		_							
25				ر.2	40	50/3			2	Same (rock in tip)				ļ	
7	0	13	25	27	22	32		<del></del>	<u> </u>						
Γ				<del></del> +		- 32	50/3		12	Same					
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<u> </u>								$\dashv$	<u>-</u>  3	ame (rock in tip)				7	
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<del></del>	
Nominal I.D. of Hole	In The subcurface information
Nominal I.D. of Split Barrel Sampler	In The subsurface information shown hereon was obtained for the design and estimating purposes for our client.  1% in It is made available to authorized users only that there.
Weight/type of Hammer on Drive Pipe	
Weight/lype of Hammer on Split Barrel	
Drop of Hammer on Drive Pipe	
Core Size	in engineers recommendations contained in the report from which these logs were extracted.  Pp: Pocket Penetrometer; WOH: Weight of Hammer; WOR: Weight of Rod
Soil descriptions represent a field identification	n after D. M. Burmister unless otherwise noted.
	FIGURE #
	Page 1 of #

	77 C				LOCA	TION	NAME: C	oncord	Resort	BORING NO MINI 4 LOW	BORING NO. MW-1/OU-1c			
		CNSUL	TING					nticello,		JOB NO. 7180				
		でこうことに	ER9							GROUND ELEVATION:				
		BI/Jim C	. & Dave	e C.		STAR		10/2	9/2007	GROUNDWATER TABLE DEPTH	_			
	ECTOR: .	JZ			DATE	COMP	LETED	10/3	0/2007	0 Hr. N/A Date N/A 24 Hr. N/A Date N/A	—			
DEPTH	2	SAMPLE		PTH		Blows	оп Ѕроо	n	REC	Date 14X				
(ft)	PID (ppm)	No.	FROM	-	<u> </u>	<u>-</u>			.,,	SOIL DESCRIPTION AND STRATIFICATION SYN	(BO)			
40	<u> </u>		(ft)	(ft)	0/6	6/12	12/18	18/24	(in)					
	0	15	40	42	50/5	├—	<del> </del> -	<b> </b>	4	Same (rock in tip)	_			
1		ļ	ļ	<del> </del>	<b>├</b>	<u> </u>			<u> </u>	_				
	<b> </b> -		<u> </u>		ļ <u>.</u>	<del> </del>	↓	ļ						
45				-	├		╂	ļ	<u> </u>	_	_			
70	0	16	AE.	47	15					_	_			
-	-	10	45	47	45	50	44	60	12	Same (rock in tip)	_			
	<del></del>	<u></u>	<del> </del>	<del> </del> -	<del> </del>	<del> </del>	<del> </del> -		<u> </u>	4	_			
			<b> </b>	<del> </del> -	<del> </del>	<del>  -</del> -		<u> </u>	ļ	<u>-</u>				
50	0	17	50	52	50/1	├	·		<u> </u>	4.	_			
	<b>—</b> —		50	32	30/1	-	+	<del> </del> -	1	Same (rock in tip)				
					<del> </del>	_				END OF BORING AT 50 FEET				
					<del>                                     </del>				<del></del>	Refusal on Rock	_			
							<del> </del>	<del> </del>		NO Ground Water				
55							<del> </del>			NO Monitoring Well				
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Nominal I.D. of Hole	in	The subsurface information shows because the subsurface information shows because the subsurface information shows the subsurface in
Nominal I.D. of Split Barrel Sampler	1½ in	The subsurface information shown hereon was obtained for the design and estimating purposes for our client.
	300 lb	It is made available to authorized users only that they may have access to the same information available to our client. It is presented in good faith, but it is not intended as a substitute for investigations, interpretations
Weight/type of Hammer on Split Barrel	140 lb	or judgment of such authorized users. Information on the logs should not be relied upon without the geotechnica
Drop of Hammer on Drive Pipe	In	engineers recommendations contained in the report from which these logs were extracted.
Core Size		Pp: Pocket Penetrometer; WOH: Weight of Hammer; WOR: Weight of Rod

Approximate Change in Strata: \_ Inferred Change in Strata: \_\_

Soil descriptions represent a field identification after D. M. Burmister unless otherwise noted.

FIGURE #

	117 G				TLOCA'	TION N	ANGELO			<del></del>			<del></del>	<del></del>	.100	*******	V-412
	E CHICAGO	، کا د	5		LUCA	HON IN		oncord R				BORING NO. MW-23 / OU		/ OU-1/			
	c	ENGINET	TING		Monticello, NY							JOB NO.				180	
BORI		BI/Jim C.			DATE STARTED 11/1/2007					GROUND ELEVATION:							
INSPI	ECTOR: J	17 Jill C.	Or Dave	C.					/2007	<u> </u>		GR/	OUNDWAT				
DEPTH		<del></del>	DE	PTH	DATE	DATE COMPLETED 11/				0 Hr.	N/A	Date	N/A	24 Hr.	N/A	Date N	/4
(ft)	PID (ppm)	SAMPLE			4	Blows (	on Spoor	n	REC			• ,					Ť-
0	, in their	No.	FROM	<del></del>	<u> </u>	<del>-,</del> -		-	<u> </u>	_	SOIL	DESCR	IPTION AN	ID STRAT	TEICATIO	N	SYMBO
-	<del> </del>	<del> </del>	(fl)	(ft)	0/6			18/24	(in)								
"	0	1	0	2	3	6	5	2	12	(Top 6")	Topsoil	Brown	c-f Sand, sor	me c-f Gra	vel. little Si		<del> </del>
	<u> </u>	<u> </u>	<b> </b> -'	<u> </u>	<del> </del>	ــــــ	<u> </u>			(Bottom	6") Red/	Brown :	m-f Sand, so	ome Silt. tr	are Gravel	CERN.	┼──
	0	2	2	4	9	22	34	50	24	Same	-				no charer	Tin	
_	<u> </u>	<del> </del> /	<u> </u>	<u> </u>		<u> </u>				]					-		<del></del>
5	0	3	4	6	50/3	<u>L</u>			3	Same							<del> </del>
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İ	0	4	6	8	32	56	50/3		12	Same						٠.	<b> </b>
	<b></b>	<b></b>			<u> </u>					İ							
	0	5	8	10	41	50/3			8	Same							<u></u>
10	<b></b> !			'													<u></u>
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Nominal I.D. of Hole	ln	The subsurface information shows because was able and a subsurface information shows because we are all the subsurface information shows because we are all the subsurface information shows because we are all the subsurface information shows because we are all the subsurface information shows because we are all the subsurface information shows because we are all the subsurface information shows because we are all the subsurface information shows because we are all the subsurface information shows because we are all the subsurface information shows because we are all the subsurface information shows because we are all the subsurface information shows because we are all the subsurface information shows because we are all the subsurface informations and the subsurface information shows because we are all the subsurface informations and the subsurface information shows a subsurface informati
Nominal t.D. of Split Barrel Sampler	1% in	The subsurface information shown hereon was obtained for the design and estimating purposes for our client.  It is made available to authorized users only that they may have access to the same information available
Weight/type of Hammer on Drive Pipe	300 lb	to our client. It is presented in good faith, but it is not intended as a substitute for investigations, interpretations
Veight/type of Hammer on Splil Barrel	140 lb	or judgment of such authorized users. Information on the logs should not be relied upon without the geotechnical
Orop of Hammer on Drive Pipe	ln	engineers recommendations contained in the report from which these logs were extracted.
Core Size		Pp: Pocket Penetrometer; WOH: Weight of Hammer; WOR: Weight of Rod
oil descriptions represent a field identification	after D	Approximate Change in Strata

Soil descriptions represent a field identification after D. M. Burmister unless otherwise noted.	
FIGURE#	•

	V L G				LOCAT	ON NA	ME: Co	ncord R	esort	BORING NO. MW-32 / OU-			
		<b>* 1</b>	I No				Mon	ticello, i	٧Y	JOB NO. 7180			
		BHGINE								GROUND ELEVATION:			
		BI / Jim C.	& Dave	C.	DATE	STARTE	ED	11/1/	2007	GROUNDWATER TABLE DEPTH			
1	CTOR: J	··				COMPL	ETED	11/2	2007	0 Hr. N/A Date N/A 24 Hr. N/A Date N/A			
DEPTH		SAMPLE	DEF	TH		Blows on Spoon REC							
(ft)	PID (ppm)	No.	FROM	TO					INEO	SOIL DESCRIPTION AND STRATIFICATION SYM			
0		ļ <u>.</u>	(ft)	(ft)	0/6	6/12	12/18	18/24	(in)				
C	0	1	0	2	2	2	_ 4	2	10	Brown m-f SAND, little m-f Gravel, little Silt			
				<u></u>						<u></u>			
	0	2	2	4	8	16	24	28	24	(Top 6") Same			
ļ _					ļ			<u> </u>	L	(Bottom 18") Red/Brown m-f SAND, some c-f Gravel, little Silt			
5	0	3	4	6	50/3			<u> </u>	3	(Very Dense, Hard to drill, and very dry)			
								<u> </u>		_			
	0	4	6	8	14	28	36	46	18	Same			
	<u> </u>	<del></del>		- <u>:</u> -	F0::	<u> </u>		<u> </u>		_			
10	0	5	8	10	50/3	<b>├</b> ──	ļ	<u> </u>	3	Same			
<u> </u>	0	6	10	10	50/2	<del> </del>	ļ	<u> </u>	<b></b> _	_			
	<del> </del>	-	10	12	50/3		<u> </u>		3	Same			
1	0	7	12	14	50/4		<del> </del>						
	- <u>`</u> -	<del></del>	12	14	30/4				4	Same			
15								<b>}</b>		-			
<del>                                     </del>	0	- 8	15	17	50/4		<del> </del>			<u> </u>			
l	├ <del>~</del> ~	-	1.5		30/4		<del> </del>	<del> </del>	3	Same			
l	0	9	17	19	30	44	60/4	<del> </del>	<del></del>	-			
	<b></b>						00/4		12	Same			
20	0	10	19	21	50/1		<del>                                     </del>		1				
					50/1		<del></del> -	<del>                                     </del>		Fractured Cobbles			
										<del> </del>			
										<del> </del>			
										┪			
25	0	11	25	27	100/5	_			0	No Recovery (Rock in tip)			
										END OF BORING AT 25 FEBT			
<b>i</b>										Refusal on Rock			
										Monitoring Well Installed			
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30										]			
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Nominal I.D. of Hole	in	The subsurface information shown hereon	was obtained for the design and estimating purposes for our client.				
Nominal I.D. of Split Barrel Sampler	1% in	is made available to authorized users only that they may have access to the same information avail					
Weight/type of Hammer on Drive Pipe	300 lb	to our client. It is presented in good faith,	but it is not intended as a substitute for investigations, interpretations				
Weight/type of Hammer on Splil Barrel	140 lb	or judgment of such authorized users. Information on the logs should not be relied upon without the geotechn					
Drop of Hammer on Drive Pipe	in	engineers recommendations contained in the report from which these logs were extracted.					
Core Size		Pp: Pocket Penetrometer; WOH: Weight of Hammer; WOR: Weight of Rod					
		Approximate Change in Strate:	Inferred Change in Strain				

Soil descriptions represent a field identification after D. M. Burmister unless otherwise noted.

FIGURE #

	ZZ C	LOCA	TION N	AME: C	oncord F	Resort	BORING NO. MW-36 //				
(E)		ONBUL!	TING		<u> </u>			nticello,		100 110	
POR		SHEDINE			<del> </del>	·				GROUND ELEVATION: 718	10
INSP	ECTOR:	BI / Jim C.	& Dave	C.		START			/2007	GROUNDWATER TABLE DEPTH	
DEPT		1	DE	PTH	DATE	DATE COMPLETED 11/2			/2007	0 Hr. N/A Date N/A 24 Hr. N/A Date N/A	
(ft)	PID (ppm)	SAMPLE	FROM		┪	Blows	on Spoo	n	REC		
0		No.	(ft)	(ft)	0/6	6/12	12/18	18/24	(In)	SOIL DESCRIPTION AND STRATIFICATION	SYMBOL
				<u> </u>	1		12710	10/24	(111)	Antak	
						1	<u> </u>	<del> </del>	<del>                                     </del>	Asphalt Concrete	
							<del>                                     </del>	1	<del></del>	Gravel	
_		<u> </u>								Concrete	
5	0	1	4	6	13	11	7	16	6	Brown c-f SAND, some c-f Gravel, little Silt (rock in tip)	
		<u> </u>			<b> </b>	ļ	$oldsymbol{ol}}}}}}}}}}}}}}}}}}$			- Control, fille ant (lock in tip)	
	0	2	6	.8	17	31	14	25	10	(Top 6") Gray c-f Gravel, little c-f Sand, trace Silt	
	0	3	- 8	10		F0.40	<del> </del>			(Bottom 4") Red/Brown c-f SAND, some Silt, little c-f Gravel	
10	- <u>*</u> -		6_	10	28	50/2	<del> </del>		6	Rocks in Tip/Rocks in spoon	
	0	4	10	12	31	27	24	28			
l					<del></del>		-27	20	12	Red/Brown c-f SAND, little Gravel, little Silt	
	0	5	12	14	61	50/3	<del> </del>		9	Red/Brown Shale	_
										Record town Shale	
15	<u> </u>									<u> </u>	
										<u> </u>	
	<u>  </u>									<u> </u>	
}	<del></del>										
20	<del>  </del>									END OF BORING AT 18 FEET	
										Auger Refusal on Rock	
]		··								Monitoring Well Installed	$\dashv$
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Nominal I.D. of Hole	in	The subsurface information above to
Nominal I.D. of Split Barrel Sampler	13% in	The subsurface information shown hereon was obtained for the design and estimating purposes for our client.
Weight/type of Hammer on Drive Pipe	300 lb	It is made available to authorized users only that they may have access to the same information available to our client. It is presented in good faith have in formation available
Weight/type of Hammer on Split Barrel	140 lb	to our client. It is presented in good faith, but it is not intended as a substitute for investigations, interpretations or judgment of such authorized users. Information and the little of investigations interpretations
	in	or judgment of such authorized users. Information on the logs should not be relied upon without the geotechnica engineers recommendations contained in the report from which these logs were extracted.
Core Size		Pp: Pocket Penetrometer; WOH: Weight of Hammer; WOR: Weight of Rod
Soil descriptions represent a field identification		Approximate Change in Strata:

Soil descriptions represent a field identification after D. M. Burmister unless otherwise noted,



TEST PIT NO.: TP-1
SHEET 1 OF 1
DATE: START: 5/5/08
END: 5/5/08
O.G. ELEV.: +1460

		JECT NAME:		LOCATION: Klamesha Lake, NY CLIENT: Cappelli	END: 5/5/08 O.G. ELEV.: +1460
				Charlle Paternostro CONTRACTOR:	
		PMENT: _		OOMINGOOK.	
	WĄTE	ER LEVEL D		Geepage @ 6' NOT ENCOUNTERED:	
C		OI9	RELATIVE	DESCRIPTION	REMARKS
		0	Loose	FiLL: Brown – medium to fine SAND, little silty Clay, Boulders & Cobbles Shale, with brick, concrete, lumber, asphalt	
5					
		0	Medium	Brown m-f SAND, some silty Clay, Boulders Shale	
0		0		END OF TEST PIT 9.5', SHALE BEDROCK ENCOUNTERED	
		-			
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οĻ					



TEST PIT NO .:	TP-2
SHEET 1	OF 1
DATE: START:	5/5/08
END;	5/5/08
O.G. ELEV.: +*	403

	PRO	JECT NAME	: Concor	d LOCATION: Klamesha Lake, NY	END: 5/5/08		
	JOB	NO.: <u>7375</u>	<u> </u>	CLIENT: Cappelli	O.G. ELEV.: +1493		
			Chris Mazur:	Charlle Paternostro CONTRACTOR:			
	EQU	IPMENT: _					
	WAT	ER LEVEL [	DEPTH: _	NOT ENCOUNTERED: X			
		<del></del> _	T				
			l m Z				
C		OIA	ATIV TISI	DESCRIPTION	·		
		4	RELATIVE DENSITY	DESCRIPTION	REMARKS		
					٠.		
		0	Loose	FILL: Brown - coarse to fine SAND, trace silty Clay medium to fine			
		-		Gravel, with brick, concrete, asphalt, cinders, rebar, trace Silt	,		
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			<u> </u>				
		0	Medium	FILL: Brown-Gray-Black, coarse to medium Sand, little silty Clay, with			
		0		tree roots  END OF TEST PIT 8', SHALE BEDROCK ENCOUNTERED			
				THE STATE OF STATE BEDROCK ENCOUNTERED			
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PROJECT NAME: Concord LOCATION: Kiamesha Lake, NY

TEST PIT NO.: TP-3

SHEET 1 OF 1

DATE: START: 5/5/08

END: 5/5/08

	JOB I	NO.: <u>7375</u>	<u> </u>	CLIENT: Cappelli	O.G. ELEV.: <u>+1488</u>
	INSPE	ECTOR: _			
	EQUI	PMENT: _			
	WATE	R LEVEL (	DEPTH:	NOT ENCOUNTERED: X	
			<del></del>		.,
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С		PID	RELATIVE DENSITY	DESCRIPTION	•
		11.	필	DESCRIPTION	REMARKS
				·	
		0	Loose	FILL: Brown - coarse to fine SAND, little silty Clay, with brick, concrete,	
				asphalt, black tar, glass, iron piping, ply wood, asbestos pipe	
				<u>.</u>	•
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		0		END OF TEST PIT @ 8' SHALE BEDROCK ENCOUNTERED	•
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PROJECT NAME: Concord

# FIELD TEST PIT LOG

LOCATION: Kiamesha Lake, NY

TEST PIT NO.: TP-4
SHEET 1 OF 1
DATE: START: 5/5/08
END: 5/5/08
O.G. ELEV.: ±1499.5

	JOB NO.: _73	75 (	CLIENT: Cappelli	O.G. ELEV.: <u>±1499.5</u>		
			Charlie Palernostro CONTRACTOR:			
	EQUIPMENT:					
	WATER LEVE	L DEPTH: _	NOT ENCOUNTERED: X			
С	OIA .	RELATIVE DENSITY	DESCRIPTION	REMARKS		
	0	Loose	Filt: Light Brown coarse to fine Sand, with brick, glass, wood, pipe, Cobbles Shale/Sandstone			
	0	Loose	Fit.L: Larger diameter pipe, pieces of boller, asbestos boller pack, wire mesh, steele, clay tile, light bulbs, fiber glass insulation cinder blocks, coarse to fine gravel			
5	. 0		END OF TEST PIT 5.5', SANDSTONE BEDROCK ENCOUNTERED			
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PROJECT NAME:

# FIELD TEST PIT LOG

	TEST PIT NO.: TP-5
	SHEET 1 OF 1
	DATE: START: 5/8/08
1	END: 5/8/08
	O.G. ELEV .: +1476

	PROJEC.		END: 5/8/08		
	JOB NO.: <u>7375</u>				O.G. ELEV.: <u>+1476</u>
			Chris Mazur;	Charile Palernostro CONTRACTOR:	
	EQUIPME				
	WATER I	.EVEL [	DEPTH: _	NOT ENCOUNTERED: X	
			≝≿		
С		문	RELATIVE	DESCRIPTION .	REMARKS
					TETTING QUO
		0	Loose	EILL Proug come to T. Oak P.	
		·	10038	FILL: Brown coarse to fine SAND, little slity Clay, with concrete, asphalt, brick, lumber	
			1		
			,		
		0	Medium	Fill - Slight stoiglag of all days to	
		•	111001011	FILL: Slight staining of soil, dark oily liquid coat on old wood (Creosote)	
			<u> </u>		
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		0		This or trover	
		Ü	}	END OF TEST PIT 6.0', SHALE BEDROCK ENCOUNTERED	
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TEST PIT NO.: _TP-6
SHEET 1 OF 1
DATE: START: 5/8/08
END: 5/8/08
O.G. ELEV .: + 1472

	PROJE	CT NAMI	E: Concor	d LOCATION: Kiamesha Lake, NY	END: 5/8/08
				CLIENT: Cappelli	O.G. ELEV.: + 1472
	INSPECTOR: Chris Mazur;		Chris Mazur	Charlie Paternostro CONTRACTOR	
	EQUIP	_			
	WATER	RLEVEL	DEPTH: _	NOT ENCOUNTERED; X	
					<u> </u>
_		_	RELATIVE		· .
С			=LAT ENS	DESCRIPTION	REMARKS
			20		
		0	Loose	FILL: Brown-light brown coarse to medium SAND, with lumber, pipe,	
				blacktop, trace Silt	
			1		
ı					
				·	
ļ			<u> </u>		
; [			-	Concrete Slab present in test pit	•
		0	Medium	Dark brown to Gray, coarse to fine SAND, Shale Cobbles,	
ŀ				trace Silt	
-				,	
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F		0		END OF TEST PIT 9.0', SHALE BEDROCK ENCOUNTERED	
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PROJECT NAME: Concord

TEST PIT	NO.:	_TP-7	
SHEET _	1_	OF _	1
DATE: ST	ART:	5/8/08	
;	END:	5/8/08	

	PROJECT	NAME: Concor	d LOCATION: Kiamesha Lake, NY	END: 5/8/08
	JOB NO.: _		CLIENT: Cappelli	O.G. ELEV.: <u>+ 1474</u>
	INSPECTO	R: Chris Mazur	Charlie Paternostro CONTRACTOR:	
	EQUIPMEN	T:		
	WATER LEV	VEL DEPTH:	NOT ENCOUNTERED: X	
С	OIA	RELATIVE	DESCRIPTION	REMARKS
		Loose	FILL: Brown/Red Shale Rock Brown - Black-coarse to medium Sand, little silty Clay, with cinders, sheet rock, burned lumber, metal pipe, wire, copper pipe	
	Č	) Medium	FILL: Gray- Black-Brown-Orange "rust", Brown coarse to medium SAND, weathered Shale rock	·
}	ō	Medium	Brown-medium to fine SAND, some silty Clay	
			END OF TEST PIT 8', SHALE BEDROCK ENCOUNTERED	
0  -				
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٠	TEST PIT NO .: TP-8
	SHEET 1 OF 1
	DATE: START: 5/8/08
_	END: 5/8/08
_	O.G. ELEV .: ± 1470

	PROJECT	NAME: Conco	DATE: START: 5/8/08	
	JOB NO.:		END: 5/8/08	
			CLIENT: Cappelli Charlie Paternostro CONTRACTOR:	O.G, ELEV.: <u>+</u> 1470
	EQUIPMEN	T. Citils Wiazu	; Charlie Paternostro CONTRACTOR:	· · ·
	WICKE	AEL DEBIH:	3.5' NOT ENCOUNTERED:	
ſ				<u>.                                    </u>
ĺ		m /		
c	2	RELATIVE	DECORPT	
		교	DESCRIPTION	REMARKS
	ĺ	_		
<b> </b>		) Medium	FILL: Brown/Red Shale rock on our	
-			FILL: Brown/Red Shale rock on surface, coarse Gravel with burned wood, metal, cinder block, rusted pipe, wire, Soll was multicolored,	
İ			black, brown, orange white, coarse to medium sand	
<u> </u>				
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İ		ľ	END OF TEST 6.0', SHALE BEDROCK ENCOUNTERED	
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	TEST PIT NO .: TP-9	
	SHEET 1 OF 1	_
ı	DATE: START: 5/6/08	-
Ì	END: 5/6/08	_
	OG ELEVI 14100	_

	PROJECT N	NAME: Concord	LOCATION: Klamesha Lake, NY	END: 5/6/08
	JOB NO.: _		CLIENT: Cappelli	O.G. ELEV.: ± 1486
	INSPECTOR		Charite Paternosiro CONTRACTOR:	
	EQUIPMEN		OOMINGOOR.	·
	WATER LEV	/EL DEPTH:	NOT ENCOUNTERED: X	
С	ĊIA	REI	DESCRIPTION	REMARKS
	0	Medium	Brown coarse to medium SAND, shale rock cobbles, pea gravel, trace slit	
	0	Medium	Concrete slab, with Asbestos Transite pipe, steele pipe, concrete	
	0	Dense	Prowin garges to flow and 1 /	
		201139	Brown coarse to fine sand, shale rock-cobbles, trace clay	
٠			END OF TEST PIT 4.0' bg, SAND STONE BEDROCK ENCOUNTERED	·
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PROJECT NAME: Concord

#### FIELD TEST PIT LOG

TEST PIT NO .: TP-10 SHEET \_\_1 OF \_\_1 DATE: START: 5/12/08 END: 5/12/08

	PROJECT NAT			END: 5/12/08
	JOB NO.: <u>73</u>	75	CLIENT: Cappelli	O.G. ELEV.: + 1460
	INSPECTOR:	Chris Mazur	Charile Paternostro CONTRACTOR:	<u></u>
	EQUIPMENT:			
	WATER LEVE	L DEPTH: _	NOT ENCOUNTERED; X	-
				T
		RELATIVE		
С	윤	LAT ENSI	DESCRIPTION	REMARKS
		Fig		I CLIVIAL ACTION
		·		
	0	Dense	Brown- coarse to fine sand, trace slit, Shale cobbles & boulders	
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ļ	<del>-  </del>	+	END OF TEST PIT 7.0' bg, SHALE BEDROCK ENCOUNTERED	
-			S. W. W. P. P. P. M. C. MODGIAL EKED	
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TEST PIT NO.: TP-11
SHEET 1 OF 1
DATE: START: 5/13/08
END: 5/13/08

	PROJECT JOB NO.:	NAME: <u>Con</u> _7375	cuient: Cappelli LOCATION: Klamesha Lake, NY	DATE: START: _5/13/08 END: _5/13/08 O.G. ELEV.: _ + 1492
		OR: Chris Ma	ur; Charlie Paternostro CONTRACTOR:	1702
		EVEL DEPTH:	NOT ENCOUNTERED: X	
С	<u> </u>	RELATIVE	DESCRIPTION	REMARKS
į		0 Mediu	Brown, coarse to fine SAND, Shale boulders & cobbles, with concrete, blacktop, rebar, brick, carpet, metal pipe, lumber	·
				•
5				
			END OF TEST PIT 5.5' bg, BEDROCK ENCOUNTERED	
				·
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5  -	-		·	
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### FIELD TEST PIT LOG

TEST PIT NO.: AE-1
SHEET 1 OF 1
DATE: START: 5/12/08
END: 5/12/08

	PROJECT NAM	VIE: Conco	rd LOCATION: Kiamesha Lake, NY	END; 5/12/08
			CLIENT: Cappelli	O.G. ELEV.: 1480
	INSPECTOR:	Chris Mazu	: Charlie Patemostro CONTRACTOR:	
	EQUIPMENT:			
Г	WATER LEVEL	. DEPTH:	NOT ENCOUNTERED: X	
С	QIA	RELATIVE	DESCRIPTION	REMARKS
	0	Loose	Fill: Red - Brown coarse to fine SAND, angular/sub-angular Shale, coarse to fine Gravel, trace Silt	
-	0	Medium		
-	0	Dense	Orange - Brown CLAY, some Sand, little Slit Brown coarse to fine SAND, little Slit	1
-				
				·
-			ENDOFFERN	·
<u> </u>		-	END OF TEST PIT 7.0' bg, SHALE BEDROCK ENCOUNTERED	
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TEST PIT NO.: AE-2
SHEET 1 OF 1
DATE: START: 5/12/08
END: 5/12/08

•		PROJEC			Thamesha Lake, NY	END: 5/12/08
• • •		JOB NO.			CLIENT: Cappelli .	O.G. ELEV.: 1496
•					Charlie Paternostro CONTRACTOR:	
· j		EQUIPM	-			
1		WATER	revet t	JEP1H:	NOT ENCOUNTERED: X	
1			_	l ≅≿	1	
	С		문	RELATIVE	DESCRIPTION	REMARKS
				25 C		, 1011 II (10
}			ō	Medium	Sill Province	
		1 1	Ů	Wicdians	Filt: Brown coarse to medium SAND and Silt, trace Clay, Fragments of Wood and Metal present.	•
discourse of the second						
54		<del></del>	0	Medium	· ·	
Cale Company Control			Ü	Medicali	Light Brown - Gray medium to fine SAND, some Clay, trace Silt	
						•
1						
Ī	5	<u> </u>	0	Medium	Comp. Dalat D	
.j			v	Mediani	Gray - Light Brown medium to fine SAND, trace Silt, fragments of angular/sub-angular Shale	•
.j			•			
:			0	Medium	Light - Dark Brown medium to fine SAND, some Clay,	
					trace Silt, large pieces of Shale encountered	
. 72	;					
Section of the sectio						
32	10			ļ	END OF EXCAVATION 10.0' bg SHALE BEDROCK ENCOUNTERED	
1					THE REPORT OF THE PROPERTY OF	
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	PROJECT NAM	IE: Concor	d LOCATION: Klamesha Lake, NY	END: 5/14/08
	JOB NO,: _73		CLIENT: Cappelli	O.G. ELEV.: 1494
	EQUIPMENT:			
	WATER LEVEL			
	TANTE IN CEASE	DEF 1M	NOT ENCOUNTERED: X	
		<u> </u>		•
С	吕	RELATIVE	PLEODICTION	
	"-		DESCRIPTION	REMARKS
			i .	
	0	Medium	Brown coarse to fine SAND, trace Clay,	
	, i		trace Sitt, fragments of angular/sub-angular Shale	
			angulai Offate	
		Medium		
•		iviedium	Brown - Red/Brown coarse to fine SAND, and Silt, angular/sub-angular Shale	•
			The state of the s	No vapors/staining encountered
			·	No env. soil sample taken
5	<del></del>	Dense	Pod Prove CHALE (	
3		Delige	Red - Brown SHALE, trace Silt	7
Ī				
Ī		<del> </del>	END OF TESTPIT 6.0' bg,SHALE BEDROCK ENCOUNTERED	
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TEST PIT NO.: AE-4
SHEET 1 OF 1
DATE: START: 5/14/08
END: 5/14/08

	PROJ	JECT NAME:	: Concord	d LOCATION: Kiamesha Lake, NY	END: 5/14/08
	JOB N	NO.: <u>7375</u>		CLIENT: Cappelli	O.G. ELEV.: 1486
				Charlie Paternostro CONTRACTOR:	
		PMENT: _		CONTRACTOR:	
		— ER LEVEL D		NOT ENCOUNTERED: X	
		т	<del>-</del>	TO ENOUGH ENED. A	•
_			RELATIVE DENSITY		
С		<b>6</b>	ENS	DESCRIPTION	REMARKS
		ĺ	유급		
			<u> </u>		
•		0	Loose	Fill: Light Brown - Brown, medium to fine SAND, little Silt, trace Clay	
	<del>  </del>	İ			
		·			
		0	Medium	Fill: Brown medium to fine SAND, little Silt, trace Clay, Misc, debris present (Transite and Metal pipe fragments, cinder blocks, glass and	-
		 	,	Construction and Demoiltion debris)	
			ļ,		
_		0	Medium	Fill: Light Brown - Gray coarse to fine SAND, little	+
5		ļ		Silt, trace Clay, fragments of asphalt and brick	
İ			] !		No vapors/staining encountered
		-			No env. soil sample taken
			Dense		
		ا	Delise	Gray – Reddish/Brown coarse to medium SAND, little Clay, trace Silt, angular to sub-angular Shale	1
[				Salar 10 oas angula Onalo	
	<del> </del>				
				END OF TEST PIT AT 9.0 ft-bgs, SHALE BEDROCK ENCOUNTERED	†
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### FIELD TEST PIT LOG

OF

	PROJEC JOB NO			d LOCATION: Klamesha Lake, NY	END: 5/14/08
				CLIENT: Cappelli ; Charile Paternostro CONTRACTOR:	O.G. ELEV.: 1490
	EQUIPM	IENT:			<u> </u>
	WATER	•	DEPTH:	NOTCHOOLING	
				NOT ENCOUNTERED: X	
c		Oil	RELATIVE DENSITY	DESCRIPTION	REMARKS
		0	Loose	Tan - Brown SAND, trace Slit	
			1	Tally Blown GARD, liace Sill	] <del>1</del>
			Loose	Brown SAND, little Clay, trace Silt, fragments of angular/sub-angular Shale	
				·	No vapors/staining encountered No environmental soll sample
					sample
5	<b> </b>				
		····	·	END OF TEST PIT AT 6.0 ftbgs, SHALE BEDROCK ENCOUNTERED	
				S.S. II. 593, OF IALE BEDROCK ENCOUNTERED	
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Concord

PROJECT NAME:

## FIELD TEST PIT LOG

TEST PIT NO .: AE-6 SHEET 1 OF 1 DATE: START: 5/14/08 END: 5/14/08

JOB NO.: 7375 CLIENT: Cappell INSPECTOR: Chris Mazur: Charle Paternostro CONTRACTOR: EQUIPMENT:  WATER LEVEL DEPTH: NOT ENCOUNTERED: X  C		PROJECT			- Trainesha Lake, NY	END: 5/14/08
INSPECTOR: EQUIPMENT: WATER LEVEL DEPTH: NOT ENCOUNTERED: _X  C DESCRIPTION REMARKS  DESCRIPTION REMARKS  O Loose File Brown coarse - medium SAND, little Clay, angular/sub-angular Rock  O Loose File Same, with fragments of black Rock  O Medium File Same, with fragments of wood and pipe  5 O Dense Same, with shalle angular/sub-angular Rock  O Dense Light Brown - Gray, little Clay, angular/sub-angular Rock  END OF TEST PIT 100° bg, SHALE BEDROCK ENCOUNTERED		JOB NO.:	7375		CLIENT: Cappelli	O.G. ELEV .: 1490
ECUIPMENT: WATER LEVEL DEPTH:  DESCRIPTION  REMARKS						
WATER LEVEL DEPTH:  NOT ENCOUNTERED:X						
DESCRIPTION  DESCRIPTION  DESCRIPTION  DESCRIPTION  DESCRIPTION  REMARKS  Description  Loose Fill: Brown coarse - medium SAND, little Clay, angular/sub-angular Rock  Description  Descript		WATER L	EVEL DE			
O Loose Fill: Same, with fragments of black Rock  O Medium Fill: Same, with fragments of wood and pipe  O Dense Same, with shale angular/sub-angular Rock  O Dense Light Brown - Gray, little Clay, angular/sub-angular Rock  END OF TEST PIT 10.0' bg, SHALE BEDROCK ENCOUNTERED	С		OIL.	RELATIVE DENSITY	DESCRIPTION	
O Dense Same, with shale angular/sub-angular Rock  O Dense Light Brown - Gray, little Clay, angular/sub-angular Rock  END OF TEST PIT 10.0' bg, SHALE BEDROCK ENCOUNTERED			0	Loose	Fill: Brown coarse - medium SAND, little Clay, angular/sub-angular Rock	
O Dense Same, with shale angular/sub-angular Rock  O Dense Light Brown - Gray, Ittle Clay, angular/sub-angular Rock  END OF TEST PIT 10.0' bg, SHALE BEDROCK ENCOUNTERED			Ō	Loose	Fill: Same, with fragments of black Rock	
Dense Same, with shale angular/sub-angular Rock  Dense Light Brown - Gray, little Clay, angular/sub-angular Rock  END OF TEST PIT 10.0' bg, SHALE BEDROCK ENCOUNTERED			0	Medium	Fill: Same, with fragments of wood and pipe	
O Dense Light Brown - Gray, Ittlle Clay, angular/sub-angular Rock  END OF TEST PIT 10.0' bg, SHALE BEDROCK ENCOUNTERED			İ			
O Dense Light Brown - Gray, Ittlle Clay, angular/sub-angular Rock  END OF TEST PIT 10.0' bg, SHALE BEDROCK ENCOUNTERED			İ			
O Dense Light Brown - Gray, Ittlle Clay, angular/sub-angular Rock  END OF TEST PIT 10.0' bg, SHALE BEDROCK ENCOUNTERED					,	
10 Dense Light Brown - Gray, little Clay, angular/sub-angular Rock  END OF TEST PIT 10.0' bg, SHALE BEDROCK ENCOUNTERED	٥					
10 Dense Light Brown - Gray, little Clay, angular/sub-angular Rock  END OF TEST PIT 10.0' bg, SHALE BEDROCK ENCOUNTERED				<del></del>		
END OF TEST PIT 10.0' bg, SHALE BEDROCK ENCOUNTERED  15			0	Dense	Same, with shale angular/sub-angular Rock	
END OF TEST PIT 10.0' bg, SHALE BEDROCK ENCOUNTERED  15	İ					
END OF TEST PIT 10.0' bg, SHALE BEDROCK ENCOUNTERED  15			0	Dense	Light Brown - Gray, little Clay, angular/sub-angular Rock	
END OF TEST PIT 10.0' bg, SHALE BEDROCK ENCOUNTERED  15	ļ					•
15	10	<del>-  </del>		<u>-</u>	END OF TEST PIT 10 0' bg SHALE PERPOSICE TO SHALE	
	-				OF PEOPLE BEDROCK ENCOUNTERED	
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TEST PIT NO.: UST-1
SHEET 1 OF 1
DATE: START: 5/5/08
END: 5/5/08

	PROJE	CT NAM	E: Concor	d LOCATION: Klamesha Lake, NY	DATE: START: 5/5/08
	JOB NO	).: <b>7</b> 37:	5	CLIENT: Cappelli	END: <u>5/5/08</u> O.G. ELEV.: + 1482
					11702
		MENT:		Charile Paternostro CONTRACTOR:	
	WATER	LEVEL	DEPTH:		
			·	NOT ENCOUNTERED:	
		_	RELATIVE DENSITY		
£		딥	TAT IS	DESCRIPTION	REMARKS
			%g		VENMINA
		0	Medium	Brown medium to fine Sand, some slity Clay, Trace Organics (tree	
				roots)	
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5			ł		
ļ		0	Medium	Brown coarse to fine SAND, some stilty Clay, Shale cobbles &	
ŀ				boulders	
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"				END OF TEST PIT @ 10' bg, SHALE BEDROCK ENCOUNTERED	
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PROJECT NAME: Concord

#### FIELD TEST PIT LOG

LOCATION: Kiamesha Lake, NY

TEST PIT NO.: UST-2

SHEET 1 OF 1

DATE: START: 5/5/08

END: 5/5/08

	JOB M	IO.: <u>7375</u>	<u> </u>	LIENT: Cappelli	O.G. ELEV.: <u>±1472</u>
				Charile Paternostro CONTRACTOR:	
	WATE	R LEVEL C		NOT ENCOUNTERED: X	
			<del> </del>		
			Ш	·	
С		PO	RELATIVE	DESCRIPTION .	
		<u></u>		BESCHIEFTION .	REMARKS
		0	Medium	Brown coarse to fine SAND, little silty Clay, Shale cobbles	
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-	-			END OF TESTPIT 7.5' bg, SHALE BEDROCK ENCOUNTERED	
Ì				THE DESCRIPTION OF THE PERIOD OF ENCOUNTERED	
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	TEST PIT NO.: UST-3
	SHEET 1 OF 1
	DATE: START: 5/5/08
	END: 5/5/08
i	O.G. ELEV .: +1472

	PROJECT NAME JOB NO.: 7375		d LOCATION; Klamesha Lake, NY  CLIENT: Cappelli	END: 5/5/08  O.G. ELEV.: +1472
	INSPECTOR:	Chris Mazur;	Charlie Paternostro CONTRACTOR:	11472
	EQUIPMENT: _			·
	WATER LEVEL D	EPIH:	NOT ENCOUNTERED: X	
C	PIO	RELATIVE	DESCRIPTION	REMARKS
	0	Medium	Brown coarse to fine SAND, little silty Clay, Shale cobbles	-
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}		ı		
0 -	0			
-			END OF TEST PIT 10' bg, SHALE BEDROCK ENCOUNTERED	
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TEST PIT NO.: UST-4
SHEET 1 OF 1
DATE: START: 5/5/08
END: 5/5/08

	PRO	JECT NAME	E: Conco	rđ	LOCATION: Kiar		DATE: START: 5/5/08
				CLIENT: Cappelli	LOOATION: _ Klar	mesna Lake, NY	END: <u>5/5/08</u> O.G. ELEV.: <u>+ 1488</u>
				r; Charlie Paternostro	001/701		576. EEE V 1 1400
	EQU!	PMENT:		y Grasiia i dictriosito	CONTRACTOR	Ŗ:	
				Seepage 6.0'			
				ocepage o.o	NOT ENCOU	NTERED:	
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C	1	윤	RELATIVE		DESCRIPTION		
		}	~ ~ ~				REMARKS
			<u>l</u>				
		0	Loose	Brown - coarse to t	fine SAND, little slity clay,	Shale cobbles	
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5		l					
İ		2.4 ppm	ł				0
l							Gray-Slight staining of soil, PID reading taken, Slight odor
							1 12 100 tang taken, Silgitt odor
				END OF TEST PIT	@ 8', SHALE BEDROCK	FNCOUNTERED	
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TEST PIT NO.: UST-5
SHEET 1 OF 1
DATE: START: 5/5/08
END: 5/5/08
O.G. ELEV.: +1494

	PROJ	JECT NAME;	Concord	LOCATION: Kiamesha Lake, NY	END: 5/5/08
	JOB 1	NO.: <u>7375</u>	(	CLIENT: Cappelli	O.G. ELEV.: + 1494
				Charlie Paternostro CONTRACTOR:	
	EQUI	PMENT: _			
				NOT ENCOUNTERED:	
		·	<del></del>		
		!			
			RELATIVE DENSITY		
C		딦	I A S	DESCRIPTION	REMARKS
			25		
		0	MEDIUM	Brown coarse to fine SAND, some silty clay, little Shale gravel	
		{			
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		0.6ppm			
		, ,			Odor detected @ 3.0' bg
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-				END OF TEST PIT 6.5' bg, BEDROCK ENCOUNTERED	-
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## FIELD TEST PIT LOG

TEST PIT NO .: \_UST-6 SHEET 1 OF 1 DATE: START: 5/6/08 END: 5/6/08

	PROJ	JECT NAME	Concord	LOCATION: Klamesha Lake, NY	END: 5/6/08
	JOB 1	NO.: <u>7375</u>		CLIENT: Cappell	O.G. ELEV.: + 1499
				Charlie Paternostro CONTRACTOR:	· · · · · · · · · · · · · · · · · · ·
				SONTACION.	
				.5" NOT ENCOUNTERED:	
			i		
			RELATIVE DENSITY		
€		집	LAT	DESCRIPTION	REMARKS
		0	Medium	Brown-coarse to fine SAND, trace silty clay, some Shale Cobbles &	
	<u></u>	_		Boulders	
		<b>S</b>		·	•
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i				·	
				END OF TESTPIT 8.0', SHALE BEDROCK ENCOUNTERED	
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# BORING LOGS FROM SUBSURFACE INVESTIGATION 6/13/08 - 7/15/08

W7 QEQI				P	ROJECT	NAME:	CONCORD BORING NO.											
					<del></del>		CATION:					JOB NO.	100.410				SB-1	
		ONBULT Engine	TING ERS		1											5485		
BORIN	IG BY: G	BI			D.	ATE ST	ARTED:	6/13/2008 GF			GPOL	GROUND ELEVATION:				1482 <u>+</u>		
INSPE	CTOR: J	N				COMP			/2008	GROUNDWATER TABLE DEPTH: NI  O Hr. N/A Date N/A 24 Hr. N/A								
DEPTH		041451.5	DE	PTH					-		Date	IVA	44 M.	N/A	Date	ate N/A		
(ft)	METHOD	SAMPLE No.	FROM	то	1	Diows o	n Spooi	1	REC		SOIL	DESCRI	PTION AN	ID STRA	TIEICATI	ON	٦	
0		,,,,	(ft)	(ft)	0/6	6/12	12/18	18/24	(in)			-200, (			HOATI	JN	S	YMBOL
İ	\$3	1	0	2	4	8	7	7		3" Tops	soil			<del></del>			- -	
				]								line SAN	ID, little Si	lt teres Co	nral		  -	
1										1			,	is agor at	4401		⊢	
5																	-	
1	SS	2	5	7	44	54	75	69	22	Red-Br	own SILT	, some co	parse Grave	el. little cos	rse to fine	Sand .	┪-	
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																	-	
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10	<del></del>				<u> </u>												F	
	\$S	3	10	12	14	75/2"			8	Brown	medium to	fine SA	ND, trace	Silt (Weath	ered Rocl	<)	_	
				-	<del> </del>				1.0m	ROCK	CORE: RI	UN 1 (11	'-1 <i>6</i> ')		-			
				<u> </u>							/ERY = 5:		= 91.7 %				一	
15											20 / 60" ==							
-					ļ				1.0m	Gray co	arse grain	ed, slight	tly weather	ed, strong :	SANDST	ONE	_ _	
											ly spaced					<del></del>		
ŀ			_								CORE: RI							
. <b>†</b>											BRY = 5:		=91.7%					
20											30" / 60" =		• • • • • • • • • • • • • • • • • • • •				L	
									1.25m	Mear to	wn rine gr	rained, si	ightly to hi	ghtly weati	hered, ver	У	_	
					$\neg \neg$				1.2.311	BUB IN	G COMBI	Brong St.	IALE w/ cl	osely space	ed fracture	s	- -	
										DOM	G COMIFI	-615 V I	21.0 FEE	ľ			-	
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Nominal I.D. of Hole	in	The subsurface information shown hereon was obtained for the design and estimating purposes for our client.
Nominal I.D. of Split Barrel Sampler	1% in	It is made available to authorized users only that they may have access to the same information available
Weight/type of Hammer on Drive Pipe	300 lb	to our client. It is presented in good faith, but it is not intended as a substitute for investigations, interpretations
Weight/type of Hammer on Split Barrel	140 lb	or judgment of such authorized users. Information on the logs should not be relied upon without the geotechnical
Drop of Hammer on Drive Pipe	in	engineers recommendations contained in the report from which these logs were extracted.
Core Size	in	Pp: Pocket Penetrometer; WOH: Weight of Hammer; WOR: Weight of Rod
		Approximate Change in Strata: Inferred Change in Strata:

					т			5485 BORING LOG S								
					PI		NAME:			CONCORD BORING NO. SB-2	SB-2					
		ONBULT				LOC	CATION:		M	onticello, NY JOB NO. 5485						
		ENGINE	ER8		ļ					GROUND ELEVATION: 1489±						
	NG BY: G						ARTED:		/2008	GROUNDWATER TABLE DEPTH: NE						
DEPTH	CTOR: J	N	<del></del>		DATE	COMP	LETED:	6/13	/2008	0 Hr. N/A Date N/A 24 Hr. N/A Date N/A						
1		SAMPLE		PTH	4	Blows	on Spoor	1	REC	1000 1111						
(ft)	METHOD	No.	FROM		<u> </u>	.,				SOIL DESCRIPTION AND STRATIFICATION SY	MBOL					
0	<del></del>	<u> </u>	(ft)	(ft)	0/6	6/12	12/18	18/24	(in)							
	88	1	0	2	6	8	6	8	10	3" Topsoil & Roots						
				<u> </u>	.	<u> </u>	ļ			Brown coarse to fine SAND, little Silt, little coarst to medium Gravel						
		<u> </u>		<u> </u>	L					<u> </u>						
i _	ļ					<u> </u>			L							
5		<u></u>								·						
	55	2	5	7	33	75/5"			10	Red-Brown SILT, some coarse Gravel, little coarse to fine Sand						
				<u>L</u> _						diaron, natic coarse to fine Sand	_					
										Refusal on Rock						
1				ļ	<u> </u>					BORING COMPLETE AT 6.5 FEBT						
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Nominal I.D. of Split Barrel Sampler	1% ir	The subsurface information shown hereon was obtained for the design and estimating purposes for our client.
	300 iŁ	It is made available to authorized users only that they may have access to the same information available to our client. It is presented in good faith, but it is not intended as a substitute for investigations, interpretations
Weight/type of Hammer on Split Barrel	140 lb	or judgment of such authorized users. Information of the land to the state of the such authorized users. Information of the land to the such authorized users.
Drop of Hammer on Drive Pipe	in	or judgment of such authorized users. Information on the logs should not be relied upon without the geotechnical engineers recommendations contained in the report from which these logs were extracted.
Core Size .	in	Pp: Pocket Penetrometer; WOH: Weight of Hammer; WOR: Weight of Rod
Soil descriptions represent a field identification	after D	Approximate Change in Strata: Inferred Change in Strata:

188	TET 7 42	-			Т -				5485 BORING LOG SB-3.xl	ds	
	Z (C				P		NAME:			CONCORD BORING NO. SB:	3
		ONSULT	ING			LOC	CATION:		M	onticello, NY JOB NO. 5485	
2001		SHOINE	8 N S		<u> </u>					GROUND ELEVATION: 1493	
	NG BY: G						ARTED:	6/13	/2008	GROUNDWATER TABLE DEPTH: NE	
	CTOR: J	N/JZ	· ·		DATE	COMP	LETED:	6/13	/2008	0 Hr. N/A Date N/A 24 Hr. N/A Date N/A	
DEPTH	j	SAMPLE		PTH		Blows	п Ѕрооп		REC		
(ft)	METHOD	No.	FROM	то	ļ				1,120	SOIL DESCRIPTION AND STRATIFICATION s	SYMBOL
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	ss	1	0	2	8	6	9	8	16	2" Topsoil & Roots	
-				<u> </u>						Red-brown m-f SAND, some Silt, trace Gravel	
					<u>L</u> .					-	
5	<u> </u>	ļi								1	
	SS	2	5	7	27	75/4 <sup>11</sup>		-	10	Red-brown SILT, little m-f Sand, little c-f Gravel w/ fractured	
										Sand stone, w/ Sand lenses	
										-	·
				L						Fractured Rock (Cobbles)	
10											
	SS	3	10	12	41	38	31	31	12	Same	
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									l min	ROCK CORE: RUN 1 (14.5'-19.5')	
									1 min	RECOVERY = 60" / 60" = 100 %	
1 [										RQD = 49" / 60" = 81.6%	
1 L						-				Gray coarse grained, slightly weathered, strong SANDSTONE	
20									2 min	w/ closely spaced fractures	
[										ROCK CORE: RUN 1 (19.5'-24.5')	
] [						-				RECOVERY = 60" / 60" = 100%	
] [				"						RQD = 36" / 60" = 60%	
[							<del></del>  -	<del>-  </del>			
25	-								2 min	Gray coarse grained, slightly weathered, strong SANDSTONE w/ closely spaced fractures	
						~		$\neg \dashv$	Z IIIII	BORING COMPLETE AT 24.5 FEBT	
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Nominal I.D. of Split Barrel Sampler	1% in	It is made available to authorized users only that they may have access to the same information available
Weight/type of Hammer on Drive Pipe	300 lb	to our client. It is presented in good faith, but it is not intended as a substitute for investigations, interpretations
Weight/type of Hammer on Split Barrel	140 lb	or judgment of such authorized users. Information on the logs should not be relied upon without the geoteclinical
Drop of Hammer on Drive Pipe	ln	engineers recommendations contained in the report from which these logs were extracted.
Core Size	חנ	Pp: Pocket Penetrometer; WOH: Weight of Hammer; WOR: Weight of Rod
		Approximate Change in Strata: Inferred Change in Strata:

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			5		P		NAME:			CONCC				BORING NO. SB-4				
			TING			LOC	CATION:		M	onticell	o, NY		JOB NO			5485		
BOBI	NG BY: G	ENGINE DI	ERS	<del></del>	<del>  _</del>	•		<del> </del> -						ND ELEVA		14	96±	
	ECTOR: J						ARTED:		/2008	<u> </u>	·	GROL	JNDWATER TABLE DEPTH: NE			IE,		
DEPTH		<u> </u>	T 55	DTI	DATE	COMP	LETED:	6/13	/2008	0 Hr.	N/A	Date	N/A	24 Hr.	N/A	Date N	'A	
	METHOD	SAMPLE	:	PTH	4	Blows o	on Spoor	n	REC							<u></u>	Ī	
(ft)	WEIHOD	No.	FROM		<del> </del>					]	SOILD	ESCRI	PTION A	ND STRAT	(IFICATION	1	SYMBOL	
0	<del> </del>	ļ	(ft)	(ft)	0/6	6/12	12/18	18/24	(in)									
	53	1	0	2	3	5	6	11	24	Red-bi	rown c-f SA	ND, so	me c-f Gr	avel, little Si			<del> </del> -	
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	ss	2	5	7	12	50/3"	-	_	9	Same é	(rock in tip)	ı					<del> </del> _	
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lominal I.D. of Split Barrel Sampler	1% in	The subsurface information shown hereon was obtained for the design and estimating purposes for our client.  It is made available to authorized users only that they may have access to the same information available
Velght/lype of Hammer on Drive Pipe	300 lb	to our client. It is presented in good faith, but it is not intended as a substitute for investigations, interpretations
Velght/type of Hammer on Split Barrel	140 lb	or judgment of such authorized users. Information on the logs should not be relied upon without the geotechnical
Prop of Hammer on Drive Pipe	in	engineers recommendations contained in the report from which these logs were extracted.
ore Size	ln	Pp: Pocket Penetrometer; WOH: Weight of Hammer; WOR: Weight of Rod
oil descriptions represent a field identification	after D	Approximate Change in Strata: Inferred Change in Strata:

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		·					CATION			onticello, NY	JOB NO.	10.			SB-5				
		ENGINE	ERS	_							GROUND	ELEVAT	ios.		5485				
	NG BY: G				D.	ATE ST	ARTED	6/13	/2008	GROL	INDWATER	TABLE	DEDTU		1496 <u>+</u>				
	CTOR: J	Z					LETED		/2008	0 Hr. N/A Date		24 Hr.							
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(ft)	METHOD	No.	FROM	TO	7	DIOWS (	on Spoo	n	REC	SOIL DESCRI	CRIPTION AND STRATIFICATION								
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									<u> </u>	6" Concrete & Rebar / 6"	Graval		·						
]										Weathered Rock	Olavel								
İ						1.				TO SEED TO SEE T					ļ				
l										ROCK CORE: RUN 1 (2.	51 7 5N				-				
5										RECOVERY = 55"/ 60" =									
							1			RQD = 0"/60" = 0%	- 51.0/0								
										Gray medium to fine grain	and cliabely a	**************************************	31		<u> </u>				
							Ţ		realitered	, meaium									
						strong SANDSTONE w/ very closely spaced fractures  ROCK CORE: RUN 2 (7.5'-12.5')													
10										RECOVERY = 51"/ 60" =					ļ				
										RQD = 0"/ 60" = 0%	0070			-					
				<u> </u>	<u></u>					Gray medium to fine grain	ed, slightly v	veathered	medium		<u> </u>				
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Nominal I.D. of Split Barrel Sampler	1% in	The subsurface information shown hereon was obtained for the design and estimating purposes for our client.  It is made available to authorized users only that they may have access to the same information available
Welght/type of Hammer on Drive Pipe	300 lp	Ito our client. It is presented in good faith, but is in and interval.
Weight/type of Hammer on Split Barrel	140 lb	to our client. It is presented in good faith, but it is not intended as a substitute for investigations, interpretations or judgment of such authorized users. Information on the logs should not be relied upon without the geotechnical
Drop of Hammer on Drive Pipe	In	engineers recommendations contained in the report from which these logs were extracted.
Core Size	l in	Pp: Pocket Penetrometer; WOH: Weight of Hammer; WOR: Weight of Rod
Soil descriptions represent a field identification	n after D	Approximate Change in Strata: Inferred Change in Strate:

Approximate Change in Strata:

Soil descriptions represent a field identification after D. M. Burmister unless otherwise noted.

FIGURE 6

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		SE					CATION	<del></del>		onticello							B-6
1.4		<b>ЕИФІИБ</b> ) ОИВПГ.						]	141	0111100110	F 14.1		JOB NO.				485
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INSP	CTOR: J	<u>z</u>		**			LETED			0 Hr.	N/A	Date			DEPTH: N		
DEPTI	ı		DE	PTH	<del>                                     </del>				T	101111	IVA	Date	N/A	24 Hr.	N/A	Date N/	Ά
(ft)	METHOD	SAMPLE No.	FROM	то	7	Blows	on Spoo	n	REC		SOIL	DESCO	DTION A	IO OTO A	TIFICATION		
0		140.	(ft)	(ft)	0/6	6/12	12/18	18/24	(in)	┪	OOIL	DESCIN	IL HON WI	ND 51RA	HECATION		SYMBO
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Nominal I.D. of Hole	in	The subsurface information shown hereon was obtained for the design and estimating purposes for our client.
Nominal I.D. of Split Barrel Sampler	1% in	It is made available to authorized users only that they may have access to the same information available
Welght/type of Hammer on Drive Pipe	300 lb	to our client. It is presented in good faith, but it is not intended as a substitute for investigations, interpretations
Weight/type of Hammer on Split Barrel	140 lb	or judgment of such authorized users. Information on the logs should not be relied upon without the geotechnica
Drop of Hammer on Drive Pipe	in	engineers recommendations contained in the report from which these logs were extracted.
Core Size	in	Pp: Pocket Penetrometer; WOH: Weight of Hammer; WOR: Weight of Rod
Soil descriptions represent a field identification	n after D	Approximate Change in Strata: Inferred Change in Strata: M. Burmister unless otherwise noted.
		FIGURE 7 Page 1 of 1

PROJECT NAME	<b>ASS</b>	777 0		□ □					5485 BORING LOG									LOG SB-	7.xis
DORNO BY CRIT   No.   DATE STARTED   STARTED	1 (2				Pi			_					BORING NO.					B-7	
DORING BUT   STATE				FING			LO	CATION:		M	onticello, NY	<b>′</b>		JOB NO.					
DATE STARTED:   e1/18/2006   OHK.   NA   Date   DATE   D	DODU			ERŞ		<del> </del>								GROUN	D ELEVA	ATION:			
DATE COMPLETED:   0162000   016   017   016									6/16/2008 GF										
SAMPLE   Flower   Section   Sectio			Z	T		DATE	COMP	LETED:	6/16	/2008	0 Hr. N	/A	Date	N/A					
Column   From   To   From			SAMPLE			-[	Blows o	n Spoor	ì				2010 14	<del>^ -</del>					
10		WEIHOD		FROM	_	<u> </u>					j so	OIL DE	SCRI	PTION A	ND STRA	TIFICA	TION		SYMBOL
10	-0	<u> </u>			(ft)	0/6	6/12	12/18	18/24	(ln)	L								
Roots   Root		SS	1	0	2	4	7	8	13	18	Brown/Red-	-brown	c-fSA?	ND, little	c-f Gravel	little Si	li trace		<del> </del>
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88 3 10 12 100/2*											1								
2 min   ROCK CORE: RUNI (12'-17')   RECOVERY = 60" / 60" = 100%   1 min   RQD = 0" / 60" = 90%   1 min   2 min   2 min   3 min   2 min   4 min   2 min   4 min   2 min   4 min   2 min   4 min   2 min   4 min   2 min   4 min	10										1								<b> </b> -
2 min ROCK CORE: RUNI (12'-17) 2 min RECOVERY = 60" / 60" = 100%  1 min RQD = 0" / 60" = 100% RQD = 0" / 60" =	1 1	SS	3	10	12	100/3 <sup>n</sup>	-	-	-	2"	Rock in tin							_	
1 min   RECOVERY = 60" / 60" = 100%     1 min   RQD = 0" / 60" = 0%     2 min   1 min   Gray medium to fine grained, slightly weathered, medium     2 min   1 min   BORING COMPLETE AT 17.0 FHET     20							Ö				) 								
1 min   RECOVERY = 60" / 60" = 100%     1 min   RQD = 0" / 60" = 0%     2 min   1 min   Gray medium to fine grained, slightly weathered, medium     2 min   1 min   BORING COMPLETE AT 17.0 FHET     20							-			2 min	ROCK COR	E- RIA	J1 (12)	171\					
1 min   RQD = 0" / 60" = 0%     1 min   Gray medium to fine grained, elightly weathered, medium     2 min   strong SANDSTONE w/ very closely spaced fractures     BORING COMPLETE AT 17.0 FHET     20	1 ]									2 min	RECOVERY	V = KD*	ላ፤ (ፈራ ረ ճሲ። <del>-</del> -	- 1//					
20   1 min   Gray medium to fine grained, stightly weathered, medium   2 min   strong SANDSTONE w/ very closely spaced fractures   BORING COMPLETE AT 17.0 FHET   20   20   20   20   20   20   20   2	15									1 min	ROD = 0" / 6	. – 00 60" <del></del> 00	/ UU - 3/.	100%					
20   BORING COMPLETE AT 17.0 FIET   20   STORY COMPLETE AT 17.0 FIET   25   STORY COMPLETE AT 17.0 FIET   30   STORY COMPLETE AT 17.0 FIET   31   STORY COMPLETE AT 17.0 FIET   32   STORY COMPLETE AT 17.0 FIET   33   STORY COMPLETE AT 17.0 FIET   34   STORY COMPLETE AT 17.0 FIET   35   STORY COMPLETE AT 17.0 FIET   36   STORY COMPLETE AT 17.0 FIET   36   STORY COMPLETE AT 17.0 FIET   37   STORY COMPLETE AT 17.0 FIET   38   STORY COMPLETE AT 17.0 FIET   39   STORY COMPLETE AT 17.0 FIET   30   STORY COMPLETE AT 17.0 FIET   30   STORY COMPLETE AT 17.0 FIET   31   STORY COMPLETE AT 17.0 FIET   32   STORY COMPLETE AT 17.0 FIET   33   STORY COMPLETE AT 17.0 FIET   34   STORY COMPLETE AT 17.0 FIET   35   STORY COMPLETE AT 17.0 FIET   36   STORY COMPLETE AT 17.0 FIET   37   STORY COMPLETE AT 17.0 FIET   38   STORY COMPLETE AT 17.0 FIET   39   STORY COMPLETE AT 17.0 FIET   30   STORY COMPLETE AT 17.0 FIET   30   STORY COMPLETE AT 17.0 FIET   30   STORY COMPLETE AT 17.0 FIET   31   STORY COMPLETE AT 17.0 FIET   32   STORY COMPLETE AT 17.0 FIET   33   STORY COMPLETE AT 17.0 FIET   34   STORY COMPLETE AT 17.0 FIET   35   STORY COMPLETE AT 17.0 FIET   36   STORY COMPLETE AT 17.0 FIET   37   STORY COMPLETE AT 17.0 FIET   38   STORY COMPLETE AT 17.0 FIET   39   STORY COMPLETE AT 17.0 FIET   30   STORY COMPLETE AT 17.0 FIET   30   STORY COMPLETE AT 17.0 FIET   30   STORY COMPLETE AT 17.0 FIET   31   STORY COMPLETE AT 17.0 FIET   31   STORY COMPLETE AT 17.0 FIET   31   STORY COMPLETE AT 17.0 FIET   39   STORY COMPLETE AT 17.0 FIET   30   STORY COMPLETE AT 17.0 FIET   31   STORY COMPLETE AT 17.0 FIET   31   STORY COMPLETE AT 17.0 FIET   31   STORY COMPLETE AT 17.0 FIET   32   STORY COMPLETE AT 17.0 FIET   33   STORY COMPLETE AT 17.0 FIET   34   STORY COMPLETE AT 17.0 FIET   35   STORY COMPLETE AT 17.0 FIET   35   STORY COMPLETE AT 17.0 FIET   35   STORY COMPLETE AT 17.0 FIET   36   STORY COMPLETE AT 17.0 FIET   37   STORY COMPLETE AT 17.0 FIET   37   STORY COMPLETE AT 17.0 FIET   37   STORY COMPLETE AT 17.0 FIET   37   STORY COMP	l J													od olimbal	41			_	
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Nominal I.D. of Hole	ir	The subsurface information shown horses was abtained 5
Nominal I.D. of Split Barrel Sampler	1% in	The subsurface information shown hereon was obtained for the design and estimating purposes for our client.
Weight/type of Hammer on Drive Pipe	300 lb	It is made available to authorized users only that they may have access to the same information available
Weight/type of Hammer on Split Barrel	140 lb	to our client. It is presented in good faith, but it is not intended as a substitute for investigations, interpretations
Drop of Hammer on Drive Pipe	În	or judgment of such authorized users. Information on the logs should not be relied upon without the geotechnical engineers recommendations contained in the report from which these logs were extracted.
Core Size	in	Pp: Pocket Penetrometer; WOH: Weight of Hammer; WOR: Weight of Rod
		Approximate Change in Strata: Informed Change in Strata:

150	7.7.7 /2		- P		т					5485 BORING LOG SB-8	).xis
	SES ODM SULTING ENGINEERS				P		NAME:				B-8
Canal Canal			ING		1	LOC	CATION:		М	onticello, NY JOB NO. 54	85
BOD!			ERS		<u> </u>					GROUND ELEVATION: 140	86 <u>+</u>
	NG BY: G	_	<del></del>		T		ARTED:		2008	GROUNDWATER TABLE DEPTH: Seepage @ 11'	<del></del>
DEPTH		<u> </u>	_ n=	PTH	DATI	COMP	LETED:	8/16	/2008	0 Hr. 14.5' Date 6/16/08 24 Hr. N/A Date N/A	A
(ft)	METHOD	SAMPLE	FROM		-	Blows o	n Spoor	1	REC		
0	111211100	No.	(ft)		0/0	0440	Louis			SOIL DESCRIPTION AND STRATIFICATION	SYMBOL.
<u> </u>	- 22	1	0	(ft) 2	0/6		12/18	18/24	(in)		
İ	33			1-	14	14	20	22	24	Fill: Red-brown c-f SAND, some c-f Gravel, little Silt	
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1		- <u>-</u> -		<del>+ ′-</del>	2.5	32	49	58	24	Red-brown c-f SAND, some c-f Gravel, little Silt, w/ Cobbles	
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1	SS	3	10	12	27	18	24	15	24		
			·		<del></del>	1.5	27		24	Orange-brown/Red-brown/Gray c-f SAND, some c-f Gravel	
										little Silt w/ Cobbles	
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15											
	SS	4	15	17	12	22	20	41	24	Red-brown c-f SAND, some c-f Gravel, little Silt	
										(Rock in tip)	
										(Cook III (II))	
20										ļ	
	22	5	20	22	19	50/2"	-	-	8	Red-brown c-m SAND and c-f Gravel, little Silt w/ Cobbles	
1 1										Refusal on Rock	
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Nominal I.D. of Hole	in	The subsurface information shown bereau was aftering to
Nominal I.D. of Split Barrel Sampler	1% in	The subsurface information shown hereon was obtained for the design and estimating purposes for our client.  It is made available to authorized users only that they may have access to the same information available
Weight/type of Hammer on Drive Pipe	300 lb	to our client. It is presented in good faith but it is not intended access to the same information available
	140 lb	to our client. It is presented in good faith, but it is not intended as a substitute for investigations, interpretations or judgment of such authorized users. Information on the logs should not be relied upon without the geotechnical
Drop of Hammer on Drive Pipe	in	engineers recommendations contained in the report from which these logs were extracted.
Core Size	In	Pp: Pocket Penetrometer; WOH: Weight of Hammer; WOR: Weight of Rod
		Approximate Change in Strata: Inferred Change in Strata:

100	777 0		- n		<del>                                     </del>			<u> </u>		5485 BORING LOG SB-S	ek.S
					P		NAME:			CONCORD BORING NO. SI	B-9
-		CHBUL	TING			LO	CATION:		M	opticallo NV	185
BOOK	NG BY: G	ENGINE	ERS		<del> </del>					ODOLUM TO THE TOTAL TOTAL TOTAL TOTAL TOTAL TOTAL TOTAL TOTAL TOTAL TOTAL TOTAL TOTAL TOTAL TOTAL TOTAL TOTAL TOTAL TOTAL TOTAL TOTAL TOTAL TOTAL TOTAL TOTAL TOTAL TOTAL TOTAL TOTAL TOTAL TOTAL TOTAL TOTAL TOTAL TOTAL TOTAL TOTAL TOTAL TOTAL TOTAL TOTAL TOTAL TOTAL TOTAL TOTAL TOTAL TOTAL TOTAL TOTAL TOTAL TOTAL TOTAL TOTAL TOTAL TOTAL TOTAL TOTAL TOTAL TOTAL TOTAL TOTAL TOTAL TOTAL TOTAL TOTAL TOTAL TOTAL TOTAL TOTAL TOTAL TOTAL TOTAL TOTAL TOTAL TOTAL TOTAL TOTAL TOTAL TOTAL TOTAL TOTAL TOTAL TOTAL TOTAL TOTAL TOTAL TOTAL TOTAL TOTAL TOTAL TOTAL TOTAL TOTAL TOTAL TOTAL TOTAL TOTAL TOTAL TOTAL TOTAL TOTAL TOTAL TOTAL TOTAL TOTAL TOTAL TOTAL TOTAL TOTAL TOTAL TOTAL TOTAL TOTAL TOTAL TOTAL TOTAL TOTAL TOTAL TOTAL TOTAL TOTAL TOTAL TOTAL TOTAL TOTAL TOTAL TOTAL TOTAL TOTAL TOTAL TOTAL TOTAL TOTAL TOTAL TOTAL TOTAL TOTAL TOTAL TOTAL TOTAL TOTAL TOTAL TOTAL TOTAL TOTAL TOTAL TOTAL TOTAL TOTAL TOTAL TOTAL TOTAL TOTAL TOTAL TOTAL TOTAL TOTAL TOTAL TOTAL TOTAL TOTAL TOTAL TOTAL TOTAL TOTAL TOTAL TOTAL TOTAL TOTAL TOTAL TOTAL TOTAL TOTAL TOTAL TOTAL TOTAL TOTAL TOTAL TOTAL TOTAL TOTAL TOTAL TOTAL TOTAL TOTAL TOTAL TOTAL TOTAL TOTAL TOTAL TOTAL TOTAL TOTAL TOTAL TOTAL TOTAL TOTAL TOTAL TOTAL TOTAL TOTAL TOTAL TOTAL TOTAL TOTAL TOTAL TOTAL TOTAL TOTAL TOTAL TOTAL TOTAL TOTAL TOTAL TOTAL TOTAL TOTAL TOTAL TOTAL TOTAL TOTAL TOTAL TOTAL TOTAL TOTAL TOTAL TOTAL TOTAL TOTAL TOTAL TOTAL TOTAL TOTAL TOTAL TOTAL TOTAL TOTAL TOTAL TOTAL TOTAL TOTAL TOTAL TOTAL TOTAL TOTAL TOTAL TOTAL TOTAL TOTAL TOTAL TOTAL TOTAL TOTAL TOTAL TOTAL TOTAL TOTAL TOTAL TOTAL TOTAL TOTAL TOTAL TOTAL TOTAL TOTAL TOTAL TOTAL TOTAL TOTAL TOTAL TOTAL TOTAL TOTAL TOTAL TOTAL TOTAL TOTAL TOTAL TOTAL TOTAL TOTAL TOTAL TOTAL TOTAL TOTAL TOTAL TOTAL TOTAL TOTAL TOTAL TOTAL TOTAL TOTAL TOTAL TOTAL TOTAL TOTAL TOTAL TOTAL TOTAL TOTAL TOTAL TOTAL TOTAL TOTAL TOTAL TOTAL TOTAL TOTAL TOTAL TOTAL TOTAL TOTAL TOTAL TOTAL TOTAL TOTAL TOTAL TOTAL TOTAL TOTAL TOTAL TOTAL TOTAL TOTAL TOTAL TOTAL TOTAL TOTAL TOTAL TOTAL TOTAL TOTAL TOTAL TOTAL TOTAL TOTAL TOTAL TOTAL TOTA	82 <u>+</u>
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DEPTH		<u> </u>	D⊭	PTH	DATE	= COMP	LETED:	6/18	/2008	O Hr. N/A Date N/A 24 Hr. N/A Date N/A	A
(ft)	METHOD	SAMPLE	FROM		4	Blows on Spoon			REC		T
0		No.	(ft)	(ft)	0/6	0//0	Tions	1.00		SOIL DESCRIPTION AND STRATIFICATION	SYMBOL
	SS	1	0	2	18	6/12	12/18	18/24	(in)		
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_	ss	2	- 5	7	43	50/21		_			
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10					<del> </del>					Boulder (hard drilling from 8' to 9')	
	SS	3	10	12	58	37	33	38			
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Nominal I.D. of Hole	ir	The subsurface information above to
Nominal I.D. of Split Barrel Sampler	1% in	The subsurface information shown hereon was obtained for the design and estimating purposes for our client.
141 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1	300 lb	It is made available to authorized users only that they may have access to the same information available
Weight/type of Hammer on Spilt Barrel	140 lb	to our client. It is presented in good faith, but it is not intended as a substitute for investigations, interpretations
Drop of Hammer on Drive Pipe	ln	or judgment of such authorized users. Information on the logs should not be relied upon without the geotechnical
Core Size	in	engineers recommendations contained in the report from which these logs were extracted.  Pp: Pocket Penetrometer; WOH: Weight of Hammer; WOR: Weight of Rod
Soil descriptions represent a field identification	after D.	Approximate Change in Strata:

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		SE.		P	ROJECT				CONCORD BORING NO. SB-1	0			
		ONEUL	TING			LOC	CATION	<b> </b> -	M	onticello, NY JOB NO. 5485			
BORI	NG BY: G	ENGINE	ERØ		<del></del>			ļ		GROUND ELEVATION: 1478			
	ECTOR;					ATE ST		6/16/2008		GROUNDWATER TABLE DEPTH: 10'	<del>-</del>		
DEPTH		1		OTIL	DAT	DATE COMPLETED:			/2008	0 Hr. 10' Date 6/16/08 24 Hr. N/A Date N/A	N/A		
(ft)	METHOD	SAMPLE	<del></del>	EPTH	4	Blows	n Spoot	1	REC	100 415			
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İ	SS	1	0	2	16	27	30	26	24	Fill: Red-brown c-f SAND, some c-f Gravel, little Silt			
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15							-			Hard drilling (weathered rock/cobbles)			
	SS	4	15	17	39	36	45	53	15	C			
] [									13	Gray c-m SAND and c-f Gravel, little Silt w/ fractured Rock			
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	SS	5	20	22	21	50/1"			6				
[				$\vdash$				<del>-</del>		Same (rock in tip)			
								-+		Refusal on Rock			
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Nominal I.D. of Hole	ir	The subsurface information shows because were about 5 and 5.
Nominal I.D. of Split Barrel Sampler	1% ir	The subsurface information shown hereon was obtained for the design and estimating purposes for our client.
Weight/type of Hammer on Drive Pipe	300 lb	It is made available to authorized users only that they may have access to the same information available to our client. It is presented in good faith, but it is not intended as a substitute for investigations, interpretations
Weight/type of Hammer on Split Barrel	140 Jb	or judgment of such authorized users. Information on the logs should not be relied upon without the geotechnical
Drop of Hammer on Drive Pipe	in	engineers recommendations contained in the report from which these logs were extracted.
Core Size	11.1	Pp: Pocket Penetrometer; WOH: Weight of Hammer; WOR: Weight of Rod
		Approximate Change in Strata: Inferred Change in Strata:

ANT CECI					PF	ROJECT	NAME:	CONCORD					BORING NO. SB			- 11.03
		グニッ					CATION:			_	lo, NY	<del></del> -	JOB NO			SB-11
4		ENGINE ONENT.	TING Ens											D ELEVATION:		5485
	NG BY: G				D/	ATE ST	ARTED:	7/15	/2008			GROUN			7.011	1498 <u>+</u>
	CTOR: C	DM	<u> </u>				LETED:		/2008	0 Hr.	N/A	Date	DVVA	R TABLE DEPTH:		
DEPTH		SAMPLE	DE	ЭΤΗ		Blows on Spoor						Duto	IVA	24 Hr. N/A	Date	N/A
(ft)	METHOD	No.	FROM	то	1	DIOWS	n Spoor	1	REC		SOIL	DESCRI	ΡΤΙΟΝ Δ	ND STRATIFICAT	TON	
0		<u> </u>	(ft)	(ft)	0/6	6/12	12/18	18/24	(in)	1			_	AND CHANGE TOWN	ION	SYMBO
			-				<u> </u>	-		Fill: 1	Brown SIL	T, little co	oarse to fi	ne Sand, little coarse		<del>- </del> -
	SS	<del>  '</del>	1	3	10	10	11	14	17	to fine	Gravel					
	<u></u>		-							Brown	n coarse to	fine SAN	ID, little c	oarse to fine Gravel,	trace	
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<u> </u>	SS	2	5	7	20	26	-									
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						- 50	<del>  ''</del>	-12	20	Gray 8	SILT and c	oarse to fi	ine Sand (	weathered Sandstone	e)	
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Nominal I.D. of Split Barrel Sampler	1% in	The subsurface information shown hereon was obtained for the design and estimating purposes for our client.  It is made available to authorized users only that they may have access to the same information available
Weight/type of Hammer on Drive Pipe	300 lb	to our client. It is presented in good faith but it is not invested to the same information available
Weight/type of Hammer on Split Barrel	140 lb	to our client. It is presented in good faith, but it is not intended as a substitute for investigations, interpretations
Drop of Hammer on Drive Pipe	in	or judgment of such authorized users. Information on the logs should not be relied upon without the geotechnical engineers recommendations contained in the report from which these logs were extracted.
Core Size	ln	Pp: Pocket Penetrometer; WOH: Weight of Hammer; WOR: Weight of Rod
Soil descriptions represent a field identification	after D	Approximate Change in Strata: Inferred Change in Strata:

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ľ	<del></del>								1.25m	ROCK CORE: RUN 2 (18'-23')			
ŀ			-+		-+					RECOVERY = 60"/60" = 100%		Ī	
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Core Size	in engineers recommendations contained in the report from which these logs were extracted.  Pp: Pocket Penetrometer; WOH: Weight of Hammer; WOR: Weight of Rod
Soil descriptions represent a field identification	Approximate Change in Strata: Inferred Change in Strata: Inferred Change in Strata: Inferred Change in Strata: Inferred Change in Strata: Inferred Change in Strata: Inferred Change in Strata: Inferred Change in Strata: Inferred Change in Strata: Inferred Change in Strata: Inferred Change in Strata: Inferred Change in Strata: Inferred Change in Strata: Inferred Change in Strata: Inferred Change in Strata: Inferred Change in Strata: Inferred Change in Strata: Inferred Change in Strata: Inferred Change in Strata: Inferred Change in Strata: Inferred Change in Strata: Inferred Change in Strata: Inferred Change in Strata: Inferred Change in Strata: Inferred Change in Strata: Inferred Change in Strata: Inferred Change in Strata: Inferred Change in Strata: Inferred Change in Strata: Inferred Change in Strata: Inferred Change in Strata: Inferred Change in Strata: Inferred Change in Strata: Inferred Change in Strata: Inferred Change in Strata: Inferred Change in Strata: Inferred Change in Strata: Inferred Change in Strata: Inferred Change in Strata: Inferred Change in Strata: Inferred Change in Strata: Inferred Change in Strata: Inferred Change in Strata: Inferred Change in Strata: Inferred Change in Strata: Inferred Change in Strata: Inferred Change in Strata: Inferred Change in Strata: Inferred Change in Strata: Inferred Change in Strata: Inferred Change in Strata: Inferred Change in Strata: Inferred Change in Strata: Inferred Change in Strata:

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									Im	ROCK CORE: RUN I (12.5' - 17.5')	
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Nominal I.D. of Split Barrel Sampler	1% in	The subsurface information shown hereon was obtained for the design and estimating purposes for our client.
Weight/type of Hammer on Drive Pipe	300 lb	It is made available to authorized users only that they may have access to the same information available to our client. It is presented in cond faith the interest of the same information available
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Drop of Hammer on Drive Pipe	in	or judgment of such authorized users. Information on the logs should not be relied upon without the geotechnical
Core Size	in	engineers recommendations contained in the report from which these logs were extracted.  Pp: Pocket Penetrometer; WOH; Weight of Hammer; WOR: Weight of Rod
Soil descriptions represent a field identification	n after D	Approximate Change in Strata:

Approximate Change in Strata: \_\_\_\_\_\_Soil descriptions represent a field identification after D. M. Burmister unless otherwise noted.

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Section   Continue   Continue   Continue   Continue   Continue   Continue   Continue   Continue   Continue   Continue   Continue   Continue   Continue   Continue   Continue   Continue   Continue   Continue   Continue   Continue   Continue   Continue   Continue   Continue   Continue   Continue   Continue   Continue   Continue   Continue   Continue   Continue   Continue   Continue   Continue   Continue   Continue   Continue   Continue   Continue   Continue   Continue   Continue   Continue   Continue   Continue   Continue   Continue   Continue   Continue   Continue   Continue   Continue   Continue   Continue   Continue   Continue   Continue   Continue   Continue   Continue   Continue   Continue   Continue   Continue   Continue   Continue   Continue   Continue   Continue   Continue   Continue   Continue   Continue   Continue   Continue   Continue   Continue   Continue   Continue   Continue   Continue   Continue   Continue   Continue   Continue   Continue   Continue   Continue   Continue   Continue   Continue   Continue   Continue   Continue   Continue   Continue   Continue   Continue   Continue   Continue   Continue   Continue   Continue   Continue   Continue   Continue   Continue   Continue   Continue   Continue   Continue   Continue   Continue   Continue   Continue   Continue   Continue   Continue   Continue   Continue   Continue   Continue   Continue   Continue   Continue   Continue   Continue   Continue   Continue   Continue   Continue   Continue   Continue   Continue   Continue   Continue   Continue   Continue   Continue   Continue   Continue   Continue   Continue   Continue   Continue   Continue   Continue   Continue   Continue   Continue   Continue   Continue   Continue   Continue   Continue   Continue   Continue   Continue   Continue   Continue   Continue   Continue   Continue   Continue   Continue   Continue   Continue   Continue   Continue   Continue   Continue   Continue   Continue   Continue   Continue   Continue   Continue   Continue   Continue   Continue   Continue   Continue   Continue   Continue	1	METHOD	No.		┥				т		SOIL DESCRIPTION AND STRATIFICATION	SYMBOL
1	0	<del> </del>	<del> </del>	+	_	-		12/18	18/24	(in)		- Indoe
Red-brown/Gray c-f SAND, little m-f Gravel, little Silt		SS	1	10	2	4	6	8	11	13	Fill: Top 6" = Topsoil w/ Roots	├─-
5		ļ	<del> </del>	<u> </u>		<u> </u>		<u> </u>				<del></del>
SS   2   5   7   33   54   45   56   12   Red-brown c-f SAND, little c-f Gravel, little Silt   Hard drilling (weathererd Rock)   Hard drilling (weathererd Rock)   Hard drilling (weathererd Rock)   Hard drilling (weathererd Rock)   Hard drilling (weathererd Rock)   Hard drilling (weathererd Rock)   Hard drilling (weathererd Rock)   Hard drilling (weathererd Rock)   Hard drilling (weathererd Rock)   Hard drilling (weathererd Rock)   Hard drilling (weathererd Rock)   Hard drilling (weathererd Rock)   Hard drilling (weathererd Rock)   Hard drilling (weathererd Rock)   Hard drilling (weathererd Rock)   Hard drilling (weathererd Rock)   Hard drilling (weathererd Rock)   Hard drilling (weathererd Rock)   Hard drilling (weathererd Rock)   Hard drilling (weathererd Rock)   Hard drilling (weathererd Rock)   Hard drilling (weathererd Rock)   Hard drilling (weathererd Rock)   Hard drilling (weathererd Rock)   Hard drilling (weathererd Rock)   Hard drilling (weathererd Rock)   Hard drilling (weathererd Rock)   Hard drilling (weathererd Rock)   Hard drilling (weathererd Rock)   Hard drilling (weathererd Rock)   Hard drilling (weathererd Rock)   Hard drilling (weathererd Rock)   Hard drilling (weathererd Rock)   Hard drilling (weathererd Rock)   Hard drilling (weathererd Rock)   Hard drilling (weathererd Rock)   Hard drilling (weathererd Rock)   Hard drilling (weathererd Rock)   Hard drilling (weathererd Rock)   Hard drilling (weathererd Rock)   Hard drilling (weathererd Rock)   Hard drilling (weathererd Rock)   Hard drilling (weathererd Rock)   Hard drilling (weathererd Rock)   Hard drilling (weathererd Rock)   Hard drilling (weathererd Rock)   Hard drilling (weathererd Rock)   Hard drilling (weathererd Rock)   Hard drilling (weathererd Rock)   Hard drilling (weathererd Rock)   Hard drilling (weathererd Rock)   Hard dr				<del> </del>		_		ļ <u>.</u>				<del></del> -
SS   2   5   7   33   54   45   56   12   Red-brown c-f SAND, little c-f Gravel, little Silt   Hard drilling (weathererd Rock)   Hard drilling (weathererd Rock)   Hard drilling (weathererd Rock)   Hard drilling (weathererd Rock)   Hard drilling (weathererd Rock)   Hard drilling (weathererd Rock)   Hard drilling (weathererd Rock)   Hard drilling (weathererd Rock)   Hard drilling (weathererd Rock)   Hard drilling (weathererd Rock)   Hard drilling (weathererd Rock)   Hard drilling (weathererd Rock)   Hard drilling (weathererd Rock)   Hard drilling (weathererd Rock)   Hard drilling (weathererd Rock)   Hard drilling (weathererd Rock)   Hard drilling (weathererd Rock)   Hard drilling (weathererd Rock)   Hard drilling (weathererd Rock)   Hard drilling (weathererd Rock)   Hard drilling (weathererd Rock)   Hard drilling (weathererd Rock)   Hard drilling (weathererd Rock)   Hard drilling (weathererd Rock)   Hard drilling (weathererd Rock)   Hard drilling (weathererd Rock)   Hard drilling (weathererd Rock)   Hard drilling (weathererd Rock)   Hard drilling (weathererd Rock)   Hard drilling (weathererd Rock)   Hard drilling (weathererd Rock)   Hard drilling (weathererd Rock)   Hard drilling (weathererd Rock)   Hard drilling (weathererd Rock)   Hard drilling (weathererd Rock)   Hard drilling (weathererd Rock)   Hard drilling (weathererd Rock)   Hard drilling (weathererd Rock)   Hard drilling (weathererd Rock)   Hard drilling (weathererd Rock)   Hard drilling (weathererd Rock)   Hard drilling (weathererd Rock)   Hard drilling (weathererd Rock)   Hard drilling (weathererd Rock)   Hard drilling (weathererd Rock)   Hard drilling (weathererd Rock)   Hard drilling (weathererd Rock)   Hard drilling (weathererd Rock)   Hard drilling (weathererd Rock)   Hard drilling (weathererd Rock)   Hard drilling (weathererd Rock)   Hard dr	_		<u> </u>	<del> </del> -	-							<del></del>
1 min   Red-thrown of SAND, little of Gravel, little Silt	5	<del> </del> -		ļ	-	. <del> </del> _		<u> </u>				<del>                                      </del>
10	1	SS	2	5	7	33	54	45	56	12	Red-brown c-f SAND, little c-f Gravel, little Silt	<del> </del>
10		<del> </del>	<u> </u>	<del> </del>								
10		<del> </del>			┼		ļ				Hard drilling (weathererd Rock)	
I min   RCCV CORE: RUN   (9-13)	40	<del></del> -		<b> </b>		<del> </del>						
1 min   RECOVERY = 69' 48" = 12.5%     1 min   RQD = 09' 40" = 09'     1 min   RGMail SHALE and SANDSTONE w/ Boulders     1 min   ROCK CORE: RUN 2 (13-18)     1 min   ROCK CORE: RUN 2 (13-18)     1 min   ROCOVERY = 33" 60" = 55%     1 min   RQD = 14" (60" = 23.3%     1 min   ROCOVERY = 33" 60" = 55%     1 min   ROCOVERY = 33" 60" = 55%     1 min   ROCOVERY = 33" 60" = 55%     1 min   ROCOVERY = 33" 60" = 55%     1 min   ROCOVERY = 33" 60" = 55%     1 min   ROCOVERY = 33" 60" = 55%     1 min   ROCOVERY = 33" 60" = 55%     1 min   ROCOVERY = 33" 60" = 55%     1 min   ROCOVERY = 33" 60" = 55%     1 min   ROCOVERY = 33" 60" = 55%     1 min   ROCOVERY = 33" 60" = 55%     1 min   ROCOVERY = 33" 60" = 55%     1 min   ROCOVERY = 33" 60" = 55%     1 min   ROCOVERY = 33" 60" = 55%     1 min   ROCOVERY = 33" 60" = 55%     1 min   ROCOVERY = 33" 60" = 55%     1 min   ROCOVERY = 33" 60" = 55%     1 min   ROCOVERY = 33" 60" = 55%     1 min   ROCOVERY = 33" 60" = 55%     1 min   ROCOVERY = 33" 60" = 55%     1 min   ROCOVERY = 33" 60" = 55%     1 min   ROCOVERY = 33" 60" = 55%     1 min   ROCOVERY = 33" 60" = 55%     1 min   ROCOVERY = 33" 60" = 55%     1 min   ROCOVERY = 33" 60" = 55%     1 min   ROCOVERY = 33" 60" = 55%     1 min   ROCOVERY = 33" 60" = 55%     1 min   ROCOVERY = 33" 60" = 55%     1 min   ROCOVERY = 33" 60" = 55%     1 min   ROCOVERY = 33" 60" = 55%     1 min   ROCOVERY = 33" 60" = 55%     1 min   ROCOVERY = 33" 60" = 55%     1 min   ROCOVERY = 33" 60" = 55%     1 min   ROCOVERY = 33" 60" = 55%     1 min   ROCOVERY = 33" 60" = 55%     1 min   ROCOVERY = 33" 60" = 55%     1 min   ROCOVERY = 33" 60" = 55%     1 min   ROCOVERY = 33" 60" = 55%     1 min   ROCOVERY = 33" 60" = 55%     1 min   ROCOVERY = 33" 60" = 55%     1 min   ROCOVERY = 33" 60" = 55%     1 min   ROCOVERY = 33" 60" = 55%     1 min   ROCOVERY = 33" 60" = 55%     1 min   ROCOVERY = 33" 60" = 55%     1 min   ROCOVERY = 33" 60" = 55%     1 min   ROCOVERY = 33" 60" = 55%     1 min   ROCOVERY = 33" 60" = 55%     1 min   ROCOVERY = 33" 60" = 55%	10				<b>-</b>		<u> </u>		<u></u>	I min	ROCK CORE: RUN 1 (9'-13')	
1 min   RQD = 0° 40° = 0%     1 min   Residual SHALE and SANDSTONE w/ Boulders     1 min   RECOVERY = 33° (60° = 55%     1 min   RQD = 14° (60° = 23.3%     1 min   Residual SHALE and SANDSTONE w/ Boulders     1 min   Robert = 1 min     1 min   Robert = 1 min     20	ł					<del>-</del>	<u> </u>			1 min	RECOVERY = 6"/ 48" = 12.5%	
15   I min ROCK CORE: RUN 2 (13-18)   I min RECOVERY = 33" (60" = 55%   I min ROCK CORE: RUN 2 (13-18)   I min ROCK CORE: RUN 2 (13-18)   I min ROCK CORE: RUN 2 (13-18)   I min ROCK CORE: RUN 2 (13-18)   I min ROCK CORE: RUN 2 (13-18)   I min ROCK CORE: RUN 2 (13-18)   I min RECOVERY = 33" (60" = 23.3%   I min ROCK CORE: RUN 2 (13-18)   I min RECOVERY = 33" (60" = 25%   I min ROCK CORE: RUN 2 (13-18)   I min RECOVERY = 33" (60" = 25%   I min ROCK CORE: RUN 2 (13-18)   I min RECOVERY = 33" (60" = 25%   I min ROCK CORE: RUN 2 (13-18)   I min RECOVERY = 33" (60" = 25%   I min ROCK CORE: RUN 2 (13-18)   I min RECOVERY = 33" (60" = 25%   I min ROCK CORE: RUN 2 (13-18)   I min RECOVERY = 33" (60" = 25%   I min ROCK CORE: RUN 2 (13-18)   I min RECOVERY = 33" (60" = 25%   I min ROCK CORE: RUN 2 (13-18)   I min RECOVERY = 33" (60" = 25%   I min ROCK CORE: RUN 2 (13-18)   I min RECOVERY = 33" (60" = 25%   I min ROCK CORE: RUN 2 (13-18)   I min RECOVERY = 33" (60" = 25%   I min ROCK CORE: RUN 2 (13-18)   I min ROCK CORE: RUN 2 (13-18)   I min ROCK CORE: RUN 2 (13-18)   I min ROCK CORE: RUN 2 (13-18)   I min ROCK CORE: RUN 2 (13-18)   I min ROCK CORE: RUN 2 (13-18)   I min ROCK CORE: RUN 2 (13-18)   I min ROCK CORE: RUN 2 (13-18)   I min ROCK CORE: RUN 2 (13-18)   I min ROCK CORE: RUN 2 (13-18)   I min ROCK CORE: RUN 2 (13-18)   I min ROCK CORE: RUN 2 (13-18)   I min ROCK CORE: RUN 2 (13-18)   I min ROCK CORE: RUN 2 (13-18)   I min ROCK CORE: RUN 2 (13-18)   I min ROCK CORE: RUN 2 (13-18)   I min ROCK CORE: RUN 2 (13-18)   I min ROCK CORE: RUN 2 (13-18)   I min ROCK CORE: RUN 2 (13-18)   I min ROCK CORE: RUN 2 (13-18)   I min ROCK CORE: RUN 2 (13-18)   I min ROCK CORE: RUN 2 (13-18)   I min ROCK CORE: RUN 2 (13-18)   I min ROCK CORE: RUN 2 (13-18)   I min ROCK CORE: RUN 2 (13-18)   I min ROCK CORE: RUN 2 (13-18)   I min ROCK CORE: RUN 2 (13-18)   I min ROCK CORE: RUN 2 (13-18)   I min ROCK CORE: RUN 2 (13-18)   I min ROCK CORE: RUN 2 (13-18)   I min ROCK CORE: RUN 2 (13-18)   I min ROCK CORE: RUN 2 (13-18)   I min ROCK COR			<u>-</u>		<b>-</b>	·				I min	$RQD = 0^{\circ}/40^{\circ} = 0\%$	
15   I min ROCK CORE: RUN 2 (13-18)   I min RECOVERY = 33" (60" = 55%   I min ROCK CORE: RUN 2 (13-18)   I min ROCK CORE: RUN 2 (13-18)   I min ROCK CORE: RUN 2 (13-18)   I min ROCK CORE: RUN 2 (13-18)   I min ROCK CORE: RUN 2 (13-18)   I min ROCK CORE: RUN 2 (13-18)   I min RECOVERY = 33" (60" = 23.3%   I min ROCK CORE: RUN 2 (13-18)   I min RECOVERY = 33" (60" = 25%   I min ROCK CORE: RUN 2 (13-18)   I min RECOVERY = 33" (60" = 25%   I min ROCK CORE: RUN 2 (13-18)   I min RECOVERY = 33" (60" = 25%   I min ROCK CORE: RUN 2 (13-18)   I min RECOVERY = 33" (60" = 25%   I min ROCK CORE: RUN 2 (13-18)   I min RECOVERY = 33" (60" = 25%   I min ROCK CORE: RUN 2 (13-18)   I min RECOVERY = 33" (60" = 25%   I min ROCK CORE: RUN 2 (13-18)   I min RECOVERY = 33" (60" = 25%   I min ROCK CORE: RUN 2 (13-18)   I min RECOVERY = 33" (60" = 25%   I min ROCK CORE: RUN 2 (13-18)   I min RECOVERY = 33" (60" = 25%   I min ROCK CORE: RUN 2 (13-18)   I min RECOVERY = 33" (60" = 25%   I min ROCK CORE: RUN 2 (13-18)   I min ROCK CORE: RUN 2 (13-18)   I min ROCK CORE: RUN 2 (13-18)   I min ROCK CORE: RUN 2 (13-18)   I min ROCK CORE: RUN 2 (13-18)   I min ROCK CORE: RUN 2 (13-18)   I min ROCK CORE: RUN 2 (13-18)   I min ROCK CORE: RUN 2 (13-18)   I min ROCK CORE: RUN 2 (13-18)   I min ROCK CORE: RUN 2 (13-18)   I min ROCK CORE: RUN 2 (13-18)   I min ROCK CORE: RUN 2 (13-18)   I min ROCK CORE: RUN 2 (13-18)   I min ROCK CORE: RUN 2 (13-18)   I min ROCK CORE: RUN 2 (13-18)   I min ROCK CORE: RUN 2 (13-18)   I min ROCK CORE: RUN 2 (13-18)   I min ROCK CORE: RUN 2 (13-18)   I min ROCK CORE: RUN 2 (13-18)   I min ROCK CORE: RUN 2 (13-18)   I min ROCK CORE: RUN 2 (13-18)   I min ROCK CORE: RUN 2 (13-18)   I min ROCK CORE: RUN 2 (13-18)   I min ROCK CORE: RUN 2 (13-18)   I min ROCK CORE: RUN 2 (13-18)   I min ROCK CORE: RUN 2 (13-18)   I min ROCK CORE: RUN 2 (13-18)   I min ROCK CORE: RUN 2 (13-18)   I min ROCK CORE: RUN 2 (13-18)   I min ROCK CORE: RUN 2 (13-18)   I min ROCK CORE: RUN 2 (13-18)   I min ROCK CORE: RUN 2 (13-18)   I min ROCK COR					<del> </del>	<u> </u>				1 min	Residual SHALE and SANDSTONE w/ Boulders	
1 min RECOVERY = 33"/60" = 55% RQD = 14"/60" = 23.3% RQD = 14"/60" = 23.3% Residual SHALE and SANDSTONE w/ Boulders  20 BORING COMPLETE AT 18.0 FRET	46			<del></del>	<b> </b>			<u> </u>		I min	ROCK CORE: RUN 2 (13'-18')	
1 min   Residual SHALE and SANDSTONE w/ Boulders   1 min   Residual SHALE and SANDSTONE w/ Boulders   1 min   1 min   1 min   1 min   1 min   1 min   1 min   1 min   1 min   1 min   1 min   1 min   1 min   1 min   1 min   1 min   1 min   1 min   1 min   1 min   1 min   1 min   1 min   1 min   1 min   1 min   1 min   1 min   1 min   1 min   1 min   1 min   1 min   1 min   1 min   1 min   1 min   1 min   1 min   1 min   1 min   1 min   1 min   1 min   1 min   1 min   1 min   1 min   1 min   1 min   1 min   1 min   1 min   1 min   1 min   1 min   1 min   1 min   1 min   1 min   1 min   1 min   1 min   1 min   1 min   1 min   1 min   1 min   1 min   1 min   1 min   1 min   1 min   1 min   1 min   1 min   1 min   1 min   1 min   1 min   1 min   1 min   1 min   1 min   1 min   1 min   1 min   1 min   1 min   1 min   1 min   1 min   1 min   1 min   1 min   1 min   1 min   1 min   1 min   1 min   1 min   1 min   1 min   1 min   1 min   1 min   1 min   1 min   1 min   1 min   1 min   1 min   1 min   1 min   1 min   1 min   1 min   1 min   1 min   1 min   1 min   1 min   1 min   1 min   1 min   1 min   1 min   1 min   1 min   1 min   1 min   1 min   1 min   1 min   1 min   1 min   1 min   1 min   1 min   1 min   1 min   1 min   1 min   1 min   1 min   1 min   1 min   1 min   1 min   1 min   1 min   1 min   1 min   1 min   1 min   1 min   1 min   1 min   1 min   1 min   1 min   1 min   1 min   1 min   1 min   1 min   1 min   1 min   1 min   1 min   1 min   1 min   1 min   1 min   1 min   1 min   1 min   1 min   1 min   1 min   1 min   1 min   1 min   1 min   1 min   1 min   1 min   1 min   1 min   1 min   1 min   1 min   1 min   1 min   1 min   1 min   1 min   1 min   1 min   1 min   1 min   1 min   1 min   1 min   1 min   1 min   1 min   1 min   1 min   1 min   1 min   1 min   1 min   1 min   1 min   1 min   1 min   1 min   1 min   1 min   1 min   1 min   1 min   1 min   1 min   1 min   1 min   1 min   1 min   1 min   1 min   1 min   1 min   1 min   1 min   1 min   1 min   1 min   1 min   1 min   1 min   1 min   1 min	10			ļ	<u> </u>	<u> </u>				1 min	RBCOVERY = 33"/ 60" = 55%	
20					<b> </b>	<b>├</b> ──						
20	]				ļ	<u> </u>				1 min	Residual SHALE and SANDSTONE w/ Boulders	
25					<del> </del>	<del> </del>				1 min		
25	20				<del> </del>	<del> </del> -	-				BORING COMPLETE AT 18.0 FEET	
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Nominal I.D. of Hole	ir	The subsurface information about to
Nominal I.D. of Split Barrel Sampler	13% in	The subsurface information shown hereon was obtained for the design and estimating purposes for our client.
Weight/type of Hammer on Drive Pipe	300 lb	It is made available to authorized users only that they may have access to the same information available
Weight/type of Hammer on Split Barrel	140 lb	to our client. It is presented in good faith, but it is not intended as a substitute for investigations, interpretations
Drop of Hammer on Drive Pipe	in	or judgment of such authorized users. Information on the logs should not be relied upon without the geotechnical engineers recommendations contained in the report from which these logs were extracted.
Core Size	ln	Pp: Pocket Penetrometer; WOH: Weight of Hammer; WOR: Weight of Rod
Soil descriptions represent a field identification	after D.	Approximate Change in Strate:

Æ	77 6		(A) E)		<del> </del>					5485 BORING LOG SB-15.	.xis
	PROJECT NAME:									CONCORD BORING NO. SB	-15
4	OONSULTING LOCATION:						CATION		M	Ionticello, NY JOB NO. 54	
BOBI	NG BY: G		ERS		<del> </del>					GROUND ELEVATION: 147	
	ECTOR:						ARTED:	<del> </del>	/2008	GROUNDWATER TABLE DEPTH: 13'	<u> </u>
DEPTH		1		PTH	DATE	E COMF	LETED:	6/17	/2008	0 Hr. 13' Date 6/17/08 24 Hr. N/A Date N/A	
(ft)	METHOD	SAMPLE	:}		_	Blows	on Spoor	1	REC		
0	METIOD	No.	FROM	+	ļ				1,120	SOIL DESCRIPTION AND STRATIFICATION	SYMBOL
1	<del> </del>	<del> </del> _	(ft)	(ft)	0/6	6/12	12/18	18/24	(in)		
	53	1	0	2	19	28	24	31	18	6" Concrete	
Ì		<del> </del> -	<b>├</b> —	-	ļ		<u> </u>			Fill: Brown/Red-brown/Gray c-f SAND, some c-f Gravel, little	
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	SS	2	5	7	24	27	31	32	24	Red-Brown c-f SAND, some m-f Gravel, little Silt w/ pieces	
	<u> </u>	<del> </del>	ļ	ļ	-	<b></b>				of Weathered Rock (some moisture/probable perched water)	
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1 ,_	<b> </b>			ļ	<u> </u>					]	
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Nominal I.D. of Hole	In	The subsurface information shows because
Nominal I.D. of Split Barrel Sampler	1% in	The subsurface information shown hereon was obtained for the design and estimating purposes for our client.
Weight/type of Hammer on Drive Pipe	300 lb	It is made available to authorized users only that they may have access to the same information available to our client. It is presented in good to the but it is not in a lateral to our client.
Weight/type of Hammer on Split Barrel	140 lb	to our client. It is presented in good faith, but it is not intended as a substitute for investigations, interpretations or judgment of such authorized users. Information on the base of the same information and the base of the same information and the base of the same information and the base of the same information and the base of the same information and the base of the same information and the base of the same information and the base of the same information and the base of the same information and the base of the same information and the base of the same information and the base of the same information and the base of the same information and the base of the same information and the base of the same information and the base of the same information and the base of the same information and the base of the same information and the base of the same information and the base of the same information and the base of the same information and the base of the same information and the base of the same information and the base of the same information and the base of the same information and the base of the same information and the base of the same information and the base of the same information and the base of the same information and the base of the same information and the base of the same information and the base of the same information and the base of the same information and the base of the same information and the base of the same information and the base of the same information and the base of the same information and the base of the same information and the base of the same information and the base of the same information and the base of the same information and the base of the same information and the base of the same information and the base of the same information and the base of the same information and the base of the base of the same information and the base of the base of the base of the base of the base of the base of the base of the base of the base of the base of the base of the base of the base of th
Drop of Hammer on Drive Pipe	in	or judgment of such authorized users. Information on the logs should not be relied upon without the geotechnical engineers recommendations contained in the report from which these logs were extracted.
Core Size	ln	Pp: Pocket Penetrometer; WOH: Weight of Hammer; WOR: Weight of Rod
		Approximate Change in Strata: Inferred Change in Strata:

PROJECT NAME   CONCORD   BORING NO. 588-16	700	777 7	<u> </u>			T					5485 BORING LOG SB-	15.xls
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0	(ft)	METHOD	1	FROM	ТО		DIOWS C	ni Ohooi		KEC	SOIL DESCRIPTION AND STRATIFICATION	
SS   1   0   2   19   28   24   31   18   6" Concrete Fill Brown/Red-hrown/Gray of SAND, some of Gravel, little Silt with pieces of Weathered Rock (some moisture/probable perthed water)   10   12   43   44   50/2"   15   5   5   5   5   5   5   5   5	0	<u> </u>		(ft)	(ft)	0/6	6/12	12/18	18/24	(in)		SYMBOL
File Brown/Red-brown/Gray c-f SAND, some c-f Gravel, little  Silk  5		\$3	1	0	2	19	28				6# Congrete	<del> </del>
Silk   Silk   Silk   Silk   Silk   Silk   Silk   Silk   Silk   Silk   Silk   Silk   Silk   Silk   Silk   Silk   Silk   Silk   Silk   Silk   Silk   Silk   Silk   Silk   Silk   Silk   Silk   Silk   Silk   Silk   Silk   Silk   Silk   Silk   Silk   Silk   Silk   Silk   Silk   Silk   Silk   Silk   Silk   Silk   Silk   Silk   Silk   Silk   Silk   Silk   Silk   Silk   Silk   Silk   Silk   Silk   Silk   Silk   Silk   Silk   Silk   Silk   Silk   Silk   Silk   Silk   Silk   Silk   Silk   Silk   Silk   Silk   Silk   Silk   Silk   Silk   Silk   Silk   Silk   Silk   Silk   Silk   Silk   Silk   Silk   Silk   Silk   Silk   Silk   Silk   Silk   Silk   Silk   Silk   Silk   Silk   Silk   Silk   Silk   Silk   Silk   Silk   Silk   Silk   Silk   Silk   Silk   Silk   Silk   Silk   Silk   Silk   Silk   Silk   Silk   Silk   Silk   Silk   Silk   Silk   Silk   Silk   Silk   Silk   Silk   Silk   Silk   Silk   Silk   Silk   Silk   Silk   Silk   Silk   Silk   Silk   Silk   Silk   Silk   Silk   Silk   Silk   Silk   Silk   Silk   Silk   Silk   Silk   Silk   Silk   Silk   Silk   Silk   Silk   Silk   Silk   Silk   Silk   Silk   Silk   Silk   Silk   Silk   Silk   Silk   Silk   Silk   Silk   Silk   Silk   Silk   Silk   Silk   Silk   Silk   Silk   Silk   Silk   Silk   Silk   Silk   Silk   Silk   Silk   Silk   Silk   Silk   Silk   Silk   Silk   Silk   Silk   Silk   Silk   Silk   Silk   Silk   Silk   Silk   Silk   Silk   Silk   Silk   Silk   Silk   Silk   Silk   Silk   Silk   Silk   Silk   Silk   Silk   Silk   Silk   Silk   Silk   Silk   Silk   Silk   Silk   Silk   Silk   Silk   Silk   Silk   Silk   Silk   Silk   Silk   Silk   Silk   Silk   Silk   Silk   Silk   Silk   Silk   Silk   Silk   Silk   Silk   Silk   Silk   Silk   Silk   Silk   Silk   Silk   Silk   Silk   Silk   Silk   Silk   Silk   Silk   Silk   Silk   Silk   Silk   Silk   Silk   Silk   Silk   Silk   Silk   Silk   Silk   Silk   Silk   Silk   Silk   Silk   Silk   Silk   Silk   Silk   Silk   Silk   Silk   Silk   Silk   Silk   Silk   Silk   Silk   Silk   Silk   Silk   Silk   Silk   Silk	Į.							<del>                                     </del>		<del>  `</del> `		
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SS   2   5   7   24   27   31   32   24   Red-Brown of SAND, some m-Gravel, little Silt w/ pieces of Weathered Rock (some moisture/probable perched water)	1				·	<del> </del> -		<del>                                     </del>		<del> </del>		<u></u>
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Same    Same   Same   Same   Same   Same   Same   Same   Same   Same   Same   Same   Same   Same   Same   Same   Same   Same   Same   Same   Same   Same   Same   Same   Same   Same   Same   Same   Same   Same   Same   Same   Same   Same   Same   Same   Same   Same   Same   Same   Same   Same   Same   Same   Same   Same   Same   Same   Same   Same   Same   Same   Same   Same   Same   Same   Same   Same   Same   Same   Same   Same   Same   Same   Same   Same   Same   Same   Same   Same   Same   Same   Same   Same   Same   Same   Same   Same   Same   Same   Same   Same   Same   Same   Same   Same   Same   Same   Same   Same   Same   Same   Same   Same   Same   Same   Same   Same   Same   Same   Same   Same   Same   Same   Same   Same   Same   Same   Same   Same   Same   Same   Same   Same   Same   Same   Same   Same   Same   Same   Same   Same   Same   Same   Same   Same   Same   Same   Same   Same   Same   Same   Same   Same   Same   Same   Same   Same   Same   Same   Same   Same   Same   Same   Same   Same   Same   Same   Same   Same   Same   Same   Same   Same   Same   Same   Same   Same   Same   Same   Same   Same   Same   Same   Same   Same   Same   Same   Same   Same   Same   Same   Same   Same   Same   Same   Same   Same   Same   Same   Same   Same   Same   Same   Same   Same   Same   Same   Same   Same   Same   Same   Same   Same   Same   Same   Same   Same   Same   Same   Same   Same   Same   Same   Same   Same   Same   Same   Same   Same   Same   Same   Same   Same   Same   Same   Same   Same   Same   Same   Same   Same   Same   Same   Same   Same   Same   Same   Same   Same   Same   Same   Same   Same   Same   Same   Same   Same   Same   Same   Same   Same   Same   Same   Same   Same   Same   Same   Same   Same   Same   Same   Same   Same   Same   Same   Same   Same   Same   Same   Same   Same   Same   Same   Same   Same   Same   Same   Same   Same   Same   Same   Same   Same   Same   Same   Same   Same   Same   Same   Same   Same   Same   Same   Same   Same   Same   Same   Same   Same   Same   Same   S	ĺ			<del> </del>	<del> </del>	$\vdash \vdash$	<del> </del>	<del> </del>			or Weathered Rock (some moisture/probable perched water)	<u></u>
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Nominal I.D. of Hole	ir	The subsurface information shown hereon was obtained for the design and estimating purposes for our client.							
Nominal I.D. of Split Barrel Sampler	1% in	It is made available to authorized users only that they may have access to the same information available							
Weight/type of Hammer on Drive Pipe	300 lb	o our client. It is presented in good faith, but it is not intended as a substitute for investigations, interpre							
Weight/type of Hammer on Split Barrel	140 lb	or judgment of such authorized users. Information on the logs should not be relied upon without the geotechnical							
Drop of Hammer on Drive Pipe	in	engineers recommendations contained in the report from which these logs were extracted.							
Core Size	in	Pp: Pocket Penetrometer; WOH: Weight of Hammer; WOR: Weight of Rod							
Soil descriptions represent a field identification	n after D.	Approximate Change in Strata: Inferred Change in Strata:							
		FIGURE 16 Page 1 of 1							

Æ	337.7 C		- n					,		5485 BORING LOG SB-16	.xls
	I MASES!						NAME			CONCORD BORING NO. SB	-16
	CONSULTING					LOC	CATION	<b>-</b>	M	onticello, NY JOB NO. 54	85
BORI	NG BY: G		E N S		<del>  _</del>	ATT OT	ADZED.	-		GROUND ELEVATION: 146	50 <u>+</u>
	ECTOR: J						ARTED: LETED:		/2008	GROUNDWATER TABLE DEPTH: NE	
DEPTH			DE	PTH					/2008 	0 Hr. N/A Date N/A 24 Hr. N/A Date N/A	A.
(ft)	METHOD	SAMPLE	FROM		1	Blows c	л Ѕроог	1	REC	CON DECODIFICATION	
0		No.	(ft)	(ft)	0/6	6/12	12/18	18/24	(ini)	SOIL DESCRIPTION AND STRATIFICATION	SYMBOL
	SS	ı	0	2	4	3	1	2	6	The 11 can be a second of the 12 can be a second of the 12 can be a second of the 12 can be a second of the 12 can be a second of the 12 can be a second of the 12 can be a second of the 12 can be a second of the 12 can be a second of the 12 can be a second of the 12 can be a second of the 12 can be a second of the 12 can be a second of the 12 can be a second of the 12 can be a second of the 12 can be a second of the 12 can be a second of the 12 can be a second of the 12 can be a second of the 12 can be a second of the 12 can be a second of the 12 can be a second of the 12 can be a second of the 12 can be a second of the 12 can be a second of the 12 can be a second of the 12 can be a second of the 12 can be a second of the 12 can be a second of the 12 can be a second of the 12 can be a second of the 12 can be a second of the 12 can be a second of the 12 can be a second of the 12 can be a second of the 12 can be a second of the 12 can be a second of the 12 can be a second of the 12 can be a second of the 12 can be a second of the 12 can be a second of the 12 can be a second of the 12 can be a second of the 12 can be a second of the 12 can be a second of the 12 can be a second of the 12 can be a second of the 12 can be a second of the 12 can be a second of the 12 can be a second of the 12 can be a second of the 12 can be a second of the 12 can be a second of the 12 can be a second of the 12 can be a second of the 12 can be a second of the 12 can be a second of the 12 can be a second of the 12 can be a second of the 12 can be a second of the 12 can be a second of the 12 can be a second of the 12 can be a second of the 12 can be a second of the 12 can be a second of the 12 can be a second of the 12 can be a second of the 12 can be a second of the 12 can be a second of the 12 can be a second of the 12 can be a second of the 12 can be a second of the 12 can be a second of the 12 can be a second of the 12 can be a second of the 12 can be a second of the 12 can be a second of the 12 can be a second of the 12 can be a se	
			$\vdash$	1	<del> </del>	+-				Topsoil (wood in tip)	
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5										-	
	SS	2	5	7	30	13	8	14	5	Fill: Pieces of Concrete and Wood	
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_ ا	<u> </u>									***************************************	
10				<del> </del>						}	
	98	3	10	12	14	40	45	41	L5	Red-brown c-f SAND, some c-f Gravel, little Silt, w/ pieces of	
				<u> </u>						Weathered Rock	<del></del>
	<del></del>			┼-	<del> </del> -	<u> </u>	ļ				
15				<del> </del>							
	SS	4	15	17	9	17	- 07				
			12	17	, , , , , , , , , , , , , , , , , , ,	17	27	21	18	Red-brown c-f SAND, some Silt, little c-f Gravel w/	
		<u> </u>		<del>                                     </del>	<u> </u>		<b></b>			Weathered Rook	
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	SS	5	20	22	23	34	27	40	24	Same	
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40		†	1								

Nominal I.D. of Hole	in	The subsurface information shows however the subsurface information shows however
Nominal I.D. of Split Barrel Sampler	1% in	The subsurface information shown hereon was obtained for the design and estimating purposes for our client.
	300 lb	It is made available to authorized users only that they may have access to the same information available
Weight/type of Hammer on Split Barrel	140 lb	to our client. It is presented in good faith, but it is not intended as a substitute for investigations, interpretations
Drop of Hammer on Drive Pipe	in	or judgment of such authorized users. Information on the logs should not be relied upon without the geotechnical
Core Size	În	engineers recommendations contained in the report from which these logs were extracted.  Pp: Pocket Penetrometer; WOH: Weight of Hammer; WOR: Weight of Rod
		Approximate Change in Strata: Inferred Change in Strata:

Dopin (least below grade)	FITT C	<b>→</b> IF		- II	I BBO	IECT NAME:		7~	1100							5485 BOR	NG LOG	SB-17.xl:	\$
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DOUNG DEV. GET		CNS	ULT	ING	PRO	JECT LOCATION:		Mo	inticel	o, NY					JOB NO.				
MATE COMPLETION			NE	R 8		<del></del>		+		<del>-,-</del> -					GROU	ND ELEVATION:			
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Depth (feet below grades)   Top of Cashing   Top of Size   Top of Size   Top of Size   Top of Size   Top of Size   Top of Size   Top of Size   Top of Size   Top of Size   Top of Size   Top of Size   Top of Size   Top of Size   Top of Size   Top of Size   Top of Size   Top of Size   Top of Size   Top of Size   Top of Size   Top of Size   Top of Size   Top of Size   Top of Size   Top of Size   Top of Size   Top of Size   Top of Size   Top of Size   Top of Size   Top of Size   Top of Size   Top of Size   Top of Size   Top of Size   Top of Size   Top of Size   Top of Size   Top of Size   Top of Size   Top of Size   Top of Size   Top of Size   Top of Size   Top of Size   Top of Size   Top of Size   Top of Size   Top of Size   Top of Size   Top of Size   Top of Size   Top of Size   Top of Size   Top of Size   Top of Size   Top of Size   Top of Size   Top of Size   Top of Size   Top of Size   Top of Size   Top of Size   Top of Size   Top of Size   Top of Size   Top of Size   Top of Size   Top of Size   Top of Size   Top of Size   Top of Size   Top of Size   Top of Size   Top of Size   Top of Size   Top of Size   Top of Size   Top of Size   Top of Size   Top of Size   Top of Size   Top of Size   Top of Size   Top of Size   Top of Size   Top of Size   Top of Size   Top of Size   Top of Size   Top of Size   Top of Size   Top of Size   Top of Size   Top of Size   Top of Size   Top of Size   Top of Size   Top of Size   Top of Size   Top of Size   Top of Size   Top of Size   Top of Size   Top of Size   Top of Size   Top of Size   Top of Size   Top of Size   Top of Size   Top of Size   Top of Size   Top of Size   Top of Size   Top of Size   Top of Size   Top of Size   Top of Size   Top of Size   Top of Size   Top of Size   Top of Size   Top of Size   Top of Size   Top of Size   Top of Size   Top of Size   Top of Size   Top of Size   Top of Size   Top of Size   Top of Size   Top of Size   Top of Size   Top of Size   Top of Size   Top of Size   Top of Size   Top of Size   Top of Size   Top of Size   Top of Size   Top of Size		w	/ELL CC	NSTRUC	CTION		(1)	H) epands		Blow	s on Sp	oon	REC		SOIL		TION	T	.l.D.
Top of Sand Pack   16.0   Well Key:   10.0   Well Key:   10.0   Well Key:   10.0   Well Key:   10.0   Well Key:   10.0   Well Key:   10.0   Well Key:   10.0   Well Key:   10.0   Well Key:   10.0   Well Key:   10.0   Well Key:   10.0   Well Key:   10.0   Well Key:   10.0   Well Key:   10.0   Well Key:   10.0   Well Key:   10.0   Well Key:   10.0   Well Key:   10.0   Well Key:   10.0   Well Key:   10.0   Well Key:   10.0   Well Key:   10.0   Well Key:   10.0   Well Key:   10.0   Well Key:   10.0   Well Key:   10.0   Well Key:   10.0   Well Key:   10.0   Well Key:   10.0   Well Key:   10.0   Well Key:   10.0   Well Key:   10.0   Well Key:   10.0   Well Key:   10.0   Well Key:   10.0   Well Key:   10.0   Well Key:   10.0   Well Key:   10.0   Well Key:   10.0   Well Key:   10.0   Well Key:   10.0   Well Key:   10.0   Well Key:   10.0   Well Key:   10.0   Well Key:   10.0   Well Key:   10.0   Well Key:   10.0   Well Key:   10.0   Well Key:   10.0   Well Key:   10.0   Well Key:   10.0   Well Key:   10.0   Well Key:   10.0   Well Key:   10.0   Well Key:   10.0   Well Key:   10.0   Well Key:   10.0   Well Key:   10.0   Well Key:   10.0   Well Key:   10.0   Well Key:   10.0   Well Key:   10.0   Well Key:   10.0   Well Key:   10.0   Well Key:   10.0   Well Key:   10.0   Well Key:   10.0   Well Key:   10.0   Well Key:   10.0   Well Key:   10.0   Well Key:   10.0   Well Key:   10.0   Well Key:   10.0   Well Key:   10.0   Well Key:   10.0   Well Key:   10.0   Well Key:   10.0   Well Key:   10.0   Well Key:   10.0   Well Key:   10.0   Well Key:   10.0   Well Key:   10.0   Well Key:   10.0   Well Key:   10.0   Well Key:   10.0   Well Key:   10.0   Well Key:   10.0   Well Key:   10.0   Well Key:   10.0   Well Key:   10.0   Well Key:   10.0   Well Key:   10.0   Well Key:   10.0   Well Key:   10.0   Well Key:   10.0   Well Key:   10.0   Well Key:   10.0   Well Key:   10.0   Well Key:   10.0   Well Key:   10.0   Well Key:   10.0   Well Key:   10.0   Well Key:   10.0   Well Key:   10.0   Well Key:   10.0   Well Key:   10.0   Well	Doolb (foot heles						→ °	+-	O/	6/1	2 12/1	8 18/2	4 (In)						
Carcund Surface Top of Pister  Top of Seat  14.0  Well Key:  15	1	A Buside)					1.	<u> </u>	- -	-	┸			6" Asph	alt			-	
Top of Sand Pack  Well Key:  10	1 .		0 632	<b>1</b>	OS LA	<b>.</b>	1 (0 3	ES-	1 8	6	9	16	20	FIII: Re	d-Brown	n SILT, little medium to line Sand, lit	tite	H	
Wed Cap:	1		0	<b> </b> -				$\perp$	┷-	╀_								$\vdash$	
Top of Sand Pack   14.0   Well Key:   10   14   17   12   File: Cary coarse to line SAND, little SIII, little coarse to line Gravel, trace Asphall   15   10   12   25   13   23   15   17   10   10   10   10   10   10   10	1 pp of Riser					Flush Mount		<u> </u>	Щ.	Д.,				]				<u> </u>	
Top of Sand Pack  10.0    Neil Key:	ļ				M.		- 5	<del>Ļ</del> .		┸				1				$\vdash$	
Top of Sand Pack 16.0    Well Key: 10	[	14			拠拠	Well Cap;	5 to 7	\$\$-	2 9	10	14	17	12	Fill: Gra	у соаля	se to line SAND. Wille SW Wille coare			
Top of Sand Pack  18.0  Well Key;  10  10 to 12 sed 3 13 23 55 57 10 Brown coarse to fine SAND, the Still, title coarse to fine Clave (weathered Sandstone)  Plane Pipe:  4* PVC  Grout Type:  16  Grout Type:  16  Grout Type:  17  Sand  Pack Sizes  20  Filter Quarte  Filter Quarte  Filter Sizes  20  Solo 22 sed 17 30 22 22 18  Filter Sizes  20  Solo 25 sed 17 30 22 22 18  Filter Sizes  20  Solo 25 sed 33 46 65 750° 24  Filter Sizes  20  Solo 25 sed 33 46 65 750° 24  Filter Sizes  30 sed 25 sed 33 46 65 750° 24  Filter Sizes  30 sed 25 sed 33 46 65 750° 24  Filter Sizes  30 sed 25 sed 33 46 65 750° 24  Filter Sizes  30 sed 32 sed 33 46 65 750° 24  Filter Sizes  30 sed 32 sed 33 46 65 750° 24  Filter Sizes  30 sed 32 sed 33 46 65 750° 24  Filter Sizes  30 sed 32 sed 33 46 65 750° 24  Filter Sizes  30 sed 32 sed 33 46 65 750° 24  Filter Sizes  30 sed 32 sed 33 46 65 750° 24  Filter Sizes  30 sed 32 sed 33 46 65 750° 24  Filter Sizes  30 sed 32 sed 33 46 65 750° 24  Filter Sizes  30 sed 32 sed 33 46 65 750° 24  Filter Sizes  30 sed 32 sed 33 46 65 750° 24  Filter Sizes  30 sed 32 sed 33 46 65 750° 24  Filter Sizes  30 sed 32 sed 33 86  Filter Sizes  30 sed 32 sed 33 86  Filter Sizes  30 sed 32 sed 33 86  Filter Sizes  30 sed 32 sed 33 86  Filter Sizes  30 sed 32 sed 33 86  Filter Sizes  30 sed 32 sed 33 86  Filter Sizes  30 sed 32 sed 33 86  Filter Sizes  30 sed 32 sed 33 86  Filter Sizes  30 sed 32 sed 33 86  Filter Sizes  50 sed 30 sed 30 sed 30 sed 30 sed 30 sed 30 sed 30 sed 30 sed 30 sed 30 sed 30 sed 30 sed 30 sed 30 sed 30 sed 30 sed 30 sed 30 sed 30 sed 30 sed 30 sed 30 sed 30 sed 30 sed 30 sed 30 sed 30 sed 30 sed 30 sed 30 sed 30 sed 30 sed 30 sed 30 sed 30 sed 30 sed 30 sed 30 sed 30 sed 30 sed 30 sed 30 sed 30 sed 30 sed 30 sed 30 sed 30 sed 30 sed 30 sed 30 sed 30 sed 30 sed 30 sed 30 sed 30 sed 30 sed 30 sed 30 sed 30 sed 30 sed 30 sed 30 sed 30 sed 30 sed 30 sed 30 sed 30 sed 30 sed 30 sed 30 sed 30 sed 30 sed 30 sed 30 sed 30 sed 30 sed 30 sed 30 sed 30 sed 30 sed 30 sed 30 sed 30 sed 30 sed 30 sed 30	Top of Seal	14.	~‱	<b>3</b>	<b>***</b>	8		L		_[ _							SB 10	-	
Well Key		40.4	_ <b>  8888</b>	8	<b>***</b>	Ŗ.	1			$\mathbf{I}^{-}$	$\top$			1		Topinal .		$\vdash$	
Top of Stores	Top of Sand Pack	10.0				Well Key:					1			auger	athdin	a fuesthered Condelana		$\vdash$	
Riber Pipo:   10.0 12   86-3   13   22   55   57   10   Brown course to fine SAND, little Course to fine   Gravel (weathered Sandsione)					***		10			$\top$	1	<del> </del>						-⊢	
After Pipe:						}	10 to 12	2 15-3	13	23	55	57	10						
Top of Screen 18.0    34" Particular   16   16   17   14   21   22   49   33   16   18   18   18   18   18   18   18						Riser Pipa:				1		1	<del>                                     </del>				o fine	$\vdash$	
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The subsurface information shown hereon was obtained for the design and estimating purposes for our client. It is made available to authorized users only that they may have access to the same information available to our client. It is presented in good faith, but it is not intended as a substitute for investigations, interpretations or judgment of such authorized users. Information on the logs should not be relied upon without the geotechnical engineers recommendations contained in the report from which these logs were extracted. Soil descriptions represent a field identification after D. M. Burmister unless otherwise noted.

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	140 lb	to our client. It is presented in good faith, but it is not intended as a substitute for investigations, interpretations
Drop of Hammer on Drive Pipe	ln	or judgment of such authorized users. Information on the logs should not be relied upon without the geotechnical
Core Size	in	engineers recommendations contained in the report from which these logs were extracted.  Pu: Pocket Penetrometer: W/VI. Weight 1975.
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Soil descriptions represent a field identification after D. M. Burmister unless otherwise noted.

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0		No.	(ft)	(ft)	0/6	6/12	12/18	18/24	(In)	SOIL DESCRIPTION AND STRATIFICATION	SYMBOL
	SS	1	0.5	2.5	4	14	19	19	10	6" Asphalt	
						<del>                                     </del>		- 1		Top 5": Red-brown c-f SAND, some c-f Gravel, little Silt	
										Bottom 5": Gray c-f GRAVEL, some c-f Sand, trace Silt	
1_	<u> </u>	<u> </u>							_	Total of Chart And, Some Col Sand, trace Safe	
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1	SS	2	5	7	15	23	29	38	15	Red-brown m-f SAND, some Silt, little m-f Gravel w/	
	<u> </u>						<u> </u>			Weathered Rock	
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	SS	3	10	10	<del> </del>					i l	
	35		10	12	8	15	20	16	18	Red-brown clayey SILT, little fine Sand, trace Gravel w/ layers	
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	SS	4	15	17	13	i i	39	38	24	Red brown a COANTO	
									27	Red-brown c-f SAND, some c-f Gravel, little Silt w/ layers of Red-brown clayey SILT, little f Sand, trace Gravel	
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20										<u> </u>	
	SS	5	_20	22	50/2"		-	-	0	Rock in tip (Weathered Rock)	
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Nominal I.D. of Spilt Barrel Sampler	13% in	The subsurface information shown hereon was obtained for the design and estimating purposes for our client.
Weight/type of Hammer on Drive Pipe	300 lb	It is made available to authorized users only that they may have access to the same information available
Weight/type of Hammer on Split Barrel	140 lb	to our client. It is presented in good faith, but it is not intended as a substitute for investigations, interpretations
Drop of Hammer on Drive Pipe	in	or judgment of such authorized users. Information on the logs should not be relied upon without the geotechnical
Core Size	in	engineers recommendations contained in the report from which these logs were extracted.  Pp: Pocket Penetrometer; WOH: Weight of Hammer; WOR: Weight of Rod
Soil descriptions represent a Sold ideast		Approximate Change in Strata: Inferred Change in Strata:

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Nominal I.D. of Split Barrel Sampler	1% in	The subsurface information shown hereon was obtained for the design and estimating purposes for our client.
Weight/type of Hammer on Drive Pipe	300 lb	It is made available to authorized users only that they may have access to the same information available to our client. It is presented in good faith, but it is not in the same information available
Weight/type of Hammer on Split Barrel	140 lb	to our client. It is presented in good faith, but it is not intended as a substitute for investigations, interpretations or judgment of such authorized users. Information as the law is the such authorized users.
Drop of Hammer on Drive Pipe	ln	or judgment of such authorized users. Information on the logs should not be relied upon without the geotechnical engineers recommendations contained in the report from which these logs were extracted.
Core Size	in	Pp: Pocket Penetrometer; WOH: Weight of Hammer; WOR: Weight of Rod
Soil descriptions represent a field identification		Approximate Change in Strata: Inferred Change in Strata:

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										ROCK CORE: RUN 1 (12'-17')			
4=								]		RECOVERY = 42"/60" = 70%	<u> </u>		
15	<del>~</del>									$RQD = 0^{11}/60^{n} = 0\%$	<b> </b>		
ŀ										Gray medium to fine grained, slightly weathered, medium	<del> </del>		
` }										strong SANDSTONE w/ very closely spaced fractures	<del></del> -i		
ŀ										ROCK CORE: RUN 2 (17'-22')	<del> </del> -		
20										RECOVERY = $58^{\circ}/60^{\circ} = 96\%$	<b> </b>		
										RQD = 29"/ 60" = 48%	<b>——</b>		
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Nominal I.D. of Split Barrel Sampler	13% in	The subsurface information shown hereon was obtained for the design and estimating purposes for our client.
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	140 lb	to our client. It is presented in good faith, but it is not intended as a substitute for investigations, interpretations
Drop of Hammer on Drive Pipe	İn	or judgment of such authorized users. Information on the logs should not be relied upon without the geotechnical
Core Size	ln	engineers recommendations contained in the report from which these logs were extracted.  Pp: Pocket Penetrometer; WOH: Weight of Hammer; WOR: Weight of Rod
Soil descriptions represent a field identification	after D.	Approximate Change in Strate:

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DATE STARTED:			ONBUL	1 N G			LO	CATION:		M	onticelle NV							
Non-correct Column	BORI			E (1 8		<del>  -</del>	ATC OT		<u> </u>		GROUND ELEVATION: 147							
Depth   No.   Depth   Blows on Spoon   REC   OHY. NA   Date NA   24 Hr. NA   Date NA   Symbol   No.   OHY   OHY   No.   OHY   OHY   No.   OHY   OHY   No.   OHY   OHY   OHY   OHY   OHY   OHY   OHY   OHY   OHY   OHY   OHY   OHY   OHY   OHY   OHY   OHY   OHY   OHY   OHY   OHY   OHY   OHY   OHY   OHY   OHY   OHY   OHY   OHY   OHY   OHY   OHY   OHY   OHY   OHY   OHY   OHY   OHY   OHY   OHY   OHY   OHY   OHY   OHY   OHY   OHY   OHY   OHY   OHY   OHY   OHY   OHY   OHY   OHY   OHY   OHY   OHY   OHY   OHY   OHY   OHY   OHY   OHY   OHY   OHY   OHY   OHY   OHY   OHY   OHY   OHY   OHY   OHY   OHY   OHY   OHY   OHY   OHY   OHY   OHY   OHY   OHY   OHY   OHY   OHY   OHY   OHY   OHY   OHY   OHY   OHY   OHY   OHY   OHY   OHY   OHY   OHY   OHY   OHY   OHY   OHY   OHY   OHY   OHY   OHY   OHY   OHY   OHY   OHY   OHY   OHY   OHY   OHY   OHY   OHY   OHY   OHY   OHY   OHY   OHY   OHY   OHY   OHY   OHY   OHY   OHY   OHY   OHY   OHY   OHY   OHY   OHY   OHY   OHY   OHY   OHY   OHY   OHY   OHY   OHY   OHY   OHY   OHY   OHY   OHY   OHY   OHY   OHY   OHY   OHY   OHY   OHY   OHY   OHY   OHY   OHY   OHY   OHY   OHY   OHY   OHY   OHY   OHY   OHY   OHY   OHY   OHY   OHY   OHY   OHY   OHY   OHY   OHY   OHY   OHY   OHY   OHY   OHY   OHY   OHY   OHY   OHY   OHY   OHY   OHY   OHY   OHY   OHY   OHY   OHY   OHY   OHY   OHY   OHY   OHY   OHY   OHY   OHY   OHY   OHY   OHY   OHY   OHY   OHY   OHY   OHY   OHY   OHY   OHY   OHY   OHY   OHY   OHY   OHY   OHY   OHY   OHY   OHY   OHY   OHY   OHY   OHY   OHY   OHY   OHY   OHY   OHY   OHY   OHY   OHY   OHY   OHY   OHY   OHY   OHY   OHY   OHY   OHY   OHY   OHY   OHY   OHY   OHY   OHY   OHY   OHY   OHY   OHY   OHY   OHY   OHY   OHY   OHY   OHY   OHY   OHY   OHY   OHY   OHY   OHY   OHY   OHY   OHY   OHY   OHY   OHY   OHY   OHY   OHY   OHY   OHY   OHY   OHY   OHY   OHY   OHY   OHY   OHY   OHY   OHY   OHY   OHY   OHY   OHY   OHY   OHY   OHY   OHY   OHY   OHY   OHY   OHY   OHY   OHY   OHY   OHY   OHY   OHY   OHY   OHY   OHY   OHY   OHY   OHY   OHY   OHY   OHY   OHY   OHY   OHY   OHY   OHY   OHY   OHY   OHY   OHY					<u>-</u>						GROUNDWATER TABLE DEPTH: Approx, 16ft.							
REFORM   No.   FROM   TO   Slows on Spoon   REC   SOIL DESCRIPTION AND STRATIFICATION   SYMBOL			1	DE	PTH	DAIL	DATE COMPLETED:		DATE COMPLETED:		DATE COMPLETED:		DATE COMPLETED:		7/10	/2008	10 Hz 104 10-4	
SOIL DESCRIPTION AND STRATIFICATION   SYMBOL	1	1		<del></del>		┨	Blows	on Spoor	נ	REC								
SS	1		No.	<del> </del> -		O/6	6140	4040	40/04	<del> </del>	SOIL DESCRIPTION AND STRATIFICATION	SYMBOL						
SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME		98	<del></del>		_	<del>                                     </del>					<u> </u>							
5   SS   2   5   7   22   55   43   46   18   Red-Brown SILT, some fine Sand, trace Gravel    10   SS   3   10   12   18   75/2"   6   SAME    15   SS   4   15   17   27   60   75/2"   - 10   SAME (Weathered Rock)    16   SS   4   15   17   27   60   75/2"   - 10   SAME (Weathered Rock)    17   SROCVER RUN 1 (16.5"-21.5")   RECOVER RY = 32" / 60" = 53 %   ROCK CORE: RUN 2 (21.5"-23.5")   RECOVER RY = 19" / 24" = 79 %   RECOVER RY = 19" / 24" = 79 %   RECOVER RY = 19" / 24" = 79 %   RECOVER RY = 10" / 24" = 25%   RECOVER RY = 26" / 54" = 48 %   ROCK CORE: RUN 3 (23.5"-22.0")   RECOVER RY = 16" / 54" = 18 %   ROCK CORE: RUN 3 (23.5"-22.0")   RECOVER RY = 16" / 54" = 18 %   ROCK CORE: RUN 4 (28.6"-33.0")   RECOVER RY = 60" / 60" = 100 %   RECOVER RY = 60" / 60" = 100 %   RECOVER RY = 60" / 60" = 100 %   RECOVER RY = 60" / 60" = 100 %   RECOVER RY = 60" / 60" = 100 %   RECOVER RY = 60" / 60" = 100 %   RECOVER RY = 60" / 60" = 100 %   RECOVER RY = 60" / 60" = 100 %   RECOVER RY = 60" / 60" = 100 %   RECOVER RY = 60" / 60" = 100 %   RECOVER RY = 60" / 60" = 100 %   RECOVER RY = 60" / 60" = 100 %   RECOVER RY = 60" / 60" = 100 %   RECOVER RY = 60" / 60" = 100 %   RECOVER RY = 60" / 60" = 100 %   RECOVER RY = 60" / 60" = 100 %   RECOVER RY = 60" / 60" = 100 %   RECOVER RY = 60" / 60" = 100 %   RECOVER RY = 60" / 60" = 100 %   RECOVER RY = 60" / 60" = 100 %   RECOVER RY = 60" / 60" = 100 %   RECOVER RY = 60" / 60" = 100 %   RECOVER RY = 60" / 60" = 100 %   RECOVER RY = 60" / 60" = 100 %   RECOVER RY = 60" / 60" = 100 %   RECOVER RY = 60" / 60" = 100 %   RECOVER RY = 60" / 60" = 100 %   RECOVER RY = 60" / 60" = 100 %   RECOVER RY = 60" / 60" = 100 %   RECOVER RY = 60" / 60" = 100 %   RECOVER RY = 60" / 60" = 100 %   RECOVER RY = 60" / 60" = 100 %   RECOVER RY = 60" / 60" = 100 %   RECOVER RY = 60" / 60" = 100 %   RECOVER RY = 60" / 60" = 100 %   RECOVER RY = 60" / 60" = 100 %   RECOVER RY = 60" / 60" = 100 %   RECOVER RY = 60" / 60" = 100 %   RECOVER RY = 60" / 60" = 100 %   RECOVER RY = 60" / 60" = 100 %   RECOVER		<del></del>	<del></del> -		1-	<del>                                     </del>	<del>  8</del>	0	15	20	4 ·							
SS   2   5   7   22   55   43   46   18	1				<del>                                     </del>	╁─	<del> </del> -	<del> </del>			Brown coarse to fine SAND, little Silt, trace Gravel							
SS   2   5   7   22   55   43   46   18					<del> </del> -	<del> </del>	<del></del>	<del> </del>			ļ							
10	5				<del> </del>	<del> </del>	<del> </del>											
10		SS	2	5	7	22		12	46									
SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME					<del>                                     </del>		33	43	40	18	Red-Brown SILT, some fine Sand, trace Gravel							
SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME   SAME					-	<del> </del> -	<del> </del>	<del>   </del>		<del> </del> -	]							
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15		SS	3	10	12	18	75/2"	<del>                                     </del>			<u></u>							
SS   4   15   17   27   60   75/2"   10   SAME (Weathered Rock)						<u> </u>	1312			- 6	JSAMB							
SS   4   15   17   27   60   75/2"   10   SAME (Weathered Rock)								$\vdash$			[							
SS   4   15   17   27   60   75/2"   10   SAME (Weathered Rock)								<del>  </del>		<del></del> -								
20   Im ROCK CORE: RUN 1 (16.5'-21.5')   Im ROCK CORE: RUN 1 (16.5'-21.5')   RECOVERY = 32"   60" = 53 %     ROCK CORE: RUN 2 (21.5'-23.5')   RECOVERY = 19"   24" = 79 %     RECOVERY = 10"   24" = 25%     RECOVERY = 25"   ROCK CORE: RUN 2 (21.5'-23.5')   RECOVERY = 10"   74" = 79 %     RECOVERY = 10"   74" = 79 %     RECOVERY = 26"   754" = 48 %     RECOVERY = 26"   754" = 48 %     RECOVERY = 26"   754" = 48 %     RECOVERY = 26"   754" = 48 %     RECOVERY = 26"   754" = 48 %     RECOVERY = 26"   754" = 48 %     RECOVERY = 26"   754" = 11%     RECOVERY = 26"   754" = 48 %     RECOVERY = 26"   754" = 48 %     RECOVERY = 26"   754" = 10 %     RECOVERY = 60"   760" = 100 %     RECOVERY = 60"   760" = 100 %     RECOVERY = 60"   760" = 58%     RECOVERY = 60"   760" = 58%     RECOVERY = 60"   760" = 58%     RECOVERY = 60"   760" = 58%     RECOVERY = 60"   760" = 58%     RECOVERY = 60"   760" = 58%     RECOVERY = 60"   760" = 58%     RECOVERY = 60"   760" = 100 %     RECOVERY = 60"   760" = 100 %     RECOVERY = 60"   760" = 100 %     RECOVERY = 60"   760" = 100 %     RECOVERY = 60"   760" = 100 %     RECOVERY = 60"   760" = 100 %     RECOVERY = 60"   760" = 100 %     RECOVERY = 60"   760" = 100 %     RECOVERY = 60"   760" = 100 %     RECOVERY = 60"   760" = 100 %     RECOVERY = 60"   760" = 100 %     RECOVERY = 60"   760" = 100 %     RECOVERY = 60"   760" = 100 %     RECOVERY = 60"   760" = 100 %     RECOVERY = 60"   760" = 100 %     RECOVERY = 60"   760" = 100 %     RECOVERY = 60"   760" = 100 %     RECOVERY = 60"   760" = 100 %     RECOVERY = 60"   760" = 100 %     RECOVERY = 75 %     RECOVERY = 75 %     RECOVERY = 75 %     RECOVERY = 75 %     RECOVERY = 75 %     RECOVERY = 75 %     RECOVERY = 75 %     RECOVERY = 75 %     RECOVERY = 75 %     RECOVERY = 75 %     RECOVERY = 75 %     RECOVERY = 75 %     RECOVERY = 75 %     RECOVERY = 75 %     RECOVERY = 75 %     RECOVERY = 75 %     RECOVERY = 75 %     RECOVERY = 75 %     RECOVERY = 75 %     RECOVERY = 75 %     RECOVERY = 75 %     RECOVERY = 75 %     RECOVERY = 75 %	15																	
1 m   ROCK CORE: RUN 1 (16.5°-21.5°)   RECOVERY = 32" / 60" = 53 %   RQD = 0" / 60" = 0%   Red-Brown coarse to fine grained, highly weathered SANDSTONE   1.5m   RECOVERY = 30" / 60" = 53 %   ROCK CORE: RUN 2 (21.5°-23.5°)   RECOVERY = 19" / 24" = 79 %   RCD = 6" / 24" = 25%   RADDSTONE   RECOVERY = 10" / 24" = 25%   RADDSTONE   RECOVERY = 26" / 54" = 48 %   RCD = 6" / 54" = 11%   RECOVERY = 26" / 54" = 48 %   RCD = 6" / 54" = 11%   RECOVERY = 26" / 54" = 11%   RECOVERY = 26" / 54" = 11%   RECOVERY = 26" / 54" = 11%   RECOVERY = 26" / 54" = 11%   RECOVERY = 26" / 54" = 11%   RECOVERY = 26" / 54" = 11%   RECOVERY = 26" / 54" = 11%   RECOVERY = 26" / 54" = 11%   RECOVERY = 26" / 54" = 11%   RECOVERY = 60" / 60" = 100 %   RCDC CORE: RUN 4 (28.0°-33.0°)   RECOVERY = 60" / 60" = 100 %   RCDC CORE: RUN 4 (28.0°-33.0°)   RECOVERY = 60" / 60" = 58%   RECOVERY = 60" / 60" = 58%   RECOVERY = 60" / 60" = 58%   RECOVERY = 60" / 60" = 58%   RECOVERY = 60" / 60" = 58%   RECOVERY = 60" / 60" = 58%   RECOVERY = 60" / 60" = 100 %   RCDC CORE: RUN 4 (28.0°-33.0°)   RECOVERY = 60" / 60" = 58%   RECOVERY = 60" / 60" = 58%   RECOVERY = 60" / 60" = 58%   RECOVERY = 60" / 60" = 58%   RECOVERY = 60" / 60" = 58%   RECOVERY = 60" / 60" = 58%   RECOVERY = 60" / 60" = 58%   RECOVERY = 60" / 60" = 58%   RECOVERY = 60" / 60" = 58%   RECOVERY = 60" / 60" = 58%   RECOVERY = 60" / 60" = 58%   RECOVERY = 60" / 60" = 58%   RECOVERY = 60" / 60" = 58%   RECOVERY = 60" / 60" = 58%   RECOVERY = 60" / 60" = 58%   RECOVERY = 60" / 60" = 58%   RECOVERY = 60" / 60" = 58%   RECOVERY = 60" / 60" = 58%   RECOVERY = 60" / 60" = 58%   RECOVERY = 60" / 60" = 58%   RECOVERY = 60" / 60" = 58%   RECOVERY = 60" / 60" = 58%   RECOVERY = 60" / 60" = 58%   RECOVERY = 60" / 60" = 58%   RECOVERY = 60" / 60" = 58%   RECOVERY = 60" / 60" = 58%   RECOVERY = 60" / 60" = 58%   RECOVERY = 60" / 60" = 58%   RECOVERY = 60" / 60" = 58%   RECOVERY = 60" / 60" = 58%   RECOVERY = 60" / 60" = 58%   RECOVERY = 60" / 60" = 58%   RECOVERY = 60" / 60" = 58%   RECOVERY = 60" / 60" = 5		22	4	15	17	27	60	75/2"	ᅳᅥ	- 10								
1m	[							1,5,7,2		10	SAME (Weathered Rock)							
1m							_	<del></del>		Im	POOR CONT. PURIL CO.							
1.25m   RQD = 0" / 60" = 0%   Red-Brown coarse to fine grained, highly weathered SANDSTONE   1.5m   ROCK CORE: RUN 2 (21.5¹-23.5')   RECOVERY = 19" / 24" = 79 %   RQD = 6" / 24* = 25%   Red-Brown medium to fine grained, highly weathered   SANDSTONE   2.5m   RECOVERY = 26" / 54" = 48 %   RQD = 6" / 54* = 48 %   RQD = 6" / 54* = 11%   Red-Brown medium to fine grained, highly weathered   SANDSTONE   2.5m   RECOVERY = 26" / 54" = 48 %   RQD = 6" / 54* = 11%   Red-Brown medium to fine grained, highly weathered   SANDSTONE   2.5m   RECOVERY = 60" / 60" = 100 %   RQD = 35" / 60" = 58%   Red-Brown medium to fine grained, slightly weathered   SANDSTONE   RECOVERY = 60" / 60" = 58%   Red-Brown medium to fine grained, slightly weathered   SANDSTONE   BORING COMPLTE AT 33.0'											PECOVERY - 2011 ( COIL - 2011)							
2m	20								$\neg \neg$		ROD 04 / 504 00/							
2.5m   ROCK CORE: RUN 2 (21.5'-23.5')   RECOVERY = 19" / 24" = 79 %   RQD = 6" / 24" = 25%   RQD = 6" / 24" = 25%   RADSTONE   ROCK CORE: RUN 3 (23.5'-28.0')   RECOVERY = 26" / 54" = 48 %   RODSTONE   ROCK CORE: RUN 4 (28.0'-33.0')   RECOVERY = 60" / 60" = 100 %   ROCK CORE: RUN 4 (28.0'-33.0')   RECOVERY = 60" / 60" = 58%   ROCK CORE: RUN 4 (8.0'-33.0')   RECOVERY = 60" / 60" = 58%   ROCK CORE: RUN 4 (8.0'-33.0')   RECOVERY = 60" / 60" = 58%   ROCK CORE: RUN 4 (8.0'-33.0')   RECOVERY = 60" / 60" = 58%   ROCK CORE: RUN 4 (8.0'-33.0')   RECOVERY = 60" / 60" = 58%   ROCK CORE: RUN 4 (8.0'-33.0')   RECOVERY = 60" / 60" = 58%   ROCK CORE: RUN 4 (8.0'-33.0')   RECOVERY = 60" / 60" = 58%   ROCK CORE: RUN 4 (8.0'-33.0')   RECOVERY = 60" / 60" = 58%   ROCK CORE: RUN 4 (8.0'-33.0')   RECOVERY = 60" / 60" = 58%   ROCK CORE: RUN 4 (8.0'-33.0')   RECOVERY = 60" / 60" = 58%   ROCK CORE: RUN 4 (8.0'-33.0')   RECOVERY = 60" / 60" = 58%   ROCK CORE: RUN 4 (8.0'-33.0')   RECOVERY = 60" / 60" = 58%   ROCK CORE: RUN 4 (8.0'-33.0')   RECOVERY = 60" / 60" = 58%   ROCK CORE: RUN 4 (8.0'-33.0')   RECOVERY = 60" / 60" = 58%   ROCK CORE: RUN 4 (8.0'-33.0')   RECOVERY = 60" / 60" = 58%   ROCK CORE: RUN 4 (8.0'-33.0')   RECOVERY = 60" / 60" = 58%   ROCK CORE: RUN 4 (8.0'-33.0')   RECOVERY = 60" / 60" = 58%   ROCK CORE: RUN 4 (8.0'-33.0')   RECOVERY = 60" / 60" = 58%   ROCK CORE: RUN 4 (8.0'-33.0')   RECOVERY = 60" / 60" = 58%   ROCK CORE: RUN 4 (8.0'-33.0')   RECOVERY = 60" / 60" = 58%   ROCK CORE: RUN 4 (8.0'-33.0')   ROCK CORE: RUN 4 (8.0'-33.0')   ROCK CORE: RUN 4 (8.0'-33.0')   ROCK CORE: RUN 4 (8.0'-33.0')   ROCK CORE: RUN 4 (8.0'-33.0')   ROCK CORE: RUN 4 (8.0'-33.0')   ROCK CORE: RUN 4 (8.0'-33.0')   ROCK CORE: RUN 4 (8.0'-33.0')   ROCK CORE: RUN 4 (8.0'-33.0')   ROCK CORE: RUN 4 (8.0'-33.0')   ROCK CORE: RUN 4 (8.0'-33.0')   ROCK CORE: RUN 4 (8.0'-33.0')   ROCK CORE: RUN 4 (8.0'-33.0')   ROCK CORE: RUN 4 (8.0'-33.0')   ROCK CORE: RUN 4 (8.0'-33.0')   ROCK CORE: RUN 4 (8.0'-33.0')   ROCK CORE: RUN 4 (8.0'-33.0')   ROCK CORE: RUN 4																		
2.5m   2m   ROCK CORE: RUN 2 (21.5'-23.5')   RECOVERY = 19" / 24" = 79 %   RQD = 6" / 24" = 25%   RQD = 6" / 24" = 25%   Red-Brown medium to fine grained, highly weathered   SANDSTONE   RECOVERY = 26" / 54" = 48 %   RQD = 6" / 54" = 11%   RQD = 6" / 54" = 11%   RQD = 6" / 54" = 11%   RECOVERY = 26" / 54" = 10 %   RECOVERY = 60" / 60" = 100 %   RECOVERY = 60" / 60" = 100 %   RECOVERY = 60" / 60" = 58%   Red-Brown medium to fine grained, slightly weathered   SANDSTONE   RECOVERY = 60" / 60" = 58%   Red-Brown medium to fine grained, slightly weathered   SANDSTONE   RECOVERY = 60" / 60" = 58%   Red-Brown medium to fine grained, slightly weathered   SANDSTONE   BORING COMPLTE AT 33.0'											Toda-Drown coarse to time grained, highly weathered SANDSTONE							
25   2m   RECOVERY = 19" / 24" = 79 %   RQD = 6" / 24" = 25%   Red-Brown medium to fine grained, highly weathered   SANDSTONE   SANDSTONE   Red-Brown medium to fine grained, highly weathered   SANDSTONE   Recovery = 26" / 54" = 48 %   RQD = 6" / 54" = 11%   Red-Brown medium to fine grained, highly weathered   SANDSTONE   Recovery = 60" / 60" = 100 %   Recovery = 60" / 60" = 58%   Red-Brown medium to fine grained, slightly weathered   SANDSTONE   Red-Brown medium to fine grained, slightly weathered   SANDSTONE   Red-Brown medium to fine grained, slightly weathered   SANDSTONE   Red-Brown medium to fine grained, slightly weathered   SANDSTONE   Red-Brown medium to fine grained, slightly weathered   SANDSTONE   Red-Brown medium to fine grained, slightly weathered   SANDSTONE   Red-Brown medium to fine grained, slightly weathered   SANDSTONE   Red-Brown medium to fine grained, slightly weathered   SANDSTONE   Red-Brown medium to fine grained, slightly weathered   SANDSTONE   Red-Brown medium to fine grained, slightly weathered   SANDSTONE   Red-Brown medium to fine grained, slightly weathered   SANDSTONE   Red-Brown medium to fine grained, slightly weathered   SANDSTONE   Red-Brown medium to fine grained, slightly weathered   SANDSTONE   Red-Brown medium to fine grained, slightly weathered   SANDSTONE   Red-Brown medium to fine grained, slightly weathered   Red-Brown medium to fine grained, slightly weathered   Red-Brown medium to fine grained, slightly weathered   Red-Brown medium to fine grained, slightly weathered   Red-Brown medium to fine grained, slightly weathered   Red-Brown medium to fine grained, slightly weathered   Red-Brown medium to fine grained, slightly weathered   Red-Brown medium to fine grained, slightly weathered   Red-Brown medium to fine grained, slightly weathered   Red-Brown medium to fine grained, slightly weathered   Red-Brown medium to fine grained, slightly weathered   Red-Brown medium to fine grained, slightly weathered   Red-Brown medium to fine grained, slightly weathered   Red-Brown											ROCK CORREDING (21 Steep SP							
2m	` <u>}</u>										RECOVERY = 10" / 24" - 70 %							
1.75m   Red-Brown medium to fine grained, highly weathered   SANDSTONE	25									2m	ROD = 6" / 24" = 25%							
2.5m   SANDSTONE	-																	
2.5m   ROCK CORE: RUN 3 (23.5'-28.0')	Ļ									2.25m	SANDSTONE -							
30	-									2.5m	ROCK CORE: RUN 3 (23.51-28 01)							
1.75m   RQD = 6" / 54" = 11%   Red-Brown medium to fine grained, highly weathered   SANDSTONE   ROCK CORE: RUN 4 (28.0'-33.0')   RECOVERY = 60" / 60" = 100 %   RQD = 35" / 60" = 58%   Red-Brown medium to fine grained, slightly weathered   SANDSTONE   BORING COMPLTE AT 33.0'	.,  -									.75m	RECOVERY = 26" / 54" = 48 %							
2m   Red-Brown medium to fine grained, highly weathered   SANDSTONE   ROCK CORE: RUN 4 (28.0'-33.0')   RECOVERY = 60" / 60" = 100 %   RQD = 35" / 60" = 58%   Red-Brown medium to fine grained, slightly weathered   SANDSTONE   BORING COMPLTE AT 33.0'	30									1.75m	$RQD = 6^{u} / 54^{u} = 11%$							
SANDSTONE     SANDSTONE     ROCK CORE: RUN 4 (28.0'-33.0')     RECOVERY = 60" / 60" = 100 %   RQD = 35" / 60" = 58%   Red-Brown medium to fine grained, slightly weathered   SANDSTONE   BORING COMPLTE AT 33.0'	- }																	
2m   ROCK CORE: RUN 4 (28.0'-33.0')     2.5m   RECOVERY = 60" / 60" = 100 %     RQD = 35" / 60" = 58%     Red-Brown medium to fine grained, slightly weathered     SANDSTONE     BORING COMPLTE AT 33.0'	F								$\Box$	2m	SANDSTONE							
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Red-Brown medium to fine grained, slightly weathered  SANDSTONE  BORING COMPLTE AT 33.0'	33					.						<u></u>						
ISANDSTONE   BORING COMPLTE AT 33.0'	-	-10.										——						
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Nominal I.D. of Hole	ſn	The subsurface information shows because
Nominal I.D. of Spilt Barrel Sampler	1% in	The subsurface information shown hereon was obtained for the design and estimating purposes for our client.
Weight/type of Hammer on Drive Pipe	300 lb	It is made available to authorized users only that they may have access to the same information available to our client. It is presented in good faith but it is not at a same information available
Weight/type of Hammer on Split Barrel	140 lb	to our client. It is presented in good faith, but it is not intended as a substitute for investigations, interpretations
Drop of Hammer on Drive Pipe	in	or judgment of such authorized users. Information on the logs should not be relied upon without the geotechnica engineers recommendations contained in the report from which these logs were extracted.
Core Size	in	Pp: Pocket Penetrometer; WOH: Weight of Hammer; WOR: Weight of Rod
Soil descriptions represent a field identification	after D	Approximate Change in Strate:

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		うに	5		-		TNAME			CONCORD BORING NO. SB-2	BORING NO. SB-24		
		ENGINE Chrinand	FING			ĽO	CATION	<b>-</b>	M	Monticello, NY JOB NO. 5485			
BOR	NG BY: G		4110		<del> </del>	ATE OT	ARTED:			GROUND ELEVATION: 1482			
	ECTOR: 0						LETED:		/2008	GROUNDWATER TABLE DEPTH: Approx. 16ft.			
DEPTI			DE	PTH	DAIL	COM	LE IED:	7/10	/2008	0 Hr. N/A Date N/A 24 Hr. N/A Date N/A			
(ft)	METHOD	SAMPLE	FROM		1	Blows	on Spoo	n	REC	·			
0		No.	(ft)	(ft)	0/6	6/12	12/18	40/04		SOIL DESCRIPTION AND STRATIFICATION S	YMBOL		
	SS	1	0	2	5	8		18/24	(in)	<u></u>			
			<u> </u>	+-	<del> </del>	<del>                                     </del>	10	10	12	6" Topsoil			
			<del> </del>	<del> </del> -	<del>                                      </del>					Brown coarse to fine SAND, little Silt, trace Gravel			
				<del>                                     </del>	<del> </del> -	┨	┼			-			
_ 5					<del> </del>	<del> </del>	<del>├</del> ─┈						
	SS	2	5	7	44	52	75/5"						
}				<u> </u>	<u> </u>	- 32	1313		10	Red-Brown SILT, little medium to fine Sand, little coarse to fine			
						<del> </del> -				Gravel (weathered Sandstone)			
						<del></del>				-l			
10						<del> </del> -	<del> </del>	<del></del> -		Weathered Rock (auger grinding)			
	SS	3	10	12	69	72	75/4"	<del>  </del>		- CANGE			
1							70/4		12	SAME			
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Nominal I.D. of Hole	ir	The subsurface information at any
Nominal I.D. of Split Barrel Sampler	1% ir	The subsurface information shown hereon was obtained for the design and estimating purposes for our client.
		4 When a railed to audiorized users only that they may have appear to the service of
Weight/type of Hammer on Split Barrel		The state of the prescrited in 8000 1200, but if is not intended as a sub-time of the
Drop of Hammer on Drive Pipe		a book deposited users. Information on the long should not be self-a
Core Size	in	I which there loss warmen a straight and the second warmen and the second warmen and the second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second second seco
Soil descriptions represent a field identification	after D	Pp: Pocket Penetrometer; WOH: Weight of Hammer; WOR: Weight of Rod  Approximate Change in Strata:  Inferred Change in Strata:  M. Burmister unless otherwise noted.

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	S)				<u> </u>		CATION			CONCORD BORING NO. SE	3-25
4		<u> </u>	ΤΙΝ (A) Ε (R 8)				J. 111011	` <del> </del>		Ionticello, NY JOB NO. 54	185
	NG BY: (				D	ATE ST	ARTED	7/1	0/2008	GROUND ELEVATION: 14	87 <u>+</u>
	ECTOR:	CDM			DATE	E COMF	LETED		0/2008	GROUNDWATER TABLE DEPTH: NE  0 Hr. N/A Date N/A 24 Hr. N/A Date N/A	
OEPTI-	METHOD	SAMPLE		PTH	4	Blows	on Spoo		REC	Tate W	A. 7
0		No. FROM TO					SOIL DESCRIPTION AND STRATIFICATION	SYMBOL			
Ť	ss	1	(ft) 0	(ft) 2	0/6		12/18	18/24	<del>\```</del>	<u>'                                     </u>	SIMBOL
1		<del> </del> -	<del>  ~ -</del>	<del>                                     </del>	<del>                                     </del>	8	8	30	8	4" Topsoil	<del>                                     </del>
				<del>                                     </del>	1	<del>                                     </del>	<del> </del>		<del> </del>	Brown medium to fine SAND, little Silt, trace Gravel	<del></del>
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	SS	3	10	12	28	33	22		ļ	·	
<u> </u>		<del>                                     </del>			26	33	37	41	20	Red-Brown SILT, little medium to fine Sand, little coarse to	
									<del> </del>	fine Gravel (Gray weathered Sandstone)	
15										· ·	
- 1	22	4	15	17	22	31	43	75/3"	19	Red-Brown SILT, little medium to fine Sand, trace Gravel	
· •										(Gray weathered Sandstone)	
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20									1.5		
									lm	ROCK CORE: RUN 1 (18.5'-23.5')	
-									2m	RECOVERY = 60"/60" = 100% RQD = 49"/60" = 82%	
-									2.25m	Gray coarse grained, slightly weathered SANDSTONE	
25							$\Box$		2m	State of State of State of SandSTONE	
										BORING COMPLETE AT 23.5 FEET	
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Nominal I.D. of Hole	in	The subsurface information above 1
Nominal I.D. of Split Barrel Sampler	1% in	The subsurface information shown hereon was obtained for the design and estimating purposes for our client.
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Weight/type of Hammer on Split Barrel	140 lb	to our client. It is presented in good faith, but it is not intended as a substitute for investigations, interpretations
Orop of Hammer on Drive Pipe	ln	or judgment of such authorized users. Information on the logs should not be relied upon without the geotechnical
Core Size	in	I the report from which these loss were extracted
· · · · · · · · · · · · · · · · · · ·		Pp: Pocket Penetrometer; WOH: Weight of Hammer; WOR: Weight of Rod  Approximate Change in Strata: Inferred Change in Strata:
oil descriptions represent a field identification	ofter 13	Approximate Change in Strata: Inferred Change in Strata:

	77 (					PRO IFO	T NAME			5485 BORING LOG \$B-2	ek.8
					<b> </b>		CATION			CONCORD BORING NO. SE	 3-26
		<b>三日の11日</b> 11日日日 11日日日	TINE.		1			<u> </u>	IV	ORLICEIIO, NY JOB NO. 54	485
	NG BY: (				1	DATE ST	FARTED	7/10	0/2008	GROUND ELEVATION: 15	00 <u>+</u>
INSPE	CTOR:	CDM					PLETED	<del> </del>	/2008	GROUNDWATER TABLE DEPTH: NE	
DEPTH	ļ	SAMPLE	DE	PTH					$\overline{}$	0 Hr. N/A Date N/A 24 Hr. N/A Date N/A	A
(ft) METHO		No.	FROM	ТО	7	Blows	on Spoo	n	REC	SOIL DESCRIPTION	
0			(ft)	(ft)	0/6	6/12	12/18	18/24	(in)	SOIL DESCRIPTION AND STRATIFICATION	SYN
	102.1 (11)								Fill: Mice CAD dalast (		
		<del> </del>		<u> </u>					† <del></del>	Fill: Misc. C+D debris (concrete, cinder blocks, rebar etc.)	
	<del></del> -	<del> </del>	<u> </u>	<u> </u>	<u> </u>				† <del></del>		
5		ļ			<u> </u>						<u> </u>
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- 1	28	1	5	7	10	9	8	13	6	Brown coarse to fine SAND, little Silt, little coarse to fine	
Í		<del> </del>	<del> </del>	<del></del>	<del> </del> -	<u> </u>				Gravel	
ŀ			<del> </del> — —	<u> </u>	<b>├</b>	ļ	<u> </u>				<b>—</b> —
10		<del>                                      </del>		<u></u>	<b></b> -	<del> </del>					
	SS	2	10	12			<b></b> -				
<u> </u>		<del>  _</del>	10	12	38	75/1"			10	Brown SILT, little coarse to fine Sand, trace Gravel	<del></del>
ļ						├	<u> </u>			(Gray weathered Sandstone)	
Γ						<del> </del>			1.25m	ROCK CORB: RUN 1 (12'-17')	
15									1.25m	RECOVERY = 33"/60" = 55%	
				,					RQD = 4"/60" = 7%		
					<del></del>				1m	Gray coarse grained, slightly weathered SANDSTONE	
									1.5m	POOT GODD	
							<del></del>		1.500	ROCK CORE: RUN 2 (17'-22') RECOVERY = 58"/60" = 97%	
20	/								1.25m	RQD = 29.5"/60" = 49%	
$\vdash$	<del>-</del>										
F									lm	Gray coarse grained, slightly weathered SANDSTONE	
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ninal C	D. of Ho	<del></del> _									
				_	in T	ne subsur	face info	mation s	hown he	seon was obtained for the design and estimating purposes for our client.	
abtitus	o. u. op)	it Barrel Si nmer on D	mpler	1	≯ in It	is made a	vailable (	o author	ized use	s only that they may have access to the same information available	ı

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Weight/type of Hammer on Split Barrel		
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Soil descriptions represent a field identification	n after D	Pp: Pocket Penetrometer; WOH: Weight of Hammer; WOR: Weight of Rod  Approximate Change in Strata: Inferred Change in Strata:  M. Burmister unless otherwise and the
		ATTENDED OFFICE MISE HOREOF

	177 6		(C) (I		Р	ROJEC	TNAME			5485 BORING LOG SB-2	27.xds
		シニ・			<u>:</u>		CATION			CONCORD BORING NO. SE	B-27
		ENGINE OMBUÉ.	FING Cho				OATION	├─-	M	onticello, NY JOB NO, 56	485
BORII	NG BY: G			<del></del>		ATE OT	ARTED	<del> </del>		GROUND ELEVATION: 14	97 ±
	CTOR: C								/2008	GROUNDWATER TABLE DEPTH: NE	
DEPTH			DEF	PTH	DAII	COM	LETED:	7/11	/2008	0 Hr. N/A Date N/A 24 Hr. N/A Date N/	/4
(ft)	METHOD	SAMPLE	FROM		-1	Blows	on Spoo	n	REC	i	<del>;</del> -
0	<u> </u>	No.	(ft)	(ft)	0/6	1 040	10115		<u> </u>	SOIL DESCRIPTION AND STRATIFICATION	SYMB
	SS	1	0	-	-}	6/12	12/18	18/24	(in)	<u></u>	1011412
ł	<del></del>		<del>ا -</del>	2	26	31	47	40	12"	Red-brown/Orange-brown mottled coarse to fine SAND, little	┿
	<del>                                     </del>				<del> </del> -	<del> </del>	<b> </b>			Silt, little coarse to fine Gravel	<u> </u>
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5				٠		<u> </u>	<u> </u>				╁—-
-					<b> </b>						
	SS	2	5	7	46	75/2°	<u> </u>	-	6"	Brown coarse to fine SAND and coarse Gravel, trace Silt	<b>├</b> ——
					ļ					and coalse Graver, trace Silt	<u> </u>
					<u> </u>					Auger grinding (weatherered Sandstone)	
40										Samuel Committee (According to Committee)	<u> </u>
10					<u> </u>				lm	ROCK CORE: RUN 1 (9'-14')	ļ <u>.</u>
ŀ					<u> </u>	<u> </u>			lm	RECOVERY = 60"/60" = 100%	
- 1									1.25m	$RQD = 34^{11}/60^{11} \Rightarrow 57\%$	
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Į.									lm	Gray coarse grained, slightly weathered SANDSTONE	
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										BORING COMPLETE AT 14.0 FEET	
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Nominal I.D. of Hole	in	The subsurface information shows because
Nominal I.D. of Split Barrel Sampler	1% in	The subsurface information shown hereon was obtained for the design and estimating purposes for our client.
Weight/lype of Hammer on Drive Pipe	300 lb	It is made available to authorized users only that they may have access to the same information available to our client. It is presented in good faith, but it is not in the design and estimating purposes for our client.
Weight/type of Hammer on Split Barrel	140 lb	to our client. It is presented in good faith, but it is not intended as a substitute for investigations, interpretations
Drop of Hammer on Drive Pipe	in	or judgment of such authorized users. Information on the logs should not be relied upon without the geotechnical engineers recommendations contained in the revert S.
Core Size	ln	engineers recommendations contained in the report from which these logs were extracted.  Pp: Pocket Penetrometer; WOH: Weight of Hammer; WOR: Weight of Rod
Soil descriptions represent a field identification	_ ^ _	Approximate Change in Strata: Inferred Change in Strata:
Soil descriptions represent a field identification	1 atter D.	. M. Burmister unless otherwise noted.
•		FIGURE 28
		Page 1 of :

		SE.		P	ROJEC				CONCORD BORING NO. SH 20	
			TING			ro	CATIO!	V:	ſ,	Monticello, NY JOB NO
BORII	NG BY: G	ENGINE RI	8 11 2		<del>  _</del>					GPOUND ELEVATION
	CTOR: (					ATE ST			14/2008	GROUNDWATER TABLE DEPTH: NE
DEPTH		<u> </u>	DE	PTH	DATI	E COMF	LETEC	):] 7/	14/2008	IU Hr. N/A IDate viv. Inch.
(ft)	METHOD	SAMPLE	FROM		Blows on Spoor			תכ	REC	Date N/A
o	ļ	No.			0/2	<del></del>				SOIL DESCRIPTION AND STRATIFICATION
	SS	1	(ft)	(ft) 2	0/6	6/12	12/18	18/2	4 (in)	SYME
	<del></del>	<del></del>	<del>                                     </del>	<del> </del>	9	17	22	15	12	4" Topsoil
				<del> </del>		<del> </del>	<u> </u>	<del> </del>		Brown coarse to fine SAND, little Silt, little coarse to fine
j				╂		<del> </del>	<u> </u>	<u> </u>		Gravel Gravel
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	SS	2	- 5	7		-	<u> </u>	<del> </del>		]
j				<del> </del>	17	75/5"	<del>-</del>	<del>  -:-</del>	11	Red-brown coarse to fine SAND, little Silt, little coarse to
- 1				<del>  </del>		<u> </u>		<del> </del> -		fine Gravel (weathered SANDSTONE)
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10									<u> </u>	<u> </u>
	SS	3	10	12	20	41			ļ	<b>↓</b>
				<del></del> -		41	52	32	12	Same
Ĺ										Auger grinding (weathered SANDSTONE)
ļ_									}	<b>-</b>
15									<del>  </del>	
<u> </u>									lm	ROCK CORE: RUN1 (13.5-18.5')
_									1.25m	RECOVERY = 51"/60" = 85%
-		$1.25m \text{ RQD} = 26^{\text{H}}/60^{\text{H}} = 43\%$	RQD = 26"/60" = 43%							
~  -									1.25m	Gray coarse to fine grained, slightly weathered SANDSTONE
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-	<del></del>				$\Box$					BORING COMPLETE AT 18.5 FEET
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nal I.D	ral I.D. of Split Barrel Sampler 1% in It is made available to								inown hei	ereon was obtained for the design and estimating purposes for our client.
ntiype	The state of the Pipe 300 lb to our client. It is present									and the same information available
nvtype	of Hamn	er on Spl	it Barrel	140	lb or ju	dgment	of such	authoria	44 guod 18	is only that they may have access to the same information available faith, but it is not intended as a substitute for investigations, interpretations. Information on the logs should not be relied upon without the geotechnical
or Han Size	uner on L	Orive Pipe		1 -	Inlengi	tieetc ron		3.00	40015.	. Lacormation on the logs should not be relied upon without the

Welster War and American Drive Pipe	300 lb to our client. It is presented in good fairly have access to it	e same information available
Weight/type of Hammer on Split Barrel	300 lb to our client. It is presented in good faith, but it is not intended as a substit	le for investigations interna
Drop of Hammer on Drive Pipe		
Core Size		
Soil descriptions represent a field identification	Approximate Change in Strata: Inferred Change in Strata: Inferred Change in Strata: Inferred Change in Strata: Inferred Change in Strata: FIGURE 29	of Rod

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			====			-		CATION			CONCORD BORING NO. SB-29				
4		CONS	ULI	TING Eng		]	LC	CAHON	` <del> </del>	M	onticello, NY JOB NO. 5485				
BORI	NG BY:						DATE O	TARTED	+		GROUND ELEVATION: 1492				
	ECTOR:							PLETED		/2008	GROUNDWATER TABLE DEPTH: NF				
DEPT			_	DE	PTH	100				/2008	0 Hr. N/A Date N/A 24 Hr. N/A Date N/A				
(ft)	METHO	SAM		FROM		$\dashv$	Blows	on Spoo	n	REC	1				
0		No	э.	(ft)	(ft)		0/6 6/12 12/18		18/24	<u> </u>	SOIL DESCRIPTION AND STRATIFICATION SY				
			_	<del></del>	1117	0/0	0/6 6/12 12/18			(in)					
		<del>                                     </del>	_		<del> </del>	╅┈	-	┼	<del> </del>	<del> </del> -	4" Asphalt				
		<b>—</b>			<del> </del>	+	┿	+	<del> </del> -	<b></b> -	Fill: Brown coarse to fine SAND, some coarse to fine Gravel, little				
		<del> </del>			<del>                                     </del>	╁	╁	<del>- </del> -	├—	<del> </del> -	Silt, trace Asphalt				
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	SS	1		5	7	16	23	29	50/11	<del> </del> _					
						<del>  ```</del>	1-2	+ 29	50/1"	19	Red-Brown medium to fine SAND, little Silt, little medium to				
						<del> </del> -	┪	<del> </del> -		<u> </u>	fine Gravel				
						- <del></del>	┿	┼			<del> </del>				
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	\$5	2	7	10	12	29	54	75/4"			<u> </u>				
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[						┼	┼──	╁╌┯┤							
15			_			┼──	<del> </del>	┢━┪			auger grinding (weathered sandstone)				
$\neg$	SS	3		15	17	30	75/I"	<del>  </del>			<u></u>				
			7			<del>                                     </del>	1	<del>                                     </del>		6	Red-Brown/Orange mottled coarse to fine SAND, little Silt, little				
Į						<u> </u>	† <del></del> -	<del>                                     </del>			medulm to fine Grayel.				
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				mele-		<u> n</u>  1	he subsu	rface info	rmation ;	shown h	creon was obtained for the design and estimating purposes for our client.				
iaht/tv	hinal I.D. of Spilt Barrel Sampler pht/type of Hammer on Drive Pipe 3														
	htt/vne of Hammer on Spill Bornel						% in it is made available to authorized users only that they may have access to the same information available to our client. It is presented in good faith, but it is not intended as a substitute for investigations internal in the control of the control of the control of the control of the control of the control of the control of the control of the control of the control of the control of the control of the control of the control of the control of the control of the control of the control of the control of the control of the control of the control of the control of the control of the control of the control of the control of the control of the control of the control of the control of the control of the control of the control of the control of the control of the control of the control of the control of the control of the control of the control of the control of the control of the control of the control of the control of the control of the control of the control of the control of the control of the control of the control of the control of the control of the control of the control of the control of the control of the control of the control of the control of the control of the control of the control of the control of the control of the control of the control of the control of the control of the control of the control of the control of the control of the control of the control of the control of the control of the control of the control of the control of the control of the control of the control of the control of the control of the control of the control of the control of the control of the control of the control of the control of the control of the control of the control of the control of the control of the control of the control of the control of the control of the control of the control of the control of the control of the control of the control of the control of the control of the control of the control of the control of the control of the control of the control of the control of the control of the control of the								

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Nominal I.D. of Split Barrel Sampler	In The subsurface information shown hereon was obtained for the design and estimating purposes for our client.
Weight/type of Hammer on Drive Pipe	The state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the s
1144 4 1 1 0	The state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the s
Drop of Hammer on Drive Pipe	The state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the s
Core Size	contained in the report from which these loss were and the
	Approximate Characteristic WOH: Weight of Hammer; WOR: Weight of Rod
Soil descriptions represent a field identification	Approximate Change in Strata: Inferred Change in Strata: after D. M. Burmister unless otherwise noted

## THE CONCORD GEOPROBE LOGS

PRO	JECT NO. 7180 PROJECT Concord GEOPRO	BE NO.	OU-1B-4
LOC	ATION SEE FIGURE 1 APPROX. ELEV. 1528.52 INSPECT	ED BY	JZ/RF/CDM
WAT	ER OBSERVATION Not Encountered DATE EX	CAVATED	9/2/2008
DEPTH FT.	DESCRIPTION / SOIL CLASSIFICATION	RECOVER	Y / P.I.D.(ppm)
0	·	R = 43"	
_	025': TOPSOIL	P.I.D. = 0.0	( 0-5')
1	.25"-3.6': Red-brown medium to fine SAND, little Silt, little Gravel		٠,
	with little Cobbles		٠.
2	·		
—		}	
3	OU-1B-4A ( .9'-2.9' ) 12:24		
4			
<u> </u>			
5		. <b></b>	
		R = 44"	
6	5'-10': Same	P.I.D. = 0.0	(5'-10')
_	·		
7			
8			
_			•
9			
		ļ	
10		· <b> </b>	
		$R = 60^{n}$	
11	10'-15': Red-brown medium to fine SAND, little Silt	P.I.D. = 0.0	0(10'-14')
12			
1.5	•		٠,
13			
14—		·	

Time: 9:00 A.M - 12:30 P.M

PRO	JECT NO. 7180 PROJECT Concord	GEOPROBE NO.	OU-1B-4
LOC	ATION SEE FIGURE 1 APPROX. ELEV.	INSPECTED BY	JZ/RF/CDM
WAT	ER OBSERVATION Not Encountered	DATE EXCAVATE	9/2/2008
DEPTH FT.	DESCRIPTION / SOIL CLASSIFICATION	RECOVE	RY / P.I.D.(ppm)
14	·		
15			
16		R = 60"	
16	•	P.I.D = 0	0(15-20')
17		·	
			:
18	15-20': Same w/ Cobbles		
19			
<del></del>		•	,
20			
21		R = 60"	
21-	20'-25': Same	P.I.D. = 0	).0( 20,25' )
22	·		
			- •
23	·		
_			
24			
25			
7.	24'-27': Red-Brown medium to fine SAND, some Silt	R = 60"	
26		P.I.D. =	0.0(25-30')
27			
			ļ
28			
Time:	9:00 A.M - 12:30 P.M SES	I CONSULTING EN	IGINEERS, PC

PRO	JECT NO. 7180 PROJECT Concord GEOPI	ROBE NO.	OU-1B-4
LOC	ATION SEB FIGURE 1 APPROX. ELEV. INSPEC	TED BY	JZ/RF/CDM
WAT	ER OBSERVATION moisture at 36.4' DATE E	EXCAVATED	9/2/2008
DEPTH FT.	DESCRIPTION / SOIL CLASSIFICATION	RECOVER	RY / P.I.D.(ppm)
28		R = 58"	
	27'-30': Same	P.I.D. = 0.0	0( 27'-30' )
29			
		·	
30			
24	End of Geo-Probe at 30 Feet		
31			
32			
_			-
33			
-			
34			
35			į
36			
—			
37			
38	·		
55			
39	·		1
40			
·			
41			
_			
42			
Time:	9:00 A.M - 12:30 P.M SESI CONSU	JLTING EN	SINEERS, PC

3 of 3

PROJ	JECT NO.	7180	PROJECT	Concord	GEOPRO	BE NO.	OU-1B-5
LOCA	LOCATION SEE FIGURE 1 APPROX. ELEV. 1529.58± INSPECTED BY			JZ/RF/CDM			
WATI	ER OBSEI	RVATION	Not Encountered		DATE EXC	CAVATED	8/20/2008
DEPTH FT.		DES	CRIPTION / SOIL CL	ASSIFICATION		RECOVER	· (Υ / P.I.D.(ppm)
0	0-6":	: Asphalt		<u> </u>		R = 35"	
_	6"-1.	.5': Red-brown	medium to fine SANI	), some medium to fir	ne Gravel	P.I.D = 179	98(2")
1	1.5-2	2.9': Brown med	ium to fine SAND, lit	tle Silt, trace Gravel		P.I.D = 228	85(10")
						P.I.D = 16	40(2.75')
2	OU-	1B-5A (6"-1.5')	18:34			:	
3 <u></u> 3 <u></u> 4 <u></u>				•			
-							
4						1	
5	<b></b>						
	5-7.4	l': Same				R = 46"	
6	7.4-8	8.251: Weathered	i Sandstone w/Asphal	t	:	P.I.D = 11	00(6')
-	8.25-8.8': Red-brown medium to fine SAND, some Silt, trace Gravel			P.I.D = 80	0(6.8')		
7						P.I.D = 14	00(7.5')
						P.I.D = 50	0(8.5')
8					•		
y							
10							
10							
11			medium to fine SAND,		•		n/10 mm
' '			nedium to fine SAND.	•		P.I.D = 48	1
12 <u>—</u>			wn medium to fine SA	·	ravel	P.I.D = 20	
'	12.8	~13.25" Ked-bro	own SILT, and Siltsto	ne		P.I.D = 12	
13	077	1D ED /11 0 10	00.10.42			P.I.D = 93	· ' '
'~	UU-	1B-5B (11.8-12	18:45			P.I.D = 77	(13.1')
14							
l						I	

Time: 9:45 am - 6:45 pm

PROJ	ECT NO. 7180 PROJECT Concord GEOPRO	BE NO.	OU-1B-5
LOC	ATION SEE FIGURE 1 APPROX. ELEV. 1529.58 H INSPECT	ED BY	JZ/RF/CDM
WAT	ER OBSERVATION Not Encountered DATE EX	CAVATED	8/20/2008
DEPTH FT.	DESCRIPTION / SOIL CLASSIFICATION	RECOVER	RY / P.I.D.(ppm)
14			
¦			
15			
	End of Geo-Probe at 15 Feet		-
16	REFUSAL AT 15 FEET		
17			
18		<u> </u> 	
		Ì	
19			
20			
21	·		
· —			
22			
23			
24			
2-7			
25		] .	
26			
· .			
27			
—			
28			

Time: 9:45 am - 6:45 pm

PRO	PROJECT NO. 7180 PROJECT Concord GEO		GEOPROBE NO.	
LOC	LOCATION SEE FIGURE 1 APPROX. ELEV. 1530.35'± INSPEC		ED BY	JZ/RF/CDM
WAT	ER OBSERVATION Not Encountered	CAVATED	8/27/2008	
DEPTH FT.	DESCRIPTION / SOIL CLASSIFICATION		RECOVER	RY / P.I.D.(ppm)
0	08': Asphalt, Concrete and Debris		R = 52"	
	.8'-4.3': Red-brown SILT, some medium to fine Sand, little Gravel		P.I.D = 1:	55.0(.7')
1	·		P.I.D. = 5	50.0(1.8')
_	·		P.LD. = 80	0.0(2.81)
2	OU-1B-6A (.8-1.8') 19:40		P.I.D. = 33	0.0(3.8')
<b>—</b> '			] 	
3		-		
4	•			
4—	•			
5				
3—			~ ~ ~	
6	5'-9.6': Same		R = 55"	70(571)
,	5-9.0; Same		P.I.D = 36 $P.I.D. = 23$	· · ·
7		•	P.I.D. = 49	
			P.I.D. = 44	· ' ' )
8				10.0(5.1)
_				
9				
10			ļ	
	•		R = 53"	,
11	10'-14': Same		P.I.D = 99	0.0(10.3')
			P.I.D. = 10	00(11.7')
12			P.I.D. = 7	7.0( 12.5' )
			P.I.D. = 43	3.8( 13.9' )
13				
14		**************************************		
Time:	11:15a.m8:00p.m. SESI	CONSUL	TING EN	GINEERS, PC

Fig.

PRO	ECT NO. 7180 PROJECT Concord GEOPR	OBE NO.	OU-1B-6
LOC	ATION SEE FIGURE 1 APPROX. ELEV. 1530.35'± INSPEC	TED BY	JZ/RF/CDM
WAT	ER OBSERVATION Not Encountered DATE EX	CAVATED	8/27/2008
		1	
DEPTH FT.	DESCRIPTION / SOIL CLASSIFICATION	RECOVE	RY / P.I.D.(ppm)
14			
		R = 56"	
15	14'-17': Same	P.I.D. = 12	27.0( 14.2' )
_ <del></del>		P.I.D. = 1	,
16	16.2'-16.4': Weathered Sandstone	P.I.D. = 98	3.4(16.01)
		P.I.D. = 59	),5(16,56')
17			
40		R = 54"	
18	17'-20: Same	i	5.9(17.67')
			090.0(18.3')
19		P.I.D. = 1	
		P.I.D. = 5	2.4(19.7')
20			44.04.00.41
]	R = 58	1	44.0( 20.1' )
21	20'-23': Same		215.0( 20.7' )
		ľ	08.0(21.5').
22	·		3.1( 22.2' )
		P.I.D. = 5	6.7( 22.7' )
23			
		" P.I.D. = 1	, ,
24	23'-25': Same	į.	5.7( 23.7' )
25	P.I.D. = 41.1(26.4	)  P.I.D. = 1	45.0(24.1')
20	7 - 5	" DID - 0	2 4( 26 21)
26		P.I.D. = 2	•
	25'-28': Same		9.9(25.7')
27	·		4.7( 26.6' )
21 —			06.6(27.2')
20	·	P.I.D. = 1	.1.3('27.9')
28		_†	

Time:

11:15a.m.-8:00p.m.

PROJ	IECT NO.	7180	PROJECT	Concord	GEOPRO	OBE NO. OU-1B-6
LOCA	AŢION	SEE FIGURE	APPROX. ELEV.	1530.	35'± INSPECT	ED BY JZ/RF/CDM
WAT	ER OBSE	RVATION	Not Encountered		DATE EX	CAVATED <u>8/27/2008</u>
DEPTH FT.		DE	SCRIPTION / SOIL CL	ASSIFICATIO	N	RECOVERY / P.I.D.(ppm)
28						
		30': Same		R = 54"	P.I.D. = 54.5(31.2')	· · · · · ·
29	i		eathered Sandstone			P.I.D. = 41.9(29.1')
30	OU-	·1B-6C (28.3'-2	9.5') 20:00		P.L.D. = 10.1(32.4')	P.I.D. = 89.8(29.4')
50	Und	of Geo-Probe	ot 20 Feet			
31	Eno	of Geo-Lione	at 50 reet			
_						
32						
_						
33						
						·
34						
<del></del>	:					
35						
<u> </u>						
36						
37						
38						
JO						
39						
40						
		•				
41						
_						
42						
Time:	11:15	ia.m8:00p.m.			SESI CONSUL	TING ENGINEERS, PC

3 of 3

PRO	JECT NO. 7180 PROJECT Concord C	SEOPROBE NO.	OU-1B-7
LOC	ATION SEE FIGURE 1 APPROX. ELEV. 1530.62'± IN	SPECTED BY	JZ/RF/CDM
WAT	ER OBSERVATION Not Encountered D	ATE EXCAVATED	8/27/2008
DEPTH FT.	DESCRIPTION / SOIL CLASSIFICATION	RECOVER	RY / P.I.D.(ppm)
0	05': Asphalt, Concrete and Debris	R = 53"	
—	.5'-4.5': Red-brown medium to fine SAND, some Silt,	P.I.D = 2	3.5(1')
1—	some medium to fine Gravel	P.I.D. = 6	1.6(2')
		P.I.D. = 21	.5( 2.83' )
2	OU-1B-7A (.5-1.5') 19:00	P.I.D. = 22	.0(3.5')
	·		
3			
4—			
—			
5			
<b> </b>		R = 53"	
6—	5'-10': Red-Brown medium to fine SAND, some Silt	P.I.D'= 7.2	(6')
	some medium to fine Gravel	P.I.D. = 5.0	6(7.5')
7—	5.8'-6.2': with Sandstone	P.I.D. = 4.	7(8.51)
<b> </b> —		P.I.D. = 4.	1(9.5')
8			
	8.6'-9.2': with Sandstone		
9			
10			
<del></del>		R = 53"	
11	10'-14': Same	P.I.D = 5.5	(1 <u>1</u> 1')
		P.I.D. = 5.	6( 12' )
12		P.I.D. = 5.	6(36')
	, .	P.I.D. = 5.	3(14')
13	13.5'-14': Weathered Sandstone		
14			

Time: 1:30p.m.-7:15p.m.

PRO.	JECT NO. 7180 PROJECT Concord	GEOPROE	BE NO.	OU-1B-7
LOC	CATION SEE FIGURE 1 APPROX. ELEV. 1530.62'± INSPECT		D BY	JZ/RF/CDM
WAT	ER OBSERVATION Not Encountered	DATE EXC	AVATED	8/27/2008
DEPTH FT.	DESCRIPTION / SOIL CLASSIFICATION	1	RECOVER	Y / P.I.D.(ppm)
14				
15		F	R = 53"	
15	14'-17': Same	P	P.I.D. = 4.50	( 14.5' )
16		i	P.I.D. = 4.30	·
10		P	P.I.D. = 4.00	(15.9')
17		P	P.I.D. = 3.86	(16.6')
''				
18	17'-20: Same		<b>₹ = 58"</b>	
	17-20. Same		P.I.D. = 2.9	- II
19		1	P.I.D. = 2.70	i i
I _		1	P.I.D. = 2.50	. 11
20		1	P.I.D. = 2.20	- 11
		<u> </u>	P.I.D. = 1.80 R = 54"	19.7")
21	20'-23': Same		P.I.D. = 1.1(	20.41)
			P.I.D. = 1.10	i i
22	OU-1B-7B (23.5' - 24.5') 19:15		P.J.D. = 0.7	·
	,		P.I.D. = 0.5	·
23		1	P.I.D. = 0.3	·
	<del></del>			
24	End of Geo-Probe at 23 Feet			
	REFUSAL AT 23 FEET			
25				
26				
27				
28				
Time:	1:30p.m7:15p.m. SESI	CONSULTI	NG ENGI	NEERS, PC

PRO.	JECT NO. 7180 PROJECT Concord	GEOPROBE NO. OU-1B-8
LOC	ATION SEE FIGURE 1 APPROX. ELEV. 1530.62'±	INSPECTED BY JZ/RF/CDM
WAT	ER OBSERVATION Not Encountered	DATE EXCAVATED 8/27/2008
DEPTH FT.	DESCRIPTION / SOIL CLASSIFICATION	RECOVERY / P.I.D.(ppm)
0	05': Asphalt	R = 46"
_	.5'-5': Red-brown medium to fine SAND, little Silt,	$P.I.D = 2.8(1^{\circ})$
1	little medium to fine Gravel	P.I.D. = 0.0(1'-5')
		:
2	OU-1B-8A (.5'-1.5') 19:25	
	-	
3		
4		
5		
		R = 54"
6	5'-10': Same	P.I.D = 0.0(5'-10')
7_		
<u> </u>		
8		
9—		
10		
_		R = 53"
11	10'-14': Same	P.I.D = 0.0(10'-14')
12		
13		
-		
14		
Time:	3:15p.m7:30p.m. SESI	CONSULTING ENGINEERS, PC

PRO.	JECT NO.	7180	PROJECT	Concord	GEOPRO	BE NO.	OU-1B-8
LOC	ATION	SEE FIGURE 1	APPROX. ELEV.	1530.62	<u>'±</u> INSPECT	ED BY	JZ/RF/CDM
WAT	ER OBSE	RVATION	Not Encountered	·	DATE EX	CAVATED	8/27/2008
DEPTH FT.		DES	CRIPTION / SOIL CL	ASSIFICATION		RECOVER	RY / P.I.D.(ppm)
14							
15	14'-1	81: Red-Brown	medium to fine SANI	) little Silt		R = 53" P.I.D. = 0.0	Y 141 101 \
			m to fine Gravel	, muo om,		F.I.D 0.0	/(14-10 )
16	15.1'	'-15.8': Sandsto	ne				
17	·						
18			**				
						R = 57"	
19	18'-2	1': Same				P.I.D. = 0.0	0( 18'-21' )
20							
—							
21					·		
— 22 <u>—</u>	211.2	2.71.0				R = 58"	
	21-2	3.7': Same				P.I.D. = 0.0	0(21'-25')
23							
	24.25	5'-25.0': Red-Bi	own SILT and Siltsto	ne			
24							
25					·		
						R = 57"	
26	25'-2	8': Same				P.I.D. = 0.0	)( 25'-28' )
—   27	Weat	thered Sandstor	ne at (26.9'-27.2') and (	(27.5'-27.9')			
28					· • • • • • • • • • • • • • • • • • • •	ļ	
Time:	3:15p.r	n7:30p.m.			SESI CONSUL	TING ENG	INEERS, PC

PRO.	JECT NO. 7180 PROJECT Concord	GEOPROBE NO.	OU-1B-8
LOC	ATION SEE FIGURE 1 APPROX. ELEV. 1530.623'±	_ INSPECTED BY	JZ/RF/CDM
WAT	ER OBSERVATION Not Encountered	DATE EXCAVATED	8/27/2008
DEPTH FT.	DESCRIPTION / SOIL CLASSIFICATION	RECOVER	RY / P.I.D.(ppm)
28			
<u> </u>	28'-30': Same	R = 53"	
29	OU-1B-8B (28.0'-29.9') 19:30	P.I.D. = 0.0	)( 28'-30' )
-			·
30			
	End of Geo-Probe at 30 Feet		
31			
-			
32			
<u> </u>			
33			
34			
-			
35			
36			
-			
37			
— Ì			
38			
39			
40			
[			
41			
_			
42			
Time:	3:15p.m7:30p.m. SE	SI CONSULTING ENG	SINEERS, PC

PRO	JECT NO.	7180	PROJECT	Concord	GEOPROBE NO.		OU-1B-10
LOC	ATION	SEE FIGURE 1	APPROX. ELEV.	1531.14' <u>+</u>	INSPECT	ED BY	RF/CDM
WAT	ER OBSE	RVATION	Not Encountered		DATE EX	CAVATED	8/28/2008
DEPTH FT.		DES	CRIPTION / SOIL CLA	ASSIFICATION		RECOVER	Y / P.I.D.(ppm)
0_	0-7"	Brown coarse	to fine SAND, trace Sil	t		R = 53"	
	7"-4.	.4': Red-brown	medium to fine SAND	, little Silt, little Gravel		P.I.D = 0.0	( 0.0'-5.0' )
1	2.6-3	3.1': Sandstone					
_						·	_
2							
_	•						
3	OU-	1B-10A (8"-2')	16:03				
4							
_							
5						<b></b>	
<del></del>	5'-10	): Same				R = 58"	
6						P.I.D = 0.0	( 5.0'-10.0' )
7							
<del></del>							
8							
9							
10				=======================================			
_	10'-1	4': Same				R = 58"	
11	•					P.I.D = 0.0	( 10.0'-14.0' )
12							
13							
<u> </u>							
14			_				

Time:

7:00am - 4:25pin

PRO.	PROJECT NO. 7180		PROJECT	Concord	Concord GEOPRO		OU-1B-10
LOC	ATION	SEE FIGURE 1	APPROX. ELEV.	1531.14'+	INSPECT	ED BY	RF/CDM
WAT	WATER OBSERVATION Not Encountered DATE EXC					CAVATED	8/28/2008
DEPTH FT.	-	DES	CRIPTION / SOIL CLA	SSIFICATION		RECOVER	RY / P.I.D.(ppm)
14							
	14'-1	l7': Same				R = 54"	
15	Sand	lstone: 15.6-16.	2'			P.I.D = 0.0	( 14.0'-17.0' )
-							
16							
17_	, 						
''	179 1	20': Same		·		D - 51"	·
18		istone: 19.5-19.	6'			R = 51"	( 17.0'-20.0' )
<u> </u>	Dane	Mtone. 17.5-17.	o .			F.I.D ~ 0.0	(17.0-20.0)
19							
						1	
20						ļ	
	20'-2	24': Same				R = 57"	
21	Very	Dense: 23.5-23	3.8'			P.I.D = 0.0	( 10.0'-24.0' )
22							
23							
24							
	0.41.0	27': Same	·····			n - 66"	
25			Coarse to fine SAND,	little Silt little Green	<b>-1</b>	R = 55"	24.0'-27.0' )
	Í	-26.5': Sandstor		mae but, mue Grave	<b>~1</b>	1.1.1.0.00(	44.U-27.U <sup>-</sup> )
26							
27	<b></b>						
-	27'-3	30': Same				R = 54"	
28	OU-	1B-10B(28.9-2	9.41			P.I.D = 0.0	0( 27.0'-30.0' )
Time:	7:00a	m - 4:25pm		SE	SI CONSUL	TING ENC	INEERS, PC

PRO.	PROJECT NO. 7180 PROJECT		PROJECT	Concord	GEOPRO	BE NO.	OU-1B-10
LOC	ATION	SEE FIGURE 1	APPROX. ELEV.	1531.14'±	INSPECTED BY		RF/CDM
WAT	ER OBSE	RVATION	Not Encountered		DATE EX	CAVATED	8/28/2008
DEPTH FT.		DES	CRIPTION / SOIL CLA	SSIFICATION	,	RECOVER	Y / P.I.D.(ppm)
28	28.6	-28.8': Sandstor	ne				
29							
30						<u> </u>	
50	Pad	of Geo-Probe a	+ 20 Fact	<del></del>			
31	Bild	of Geo-Flobe a	i 30 Feet				
32							
— <del>-</del>							
33							
_							
34							
35							
_							
36							
37							
38							
_							
39			·				
40							
41							
42							
Time:	7:00an	n - 4:25pm		SES	CONSUL	ING ENG	INEERS, PC

Fig.

PRO	JECT NO.	7180	PROJECT	Concord	GEOPRO	BE NO.	OU-1B-11	
LOC	LOCATION SEE FIGURE 1 APPROX. ELEV. 1531.44'± INSPECT				INSPECT	ED BY	JZ/RF/CDM	
WAT	ER OBSER	CAVATED	8/28/2008					
DEPTH FT.		DES	CRIPTION / SOIL CL	ASSIFICATION		RECOVERY / P.I.D.(ppm)		
0	08':	FILL: Light Br	own medium to fine S	SAND, some medium t	o fine Gravel	R = 48"		
<b> </b>		little Silt				P.I.D = 0.0	( 0.0'-4.0' )	
1	.8'-4':	Red-Brown me	edium to fine SAND,	some medium to fine (	Gravel			
		little Silt with	weathered Sandstone	at 1.4'-2.0'			į	
2	OU-1	B-11A (.6'-2.9'	) 13:30				j	
— i								
3								
<u> </u>								
4							•	
						:		
5						   <b></b>		
-						R = 54"		
6—	5'-10':	: Same				P.I.D = 0.0	( 5'-10' )	
		•				ļ		
7								
8								
-								
9 <u></u>						]		
-						1		
10						<del> </del>		
	•					R = 58"		
11	10'-14	l': Same				P.I.D = 0.0	(10'-14')	
1, -								
12								
13								
14— T:		<del></del>						
Time:	9:00a.m	ı2:00p.ın.		SES	SI CONSUL	TING ENG	SINEERS, PC	

Fig.

1 of 3

PRO	JECT NO.	7180	PROJECT	Concord	GEOPROBE NO.		OU-1B-11
LOC	ATION	SEE FIGURE 1	APPROX. ELEV.	1531.44' <u>+</u>	INSPECTED BY		JZ/RF/CDM
WAT	TER OBSERVATION Not Encountered DATE EXC.		CAVATED	8/28/2008			
DEPTH FT.		DES		RECOVERY / P.I.D.(ppm)			
14							
1-						R = 56"	
15	14'-1	17': Same				P.I.D. = 0.0	0(14'-17')
16							
17	<b></b>						
_							
18	17'-2	20': Same				R = 55"	
19	:					P.I.D. = 0.	0( 17'-20' )
20							*******
21	20'-2	24': Same				R = 56"	
_						P.I.D. = 0.	0( 20'-24' )
22							
23							
24		~~~~~~~				ļ 	
	24'-2	27': Same				R = 56"	
25						P.I.D. = 0.	0( 24'-27' )
26							
40							
27	<del>-</del>				~~~~~~~	ļ	
28							
Time:	9:00a.	m2:00p.in.		SE	SI CONSUL	TING EN	SINEERS, PC

PRO	JECT NO. 7180 PROJECT Concord	GEOPROBE NO. OU-1B-11
LOC	ATION SEE FIGURE 1 APPROX. ELEV. 1531.44'±	INSPECTED BY JZ/RF/CDM
WAT	ER OBSERVATION Not Encountered	DATE EXCAVATED 8/28/2008
DEPTH FT.	DESCRIPTION / SOIL CLASSIFICATION	RECOVERY / P.I.D.(ppm)
28		
	27'-30': Same	R = 53"
29	OU-1B-11B (29.6'-30.0') 14:00	P.I.D. = 0.0( 27'-30' )
30		
31	End of Geo-Probe at 30 Feet	
32		
33		
34		
35		
_		
36		
	·	
37		
38		
30		
39		
40		
_		
41		
42		
Time:	9:00a.m2:00p.m. SE	SI CONSULTING ENGINEERS, PC

PRO	ECT NO. 7180 PROJECT Concord	GEOPRO	GEOPROBE NO. OU-1B-					
LOC	ATION SEE FIGURE 1 APPROX. ELEV. 1531,15'±	ED BY	RF/CDM					
WAT	WATER OBSERVATION Not Encountered DATE EXC							
DEPTH FT.	DESCRIPTION / SOIL CLASSIFICATION		RECOVER	tY / P.I.D.(ppm)				
0	0-5" Asphalt		R = 38"	· · · · · · · · · · · · · · · · · · ·				
	5"-3.1': Red-brown medium to fine SAND, little Silt, little Gravel,	•	P.I.D = 0.0	( 0.0'-5.0' )				
1	w/ weathered Sandstone 2.25-2.5'			·				
2	OU-1B-12A (6"-1.5") 15:32							
			<u> </u>	!				
3—								
4—								
				:				
5								
	5'-10': Same		R = 56"					
6	Sandstone 5.4-5.6', 9.3-9.4'		P.I.D = 0.0	( 5.0'-10.0' )				
7								
'								
8			ļ					
9								
~_								
10								
∥ _	10'-14': Same		R = 56"					
11	20 211 94110		ł	( 10.0'-14.0' )				
			- 0.0	( 10.0-14.0 )				
12								
_								
13								
<b>   </b>								
14								
Time:		CONCIL						

Time:

11:00 am - 3:48 pm

PRO.	JECT NO.	7180	PROJECT	Concord	GEOPRO	BE NO.	OU-1B-12
LOC	ATION	SEE FIGURE 1	APPROX. ELEV.	1531.15' <u>+</u>	INSPECTI	ED BY	RF/CDM
WAT	ER OBSE	RVATION	Not Encountered		DATE EXC	CAVATED	8/28/2008
DEPTH FT.		DES	CRIPTION / SOIL CLA	SSIFICATION		RECOVER	Y / P.I.D.(ppm)
14	•						
4.5	14'-1'	7': Same				R = 54"	
15						P.I.D = 0.0	( 14.0'-17.0' )
16							
17				~~~~~~~~~~~			
_	17'-2	0': Same				R=55"	
18						P.I.D = 0.0	( 17.0'-20.0' )
19							
20							
20	201.2	4': Same					
21		stone: 22-22.25	ı			R = 53"	(20.01.04.01)
		ture: 21-24.4'				P.I.D = 0.0	( 20.0'-24.0' )
22							
23							
24							
25	24'-2'	7': Same				R = 57"	
~~					:	P.I.D. = 0.0	0(24.0'-27.0')
26			•				
27					•••••		
_	27'-3(	)': Same				R = 44"	
28	29.9';	Very Dense				P.I.D = 0.06	( 27.0'-30.0' )

Time:

11:00 am - 3:48 pm

PROJECT NO. 7180		PROJECT	Concord	GEOPRO	BE NO.	OU-1B-12	
LOC.	ATION	SEE FIGURE 1	APPROX. ELEV.	1531.15' <u>+</u>	INSPECTE	ED BY	RF/CDM
WAT	WATER OBSERVATION Not Encountered DATE EXCAVATE				CAVATED	8/28/2008	
DEPTH FT.		DES	CRIPTION / SOIL CLA	SSIFICATION		RECOVER	RY / P.I.D.(ppm)
28							
29							
	OU-	1B-12B(29.1-3	0.0')15:48				
30						<u> </u>	<b>-</b>
	·						
31	End	of Geo-Probe a	t 30 Feet				
32							
J2						l i	
33							
34							
_							
35							
<u></u>						]	· '
36						j	
37							
<del></del>							
38							
		-					
39	The state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the state of the s						
40							
40							
41	]						
42							

Time: 11:00 am - 3:48 pm

PROJ	ECT NO. 7180 PROJECT Concord GE	OPROI	BE NO.	OU-1B-13
LOCA	ATION SEE FIGURE 1 APPROX. ELEV. 1530.98'± INSE	PECTE	D BY	JZ/RF/CDM
WAT	ER OBSERVATION Not Encountered DAT	E EXC	AVATED	8/28/2008
DEPTH FT.	DESCRIPTION / SOIL CLASSIFICATION		RECOVER	Y / P.I.D.(ppm)
0	07': Asphalt		R = 35"	
<u> </u>			P.I.D. = 0.0	( 0.0'-5,0' )
1	.7'-1.1': Red-Brown coarse to fine SAND, little Silt, trace Gravel			
2	1.1'-1.5': SANDSTONE			
3	1.5'-2.9': Red-Brown medium to fine SAND, little Silt, little Gravel			
	1.9'-2.2': with Sandstone			
4	OU-1B-13A (1.2'-2.3') 16:35			
	00-1B-13A (1.2-2.3) 10.33			
5				
			R = 58"	
6	5'-10': Same		P.I.D. = 0.0	)( 5'-10' )
	Sandstone at (6.0'-6.2') and (6.7'-7.0')			
7				
8				
_				
9				
10				
10			D — 6011	
11	10'-14': Same		R = 58" $P.I.D. = 0.0$	1/ 10'-1 <i>A</i> ' \
	Sandstone at (10.8'-10.9') and (12.8'-12.9')			/( 10 - 14 )
12		•		
13				
14				

Time:

1:00p.m.-5:02p.m.

PRO.	PROJECT NO. 7180		PROJECT	Concord	GEOPRO	BE NO.	OU-1B-13
LOC	ATION	SEE FIGURE 1	APPROX. ELEV.	1530.98' <u>+</u>	INSPECTI	ED BY	JZ/RF/CDM
WAT	ER OBSE	RVATION	Not Encountered		DATE EX	CAVATED	8/28/2008
	ſ <u></u>						
DEPTH FT.		DES	CRIPTION / SOIL CLA	ASSIFICATION		RECOVER	RY / P.I.D.(ppm)
14							
						R = 58"	
15	14'-1	7': Same				P.I.D. = 0.0	)(14'-17')
	14'-1	4.4': Light Bro	wn medium to fine mo	ttled SAND			
16							
17	·						
11-	<del>-</del>						
18	1717	:0': Same				D 56"	
	1/-2	o : Same				R = 57" P.I.D. = 0.0	)( 17I 20I )
19						F.I.D 0.0	)(17-20')
_							
20	 					 	
_							
21	20'-2	4': Same				R = 56"	
_	17'-2	11.9': Moisture	Encountered			P.I.D. = 0.0	0(21'-24')
22	Sand	lstone at (23'-23	3.3') and (23.8'-23.9')				
						<u> </u>	
23							
24							
2-7						T	
25	ר ווגר	27': Same				R = 58" P.I.D. = 0.0	0(34) 2212
	 	.7 : Same !6': Moisture Er	ncountered			E.I.D. = 0.	J( 24°-27° )
26							
27	<b></b>			***************************************		ļ	
28		<u> </u>					
Time:	l:00p.	m5:02p.m.		S	ESI CONSUL	TING ENG	SINEERS, PC

PRO.	IECT NO. 7180 PROJECT Concord	GEOPROE	BE NO. OU-1B-13
LOC	ATION SEE FIGURE 1 APPROX. ELEV. 1530.98'±	INSPECTE	D BY JZ/RF/CDM
WAT	ER OBSERVATION Not Encountered	DATE EXC	AVATED 8/28/2008
DEPTH FT.	DESCRIPTION / SOIL CLASSIFICATION		RECOVERY / P.I.D.(ppm)
28	27'-30': Same		
	27'-29.1': Moisture Encountered	[1	R = 46"
29	Sandstone at (29.1'-29.3')	[1	P.I.D. = 0.0( 27'-30' )
	OU-1B-13B (29.1'-29.4') 17:02		
30	 		
<u> </u>	End of Geo-Probe at 30 Feet		
31			
<u> </u>			
32			
33			
	•		
34		Į	
35			
_		ļ	
36			
<u> </u>			
37			
38			
39			
40			
_			
41			
42		]	

Time:

1:00p.m.-5:02p.m.

PROJ	PROJECT NO. 7180		PROJECT	Concord	GEOPRO	BE NO.	OU-1B-14		
LOCA	ATION SEE	E FIGURE 1	APPROX. ELEV.	1536.62	INSPECT	ED BY	JZ/GP		
WAT	ER OBSERVA	TION	Not Encountered		DATE EX	CAVATED	8/29/2008		
DEPTH FT.		DES	CRIPTION / SOIL CL	ASSIFICATION		RECOVE	RECOVERY / P.I.D.(ppm)		
0						R = 56"			
	0-1': Top	soil				P.I.D. = 0.	0 (0-5')		
1—	1-3.5': Ta	n/Red-bro	wn medium to fine SA	ND, little Silt, little	Gravel				
	3.5-4.6': ]	Red-brown	medium to fine SAN	D, little Silt, little Gra	avel				
2						ļ			
_	•								
3—	Sample: 0	OU-1B-14.	A ( 1-2.5')						
-									
4									
5									
	5-10': Sa	me w/ Cob	bles			R = 41"			
6—						P.I.D. = 0.	0(5-10')		
7_						1			
10									
	10-14': S	Same				  R = 47"			
11	10-17.0	,, 3, 4, 4, 4				P.I.D. = 0	.0(12.6')		
	Sample:	OU-1B-14	B (10.8-11.6') 12:50			P.I.D. = 3			
12		4 *	(==== === <del>=</del> , =====				` ′		
_									
13									
_						į			
14									
Time	11:00 A M - 1			· ·	ESI CONSIII	TING EN	GINEERS, PC		

Time: 11:00 A.M. - 12:30 P.M.

PRO	JECT NO. 7180 PROJECT Concord GEOPROBE N				BE NO.	OU-1B-14		
LOC	ATION	SEE FIGURE 1	APPROX. ELEV.	1536.62	INSPECT	ED BY	JZ/GP	
WAT	ER OBSE	RVATION	Not Encountered		DATE EX	CAVATED	8/29/2008	
DEPTH FT.		DES	SCRIPTION / SOIL CLA	SSIFICATION		RECOVER	Y / P.I.D.(ppm)	
14								
_	14-1	5.8': Same				R = 53"		
15	15.8-	-17': Gray WE		P.I.D. = 30	.4( 15.3' )			
	17-1	8.4': Red-brown	el, w/ Cobbles	P.I.D. = 51	.7(15.5')			
16	mois	t/wet from 14.5	5-15.1'			P.I.D. = 48	3(15.6')	
_	-					P.I.D. = 49	.2(15.8')	
17						P.I.D. = 0.0	0(16)	
	17-2	0': Same				P.I.D. = 0.0	)(17.3')	
18						P.I.D. = 56	(18.25')	
			R = 48"					
19						P.I.D. = 17.7(17.1')		
							.1(17.6')	
20						P.I.D. = 7.3	7(18.4')	
	20-2	4': Same w/ fre	quent Cobbles			P.I.D. = 9.7(19.6')		
21					Ĺ	P.I.D. = 3.5(19.9')		
						R = 56"		
22						P.I.D. = 12	0( 20.1' )	
-						P.I.D. = 16	7(21.5')	
23						P.I.D. = 48	.2(22.3')	
						P.I.D. = 7.4	i i	
24						P.I.D. = 30	· · · · · · · · · · · · · · · · · · ·	
			medium to fine SAND,	some Silt, trace Grave	el, moist	P.I.D. = 43	· ' '	
25	from	24-26.3'			ļ 	P.I.D. = 16	0(24.5')	
						R = 55.5"		
26						P.I.D. = 10	`	
						P.I.D. = 12	•	
27					<del></del>	P.I.D. = 84	.2(25.25')	
		0': Same				P.I.D. = 11	· · · · · · · · · · · · · · · · · · ·	
28	30-3	1.6': Red-brow	n medium to fine SANI	D, little Silt, little Grav	el w/	P.I.D. = 10	5(27¹)	

Time: 11:00 A.M. - 12:30 P.M.

PRO	OJECT NO. 7180 PROJECT Concord GEOPROE		BE NO.	OU-1B-14			
LOC	ATION	SEE FIGURE 1	APPROX. ELEV.	1536.62	INSPECTI	ED BY	JZ/GP
WAT	ER OBSE	CAVATED	8/29/2008				
DEPTH FT.		RECOVER	Y / P.I.D.(ppm)				
28	Occa	sional Cobbles		P.I.D. = 97.6(28.1')			
-						P.I.D. = 41.	5(28.5')
29	Samı	ple: OU-1B-14	C (27-28.5') 1:10			R = 55"	
_	•					P.I.D. = 227	7(27.4')
30						P.I.D. = 53.	3(28.5')
	End	of Geo-Probe a	t 30 Feet	-		P.I.D. = 153	5(29.1')
31						P.I.D. = 200	(30.25')
						P.I.D. = 242	2(31')
32						P.I.D. = 12	8(31.5')
-							
33							
-							
34							
35							
	1						
36							
_							
37							
-							
38							
-							
39							
-							
40							
41							
_							
42							

Time: 11:00 A.M. - 12:30 P.M.

SESI CONSULTING ENGINEERS, PC

Fig.

PRO	JECT NO.	7180	PROJECT	Concord	GEOPRO	DBE NO.	OU-1B-15
LOC	ATION	SEE FIGURE 1	APPROX. ELEV.	1535.16+	INSPECT	ED BY	JZ/RF/CDM
WAT	ER OBSE	RVATION	Not Encountered		DATE EX	CAVATED	9/2/2008
DEPTH FT.		DES	CRIPTION / SOIL CLA	ASSIFICATION		RECOVER	RY / P.I.D.(ppm)
0					·	R = 59"	
_	05':	: TOPSOIL				P.I.D. = 0.0	0( 0-5')
1	.5'-4.	.9': Red-brown	medium to fine SAND	, little Silt with Cobble	s		
							*.
2							
3	Sam	nle: OH-1R-15	A ( .67'-1.83' ) 10:43				
<b></b> i	Jaili	p.o. 00-1 <b>D-</b> 13/	.1(.07-1.05 ) 10.43				
4							
5	ļ					<b></b>	
-						R = 58"	
6	5'-10	)': Same				P.I.D. = 0.	0( 5'-10' )
7							
8_							
9							
10	 						
_						R = 54"	
11	10'-1	4': Same				P.I.D. = 0.	0( 10'-14' )
12							,
_						ļ	
13						1	
14						<del>\</del>	
Time:	7:4	5 - 11:25		SES	CONSUL	TING EN	SINEERS, PC

PRO	PROJECT NO. 7180		PROJECT	Concord	GEOPRO	BE NO.	OU-1B-15	
LOC	ATION :	SEE FIGURE 1	APPROX. ELEV.	1535.16 +	INSPECT	ED BY	JZ/RF/CDM	
WAT	ER OBSER	VATION	Not Encountered		DATE EX	CAVATED	9/2/2008	
DEPTH FT.		DES	CRIPTION / SOIL CLA	ASSIFICATION		RECOVERY / P.I.D.(ppm)		
14								
						R = 53"		
15	14'-17	': Same		P.I.D. = 0.0(14'-17')				
16							٠.	
17								
18	17'-20	R = 51"						
				P.I.D. = 0.0	0( 17'-20' )			
19	Sampl	le: OU-1B-15E						
20								
						R = 56"		
21	20'-24	': Red-brown i	nedium to fine SAND	, some Silt with Cobble	es	P.I.D. = 0.0(20.2')		
-						P.I.D = 7.3	(20.8)	
22						P.I.D. = 37	·	
-						P.I.D. = 0.	0( 22.2'-24.0' )	
25—	Sampl	le: OU-1B-150	C(20.0'-21.3') 11:20					
24								
_						R = 56"		
25	24'-27	': Red-Brown	medium to fine SAND	, some Silt		P.I.D. = 16	5.3( 24.8' )	
						P.I.D. = 0.	0( 26.2'-27.9' )	
26								
		, , , , , , , , , , , , , , , , , , ,						
28								
Time:	7:45.	- 11:25		SES	CONSUL	TING EN	SINEERS, PC	

Time: 7:45 - 11:25

PROJ	ECT NO. 7180 PROJECT Concord	GEOPROBE NO.	OU-1B-15		
LOCA	ATION SEE FIGURE 1 APPROX. ELEV. 1535.16 ± II	NSPECTED BY	JZ/RF/CDM		
WATI	ER OBSERVATION Not Encountered D	OATE EXCAVATED	9/2/2008		
DEPTH FT.	DESCRIPTION / SOIL CLASSIFICATION	RECOVE	RY / P.I.D.(ppm)		
28		R = 58"			
	27'-30': Same	P.I.D. = 0	.0( 27'-30' )		
29					
30					
		R = 58"	R = 58" P.I.D. = 0.0( 30'-34' )		
31	30'-34': Same with Cobbles	P.I.D. = 0	.0( 30'-34' )		
32					
JZ.—					
33					
_					
34					
		R = 58"			
35	34'-37': Same		.0( 34'-37' )		
	moisture at 36.4'-37'				
36					
	•				
37					
-		R = 58"			
38	37'-40': Same with Cobbles	P.I.D. = 0	).0( 37'-40' )		
	very moist				
39			,		
40	Sample: OU-1B-15E ( 38.3'-39.1')				
<b>70</b>	End of Geo-Probe at 40 Feet	<b></b>			
41	End of Geo-Flode at 40 Feet				
_					
42					
Time:	7:45 - 11:25 SESI C	ONSULTING EN	GINEERS, PC		

l KKO	JECT NO.	7180	PROJECT	Concord	GEOPRO	OU-1B-16	
Loc	ATION	SEE FIGURE 1	APPROX. ELEV.	1535,36 +	INSPECTI	ED BY	JZ/RF/CDM
WAT	ER OBSEI	RVATION	Not Encountered		DATE EX	CAVATED	9/2/2008
DEPTH FT.		DES	CRIPTION / SOIL CLA	ASSIFICATION		RECOVER	RY / P.I.D.(ppm)
0						R = 52"	
	08':	: Light Brown 1	medium to fine SAND	, little Silt, little Gravel		P.I.D. = 0.0	0( 0-5')
1	.8'-4.	.3': Red-brown					
<b>—</b>							
2	Samı	ple: OU-1B-16/	A ( .8'-2' ) 14:20				
3							'
4_							•
5						<b></b>	
l —						R = 59"	
6	5'-9.9	9': Red-brown:	medium to fine SAND	, little Silt, little Gravel		P.I.D. = 0.	0( 5'-10' )
∥ —							
7							
_				•			
8—							
9							
<u> </u>							
10				<b>****</b> *** *** *** *** *** *** *** *** *			
_						R = 59"	
11	10'-1	14': Same with	weathered Sandstone a	at 12.6'-12.8'			0( 10'-15' )
12							
13							
Time:		00 - 14:30		QEQ.	CONSTI	TING EN	GINEERS, PC

1 of 3

PROJ	ECT NO. 7180 PROJECT Concord	GEOPROBE NO. OU-1B-16
LOCA	ATION SEE FIGURE 1 APPROX. ELEV. 1535.36+	INSPECTED BY JZ/RF/CDM
WATI	ER OBSERVATION Not Encountered	DATE EXCAVATED 9/2/2008
DEPTH FT.	DESCRIPTION / SOIL CLASSIFICATION	RECOVERY / P.I.D.(ppm)
14		
		R = 54"
15	14'-17': Same	P.I.D. =0.0(14'-17')
16		
17		
_		R = 51"
18	17'-20': Same	P.I.D. = 0.0(17'-20')
<u> </u>		
19		·
		·
20		
21		R = 57"
2	20'-24': Same with weathered Sandstone at 23.3'-23.6'	P.I.D. = 0.0(20'-24')
22_		
23		
∥ _		
24		
-	24'-27': Same with weathered Sandstone at 26.75'-27'	R = 55"
25		P.I.D. = 0.0(24'-27')
26		
27		
	·	
28		
Time:	10:00 - 14:30 SES	SI CONSULTING ENGINEERS, PO

PRO	JECT NO.	PROJECT NO. 7180 PROJECT Concord GEOPRO			GEOPRO	BE NO.	OU-1B-16
LOC	ATION	SEE FIGURE 1	APPROX. ELEV.	1535.36 <u>+</u>	INSPECT	ED BY	JZ/RF/CDM
WAT	ER OBSE	CAVATED	9/2/2008				
DEPTH FT.		DES	CRIPTION / SOIL CLA	ASSIFICATION		RECOVER	RY / P.I.D.(ppm)
28	27'-3	R = 58"					
						P.I.D. = 0.0	0( 27'-30' )
29	Sam	ple: OU-1B-16	3 (29.3'-30') 14:30				
30		. <b></b>				<b></b>	
31	Tend	of Geo-Probe a	t 20 Foot			·	
	End	of Geo-Flone a	t 30 Peet				
32							
							1
33							
34							
						1	
35							
36—							
37		•					
31-							
38						]	
39							
_							
40							
41							
-							
42					CL CONCUL		

10:00 - 14:30

SESI CONSULTING ENGINEERS, PC

PRO	PROJECT NO. 7180 PROJECT Concord GI					EOPROBE NO. OU-1B			
LOC	ATION	SEE FIGURE 1	APPROX. ELEV.	1533.81 <u>+</u>	INSPECT	ED BY	JZ/RF/CDM		
WAT	ER OBSE	RVATION	Not Encountered		DATE EX	CAVATED	9/2/2008		
DEPTH FT.		DES	CRIPTION / SOIL CLA	ASSIFICATION		RECOVER	RY / P.I.D.(ppm)		
0						R = 56"			
	0-2.4	l': Fill: Orange-l	prown coarse to fine S.	AND, little Silt, little	Gravel	P.I.D. = 0.0( 0-5')			
1		with traces o	f Asphalt and Concrete	e					
	2.4'-4	4.67': Red-brov	vn medium to fine SAI	ND, little Silt, little G	ravel				
2									
11	Samp	ple: OU-1B <b>-</b> 17 <i>P</i>	A ( 2.67'-4.6' ) 14:45						
3									
4									
∥ — ¦							Į		
5						<del> </del> -			
<b> </b> -						R = 58"			
6	5'-10	": Red-brown r	nedium to fine SAND,	, little Silt, little Grave	el	P.I.D. = 0.0	0( 5'-10' )		
7_									
'-									
8_									
8—									
9_									
							:		
10				~	<u>.,</u>				
						R = 58"			
11	10'-1	4': Same				P.I.D. = 0.	0( 10'-15' )		
							-(,		
12									
13									
14						· <del> </del>			
Time:	11:3	0 - 14:55		SE	SI CONSUL	TING ENG	SINEERS, PC		

PRO	JECT NO.	7180	PROJECT	Concord	GEOPRO	BE NO.	OU-1B-17
LOC	ATION	SEE FIGURE 1	APPROX. ELEV.	1533.81 +	INSPECTI	ED BY	JZ/RF/CDM
WAT	ER OBSE	RVATION	Not Encountered		DATE EX	CAVATED	9/2/2008
DEPTH							
FT.			CRIPTION / SOIL CLA	ASSIFICATION		RECOVER	RY / P.I.D.(ppm)
14				·			·
15 <b>—</b>	141.1	71. C	black coarse Sandstone	1 6 6 1 1 6 9 9 1		R = 54"	7/ 14/ 17/ \
13—	14'-1 	7: Same with	black coarse Sandstone	e at 16.6'-16.83'		P.I.D. =0.0	U( 14'-17' )
16							
17							
						R = 53"	
18	17'-2	20': Same				P.I.D. = 0.	0( 17'-20' )
19							
20							
						R = 54"	
21	20'-2	24': Same				P.I.D. = 0.	0( 20'-24')
∥ —							
22							
23							
24				·			
	244	27': Same				R = 55"	
25	24-2	er. Game				P.I.D. = 0	.0( 24'-27')
						1	
26							
-							
27	ļ <b>-</b>					· <del> </del>	
Time:	11.	30 - 14:55		QE.	SLCONSUI	TING EN	GINEERS, PO

PRO	JECT NO.	7180	PROJECT	Concord	GEOPRO	DBE NO.	OU-1B-17	
LOC	ATION	SEE FIGURE 1	APPROX. ELEV.	1533.81 ÷	- INSPECT	ED BY	JZ/RF/CDM	
WAT	ER OBSEI	RVATION	Not Encountered		DATE EX	CAVATED	9/2/2008	
DEPTH FT,		DES	CRIPTION / SOIL CL	ASSIFICATION		RECOVER	RY / P.I.D.(ppm)	
28	27'-3	0': Same with v	weathered Sandstone a	at 29.3'-30'		R = 52"		
						P.I.D. = 0,0	0(27'-30')	
29	Sam	ple: OU-1B-17E	3 (28.6'-29.3') 14:55	;				
30				· <b></b>	<b></b>	<b> </b>		
31		10 P 1	30 F					
01 —	End	of Geo-Probe at	: 30 Feet					
32								
33								
34								
_								
35								
36								
37								
38								
39								
40								
<u>-</u>								
<sup>-</sup>								
42								
Time:	11:3	0 - 14:55			SESI CONSUL	TING EN	SINEERS, PC	

Fig.

PRO	JECT NO.	7180	PROJECT	Concord	GEOPROBE NO.		OU-1B-18
LOC	ATION	SEE FIGURE 1	APPROX. ELEV.	1532.36 <u>+</u>	INSPECT	ED BY	JZ/RF/CDM
WAT	ER OBSEI	RVATION	Not Encountered		DATE EX	CAVATED	9/2/2008
-					·	1	
DEPTH FT.		DES	CRIPTION / SOIL CLA	ASSIFICATION		RECOVER	Y / P.I.D.(ppm)
0						R = 49"	
	05':	: TOPSOIL				P.I.D. = 0.0	0( 0-5')
1	.5'-4.	.1': Red-brown	medium to fine SAND	, little Silt, little Gravel			
		with little C	obbles				
2							
-	Samj	ple: OU-1B-18.	A ( .5'-1.5' ) 15:55				
3							
4—							
							1
5						<b></b>	
						R = 59"	
6	5'-10	)": Same				P.I.D. = 0.0	0(5'-10')
'							
7 8							_
0—				•		ļ	·
9							
10					~ ~	<u></u>	
						R = 59"	
11	10'-1	.4': Same				P.I.D. = 0.0	0( 10'-15' )
							,
12							
13			_				•
_			•				
14						<b>}</b>	
Time:	L:3	30 - 4:00		SESI	CONSUL	TING ENG	INEERS, PC

1 of 3

		Not Encountered  PESCRIPTION / SOIL CLA  with medium to fine SANI	ASSIFICATION		CAVATED	JZ/RF/CDM 9/2/2008 Y / P.I.D.(ppm)			
DEPTH FT. 14 15	E	ESCRIPTION / SOIL CL			RECOVER				
14 14 15						Y / P.I.D.(ppm)			
— 15 <u>—</u> —	14'-17': Red-bro	wn medium to fine SANI	D, little Silt, little Gravel		R = 59"				
	14'-17': Red-bro	wn medium to fine SANI	), little Silt, little Gravel		R = 59"				
	14'-17': Red-bro	wn medium to fine SANI	), little Silt, little Gravel		P.I.D. =0.0				
16									
il I									
17									
-					R = 48"				
18	17'-20': Same				P.I.D. = 0.0	)( 17'-20' )			
10									
19									
20									
					R = 54"				
21	20'-24': Same				P.I.D. = 0.0	)( 20'-24')			
-					-				
22									
23									
24									
	24'-27': Same			· <b></b>	R = 39"				
25					P.I.D. = 0.0	)( 24'-27')			
-						-			
26									
27						· <b></b>			
28			•						
Time:	1:30 - 4:00		SESI	CONSULT	TING FNO	INEERS, PC			

PRO	JECT NO.	7180	PROJECT	Concord	GEOPRO	BE NO.	OU-1B-18	
LOC	ATION	SEE FIGURE 1	APPROX. ELEV.	1532.36 +	INSPECT	ED BY	JZ/RF/CDM	
WAT	ER OBSE	RVATION	Not Encountered		DATE EX	CAVATED	9/2/2008	
DEPTH FT.		DES	CRIPTION / SOIL CLA	ASSIFICATION		RECOVERY / P.I.D.(ppm)		
28	27'-3	0': Same with	weathered Sandstone a	t 29.3'-30'	,	R = 55"		
				P.I.D. = 0.0( 27'-30')				
29	OÙ-	1B-18B ( 29.2'-						
30	- <b></b>		·			ļ +		
31	End	of Geo-Probe a	t 30 Feet					
32								
_								
33								
34								
35								
36								
37								
-								
38								
39								
40								
41								
42							į	

1:30 - 4:00

PRO	JECT NO. 7180 PROJECT Concord	GEOPRO	BE NO.	OU-1B-19	
LOC	ATION SEE FIGURE 1 APPROX. ELEV. 1531.03	INSPECTE	ED BY	JZ/GP	
WAT	ER OBSERVATION Not Encountered	DATE EXC	CAVATED	8/29/2008	
DEPTH FT.	DESCRIPTION / SOIL CLASSIFICATION		RECOVERY / P.I.D.(ppm)		
0	0-4": Topsoil		R = 50"		
	4"-1.75': Orange-brown Silt, medium to fine Sand, little Gravel		P.I.D. = 0.0	(0-5')	
1	1.75-4.8': Red-brown medium to fine SAND, little Gravel, little Sile	, w/ cobbles			
_					
2	Sample: OU-1B-19A (6"-2") 11:45				
3					
		:			
7		1			
5					
	5-10': Same.		R = 57.5"		
6	5 To Family.		P.I.D. = 0.0	)( 5-10')	
			11.20		
7					
_					
8					
_					
9—					
10			<b> </b>		
	10-14': Same		R = 58"		
11		!	P.I.D. = 0.0	)(10-14')	
12					
12-					
13					
14					
			L		

10:30 A.M. - 12:00 A.M.

PRO	JECT NO. 718	PROJECT	Concord	GEOPRO	BE NO.	OU-1B-20
LOC	ATION SEE FIG	SURE 1 APPROX. ELE	<b>15.9.80</b>	INSPECT	ED BY	JZ/GP
WAT	ER OBSERVATIO	Not Encountered		DATE EX	CAVATED	8/29/2008
DEPTH FT.		DESCRIPTION / SOIL C	LASSIFICATION		RECOVER	RY / P.I.D.(ppm)
0_	0-4": Topsoil				R = 58"	···
	4"-4.8': Red-l	orown medium to fine SAN	D, little Silt, little Gravel		P.I.D. = 0.0	) (0-5')
1						
	Sample: OU-	1B-20A (6"-2") 10:00				
2						
-						
3						
<b>I</b> — I						
4						
ll						
5	<del></del>				ļ 	
<u> </u>	5-10': Same v	v/ more frequent cobbles fr	om 8.25-9.8'		R = 58"	
6					P.I.D. = 0.6	0( 5-10')
6— 7— 8—						
8—						,
-						
9						
		و بين هي بين بين بين بين بين بين بين بين بين بي				
11	10-15': Same	w/ some Silt			R = 56"	0/10 145
' '		rn aan //a /a == 10 - 1			P.I.D. = 0.	U(10-15')
12	Sample: OU-	1B-20B (12-13.5') 10:05				
'	•					
13						
14						
Time	7-45 A M = 10-00 A			CONSUL	TING ENG	NIEEBO BO

7:45 A.M. - 10:00 A.M.

PRO	JECT NO.	7180	PROJECT	Concord	GEOPROBE NO.		OU-1B-20
LOC	ATION	SEE FIGURE 1	APPROX. ELEV.	1539.8	INSPECTE	ED BY	JZ/GP
WAT	ER OBSE	RVATION	Not Encountered		DATE EXC	CAVATED	8/29/2008
DEPTH FT.		DES	CRIPTION / SOIL CLA	ASSIFICATION		RECOVER	Y / P.I.D.(ppm)
14		•	<u>-</u>				
— 15——		·					
	15-1	9.5': Same		<b></b>		R = 55"	
16		<b>7.0 ( D</b>				P.I.D. = 0.0	)(15-19.5')
	Sam	ple: OU-1B-20	C (18-19.5') 10:30				
17							
18							
_							
19							
_				~~~~~~			
20		-20.3': Same				R = 10"	
21			t 20 Feet 3 Inches			P.I.D. = 0.0	0( 19.5-20.3' )
	REF	USAL ON RU	CK AT 20 FEET 3 INC	CHES			
22							
_							
23							
	ļ						
24							
25	[ ]						
26							
27							
20	<u> </u>						

Time: 7:45 A.M. - 10:00 A.M.

PRO	JECT NO.	7180	PROJECT	Concord	GEOPRO	BE NO.	OU-1B-21	
LOC	ATION	SEE FIGURE 1	APPROX. ELEV.	1535.39	INSPECTI	ED BY	JZ/GP	
WAT	ER OBSE	RVATION	Not Encountered		DATE EXC	CAVATED	8/29/2008	
DEPTH FT.		DES	GCRIPTION / SOIL CLA	ASSIFICATION		RECOVERY / P.I.D.(ppm)		
0	0-3"	: Topsoil				R = 58"		
_	3"-4	.8': Red-brown	medium to fine SAND	, little Gravel, little	Silt, w/ cobbles	P.I.D. = 0.0 (0-5')		
1						ļ		
	Sam	ple: OU-1B-21	A (6"-2') 10:45					
2								
3_								
4								
5						 		
<b> </b>	5-10	)': Same w/ som	e Silt			R = 57"		
6						P.I.D. = 0.	0( 5-10')	
	Sam	ple: OU-1B-21	B (8-9.5') 10:50					
7						ļ		
<b> </b>								
8								
9—								
10	· ·							
	10-1				·	R = 55"		
11	10-1	Duillo				$\begin{vmatrix} R - 33 \\ P.I.D. = 0. \end{vmatrix}$	0(10-14')	
<b> </b>							- \ //	
12								
13								
14								

10:15 A.M. - 10:50 A.M.

PRO	JECT NO.	7180	PROJECT	Concord	<u> </u>	GEOPRO	BE NO.	OU-1B-21	
LOC	ATION	SEE FIGURE 1	APPROX. ELEV	1535	5.39	INSPECT	ED BY	JZ/GP	
WAT	ER OBSER	RVATION	Not Encountered			DATE EX	CAVATED	8/29/2008	
DEPTH FT.		DES	CRIPTION / SOIL CI	-ASSIFICATIO	ON		RECOVERY / P.I.D.(ppm)		
14									
15	14-17 	": Same w/ mo	re frequent Cobbles				R = 58" P.I.D. = 0.0(14-17')		
_							F.I.D. – 0.	)(14-17)	
16								-	
_									
17			~ 1						
18	17-20	o': Same w/ les	s Cobbles				R = 51" P.I.D. = 0.	n(17-20')	
							T.I.D 0,	0(17-20)	
19									
20									
21	20-24	l': Same / wet f	from 20.4-20.6'				R = 55" P.I.D. = 0.	0(20.241)	
	Samp	le: OU-1B-21	C (20-21.3') 11:15				F.1.D 0.	0(20-24)	
22	1							:	
<del></del>								•	
23	<u> </u>								
24									
	24-27	': Same					R = 57"		
25	2.2,						P.I.D. = 0.	0( 24-27')	
_									
26									
27	Ĭ.						1		
	27-30	)': Same w/ W	EATHERED Rock at	: 29-30.4			R = 41"	<del></del>	
28							1	.0( 27-30' )	

10:15 A.M. - 10:50 A.M.

PROJ	ECT NO. 7180 PROJECT Concord	GEOPRO	BE NO.	OU-1B-21
LOCA	ATION SEE FIGURE 1 APPROX. ELEV. 1535.39	INSPECTE	D BY	JZ/GP
WAT	ER OBSERVATION Not Encountered	DATE EXC	CAVATED	8/29/2008
DEPTH FT.	DESCRIPTION / SOIL CLASSIFICATION		RECOVER	Y / P.I.D.(ppm)
28				
29				
30				<del></del>
24	End of Geo-Probe at 30 Feet			
31	REFUSAL ON ROCK AT 30 FEET			
32				
33				
34				
			1	
35				
				-
36				
_				
37				
38				
			:	
39				
40				
38 — 39 — 40 — 41 — 41 —				
41				
42				
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10:15 A.M. - 10:50 A.M.

LOCATION   SEE FIGURE 1   APPROX. ELEV.   1529.28   INSPECTED BY   JZ/GP	PRO	ECT NO. 7180 PROJ	GEOPROBE NO.	OU-1B-22	
DEPTH   DESCRIPTION / SOIL CLASSIFICATION   RECOVERY / P.I.D. (ppm)	LOC	ATION SEE FIGURE 1 APPR	INSPECTED BY	JZ/GP	
FT. DESCRIPTION SOIL CLASSIFICATION RECOVERY 7.P.I.B.(ppm)	WAT	ER OBSERVATION Not Enc	ountered	DATE EXCAVATED	8/29/2008
		DESCRIPTIO	N / SOIL CLASSIFICATION	RECOVE	RY / P.I.D.(ppm)
1— Sample: OU-1B-2A (6"-1.5") 9:05 2— 3— 4— 4— 4— 5— 5-10": Same w/Cobbles R = 58" P.I.D. = 0.0( 5-10")  Sample: OU-1B-2B (6-7.5") 9:15 7— 8— 9— 10— 10-14": Same w/ more frequent Grave/Cobbles R = 48" P.I.D. = 0.0(10-14")  12— 10-14": Same w/ more frequent Grave/Cobbles R = 48" P.I.D. = 0.0(10-14")	0	0-6": Topsoil		R = 58"	
2		6"-4.8': Red-brown medium t	to fine Sand, little Silt, little Gravel	P.I.D. = 0	.0 (0-5')
2	1				
3 — 4 — 4 — 5 — 5-10': Same w/Cobbles R = 58" P.I.D. = 0.0(5-10')  Sample: OU-1B-2B (6-7.5') 9:15  7 — 8 — 9 — — 10 — 10-14':Same w/ more frequent Grave/Cobbles R = 48" P.I.D. = 0.0(10-14')  11 — 12 — — 12 — — 12 — — 12 — — 12 — — 12 — — 12 — — 12 — — 12 — — 12 — — 12 — — 12 — — 12 — — 14 — — 15 — 15 — — 15 — 15 — — 15 — — 15 — 15 — — 15 — 1	_	Sample: OU-1B-2A (6"-1.5")	9:05		•
	2				
- 5-10': Same w/Cobbles  6 - P.I.D. = 0.0( 5-10')  Sample: OU-1B-2B (6-7.5') 9:15  7 - 8 - 9 - 10 - 10-14': Same w/ more frequent Grave/Cobbles  11 - 12 - 12 - 12 - 12 - 12 - 12 - 12	3—				
- 5-10': Same w/Cobbles  6 - P.I.D. = 0.0( 5-10')  Sample: OU-1B-2B (6-7.5') 9:15  7 - 8 - 9 - 10 - 10-14': Same w/ more frequent Grave/Cobbles  11 - 12 - 12 - 12 - 12 - 12 - 12 - 12	_				
- 5-10': Same w/Cobbles  6 - P.I.D. = 0.0( 5-10')  Sample: OU-1B-2B (6-7.5') 9:15  7 - 8 - 9 - 10 - 10-14': Same w/ more frequent Grave/Cobbles  11 - 12 - 12 - 12 - 12 - 12 - 12 - 12	4—	•			
- 5-10': Same w/Cobbles  6 - P.I.D. = 0.0( 5-10')  Sample: OU-1B-2B (6-7.5') 9:15  7 - 8 - 9 - 10 - 10-14': Same w/ more frequent Grave/Cobbles  11 - 12 - 12 - 12 - 12 - 12 - 12 - 12					:
6— Sample: OU-1B-2B (6-7.5') 9:15  7— 8— 9— 10— 10-14':Same w/ more frequent Grave/Cobbles R = 48" P.I.D. = 0.0(10-14')	5—				·i
Sample: OU-1B-2B (6-7.5') 9:15  7— 8— 9— 10— 10— 10-14':Same w/ more frequent Grave/Cobbles  R = 48" P.I.D. = 0.0(10-14')		5-10': Same w/Cobbles	•	Į.	
7— 8— 9— 10— 10— 11— 11— 12— 12— 12— 13— 15— 16— 17— 18— 10— 10— 10— 10— 10— 10— 10— 10— 10— 10	0	0 1 0T (D 0D (C 0 0D)		P.I.D. = 0	0.0( 5-10')
8— 9— 10— 10— 10-14':Same w/ more frequent Grave/Cobbles  11— 12— 12— 15— 16— 17— 18— 10— 10— 10— 10— 10— 10— 10— 10— 10— 10	7	Sample: OU-1B-2B (6-7.5') 9	9:15		
9	, —				
9	8				
— 10-14':Same w/ more frequent Grave/Cobbles  R = 48"  P.I.D. = 0.0(10-14')  12		-	·	: #	
— 10-14':Same w/ more frequent Grave/Cobbles  R = 48"  P.I.D. = 0.0(10-14')  12	9				
— 10-14':Same w/ more frequent Grave/Cobbles  R = 48"  P.I.D. = 0.0(10-14')  12	_				
11  12 	10		·		
11  12 		10-14':Same w/ more frequer	nt Grave/Cobbles	$R = 48^{\circ}$	
12 -	11		-		).0(10-14')
					` - ' /
13 14	12				
13	_				
	13				
14					
	14				

Time: 7:45 A.M. - 9:25 A.M.

SESI CONSULTING ENGINEERS, PC

PRO.	ECT NO. 7180 PROJECT Concord G	EOPROBE NO.	OU-1B-22
LOC	LOCATION SEE FIGURE 1 APPROX. ELEV. 1529.28 INSPEC		JZ/GP
WAT	ER OBSERVATION Not Encountered DA	ATE EXCAVATED	8/29/2008
DEPTH FT.	DESCRIPTION / SOIL CLASSIFICATION	RECOVER	RY / P.I.D.(ppm)
14			
_	14-17': Same	R = 58"	
15	•	P.I.D. = 0.	0( 14-17' )
_			
16			
	·		-
17			
40	17-20': Same w/ less Cobbles	R = 48"	
18		P.I.D. = 0.	0(17-20')
19			
19—			
20		ļ	
	20-24': Same w/some Silt	R = 59"	
21	20-24. Same wisome out	P.I.D. = 0.	0(30 341)
		1.1.1.	0(20.24)
22			
_		İ	
23			
_			
24			
	24-27': Same	R = 57"	
25		P.I.D. = 0.	0( 24-27')
<b> </b> -			
26			
<b>—</b>			
27			
_	27-30': Same	R = 54"	
28		P.I.D. = 0	
Time:	7:45 A.M 9:25 A.M. SESI CO	ONSULTING EN	GINEERS, PC

PRO	ECT NO. 7180 PROJECT Concord	GEOPROBE	NO. OU-1B-22
LOC	ATION SEE FIGURE 1 APPROX. ELEV. 1529.28	INSPECTED I	BY JZ/GP
WAT	ER OBSERVATION Not Encountered	DATE EXCAV	ATED 8/29/2008
DEPTH FT.	DESCRIPTION / SOIL CLASSIFICATION	RE	COVERY / P.I.D.(ppm)
28	Sample: OU-1B-2C (27.5-29') 9:25		
29		į	
30_			٠.
30—	T 1 00 P 1 420 F 4		
31	End of Geo-Probe at 30 Feet		
_			
32			
_			
33			
_			
34			
35			
36			
37			
38			
_			
39			
_			
40			
<b> </b>			
41			·
42			<u> </u>

7:45 A.M. - 9:25 A.M.

PRO	ECT NO. 7180 PROJECT Concord	SEOPROBE NO.	OU-1B-23		
LOC	ATION SEE FIGURE 1 APPROX. ELEV. 1530.16 IN	ISPECTED BY	JZ/RF/CDM		
WAT	WATER OBSERVATION Not Encountered DATE EXCA				
DEPTH FT.	DESCRIPTION / SOIL CLASSIFICATION	RECOVE	RY / P.I.D.(ppm)		
0		R = 12"			
<b> </b> -	0-5': Light Brown medium to fine SAND, little Silt, little Gravel				
1—					
2					
3					
_					
4			İ		
-					
5_					
		R = 43"	4 57/5 211 3		
6	5-10': Gray medium to fine GRAVEL w/ red-brown medium to fine S	and $P.I.D. = 2$ P.I.D. = 3	. ,		
7		P.I.D. = 1			
		P.I.D. = 7			
8			• /		
9					
10—			·		
		R = 49"	0.0 ( 50		
11	10-15': Red-brown medium to fine SAND, some Silt	P.I.D. = 3	02(.5') 006(11.3')		
12		P.I.D. = 1 $P.I.D. = 3$	•		
		P.I.D. = 2	, ,		
13			94(13.75')		
			· ,		
14	End of Geo-Probe at 14 Feet				
Time:	SESI CO	ONSULTING EN	GINEERS, PC		

PRO	PROJECT NO. 7180 PROJECT Concord GEOPROB			
LOC	LOCATION SEE FIGURE 1 APPROX. ELEV. 1530.16 INSPECTED			
WAT	ER OBSERVATION Not Encountered DATE E	XCAVATED	9/3/2008	
DEPTH FT.	DESCRIPTION / SOIL CLASSIFICATION	RECOVER	tY / P.I.D.(ppm)	
0		R = 22"		
	0-10": Brown fine SAND, some Silt, little Gravel	P.I.D. = 0.0	) (0-5')	
1	10"-1.8': Brown coarse to fine Sand, little Gravel, little Silt			
2	Sample: OU-1B-23A(10-1.8') 11:15			
3		:		
4				
5				
_	5-5.4': Dark gray coarse to fine SAND, some Silt, little Gravel	R = 9"		
6	5.4-6.1': Red-brown clayey SILT, trace Gravel, trace Sand	P.I.D. = 0.6	0(5-5.4')	
		P.I.D. = 2.3	5(5.4-6.1')	
7				
8				
_				
9				
10				
	10-10.8': Red-brown medium to fine Gravel, some coarse to fine Sand, little S	lt R = 14"		
11	10.8-11.1'- Red-brown clayey SILT, trace Sand, trace Gravel	P.I.D. = 38	3.4(10.6')	
		P.I.D. = 23	.5(11')	
12				
-				
13				
-				
14				

Time: 11:15 A.M. - 12:15 P.M.

PRO	JECT NO.	7180	PROJECT	Concord	GEOPRO	OBE NO.	OU-1B-23A
LOC	CATION SEE FIGURE 1 APPROX. ELEV. 1530.16 INSPECT		ED BY	JZ/GP			
∥ <sub>WAT</sub>	ER OBSE	RVATION	Not Encountered		DATE EX	CAVATED	9/3/2008
						T	
DEPTH FT.		DES	CRIPTION / SOIL CLA	SSIFICATION		RECOVER	RY / P.I.D.(ppm)
14							7.
_							
15							
	15-1	5.75': Gray med	lium to fine Gravel, so	me coarse to fine Sand	l, little Silt	R = 60"	
16	15.7:	5-20': Red-brov	vn clayey SILT, trace S	and, trace Gravel		P.I.D. = 8.9	9( 15.4' )
						P.I.D. = 15	.9(16')
17	Samj	ple: OU-1B-231	B (16.5-17.5') 12:05			P.I.D. = 19	1.1(16.5')
-						P.I.D. = 47	•
18						P.I.D. = 20	•
						P.I.D. = 17	• •
19						P.I.D. = 14	•
						P.I.D. = 11	
20						P.I.D. = 10	).0(20¹) 
	21.6-	-22.6': Same				R = 32"	
21						P.I.D. = 3.	•
						P.I.D. = 5.	-
22						P.I.D. = 5.	
						P.I.D. = 4.	•
23						P.I.D. = 4.	4( 22.5' )
24							
25							
25		- 0. 6		~	<b></b>		
26	25-2	7.8': Same				R = 34"	1/25 25!>
20	_	OII 1D 62:	O (OG OG OI)			P.I.D. = 7.	•
27	Sam	ple: OU-1B-230	∪ (∠1 <b>-</b> ∠1.8°)			P.I.D. = 7.	. ,
						P.I.D. = 6.	
28						P.I.D. = 10	
				656	EL CONSU	P.I.D. = 5.	
Time:	11:15 A.N	л 12:15 Р.М.		2E3	い いついりひし	. HING ENG	SINEERS, PC

PRO	JECT NO. 7180 PROJECT Concord	GEOPRO	BE NO.	OU-1B-23A
LOC	ATION SEE FIGURE 1 APPROX. ELEV. 1530.16	D BY	JZ/GP	
WAT	ER OBSERVATION Not Encountered	DATE EXC	AVATED	9/3/2008
DEPTH FT.	DESCRIPTION / SOIL CLASSIFICATION		RECOVER	Y / P.I.D.(ppm)
28			P.I.D. = 10.	5( 27.5' )
			P.I.D. = 6.0	( 27.6' )
29				
_				
30				
	End of Geo-Probe at 30 Feet	ļ		
31	REFUSAL ON ROCK AT 30 FEET			
_				
32				
33				
_				
34				
35				
_				
36				
37				
38				
_				
39				•
_				
40				
41				
42				

11:15 A.M. - 12:15 P.M.

PRO	ECT NO. 7180 PROJECT Concord	GEOPROBE NO.	OU-1C-1		
LOC	LOCATION SEE FIGURE 1 APPROX. ELEV. 1484.37'± INSPECT		RF/CDM		
WAT	WATER OBSERVATION Not Encountered DATE EXCA				
DEPTH FT.	DESCRIPTION / SOIL CLASSIFICATION	RECOVER	Y / P.I.D.(ppm)		
0—	0-4": Asphalt	R = 48"			
_	4"-4': Red-brown SILT, little Gravel, trace Sand	P.I.D = 0.0	( 0'-5' )		
1					
2					
2	Sample: OU-1C-1A (4"-2') 11:30		:		
_					
3					
4					
5			·		
	5-8.9': Same	R = 47"			
6		P.I.D = 0.0	(5'-10')		
7_	•				
'					
88					
_					
9					
	•				
10	***************************************				
	10-11.2': Same	R = 45"			
11	11.2-11.5': Red-brown fine SAND, little Gravel, trace Silt	P.I.D = 0.0	( 10'-15' )		
			-		
12					
	•				
13					
<u> </u>					
14					

11:00 am - 3:48 pm

PROJECT NO. 7180		PROJECT	Concord	_ GEOPROBE NO.		OU-1C-1	
LOC	ATION	SEE FIGURE 1	APPROX. ELEV.	1484.37' <u>+</u>	INSPECT	ED BY	RF/CDM
WAT	ER OBSEI	RVATION	Not Encountered		DATE EX	CAVATED	9/4/2008
DEPTH FT.		DES	CRIPTION / SOIL CLA	ASSIFICATION		RECOVER	Y / P.I.D.(ppm)
14						<del></del>	
_							
15						 	·
_	15-20	)': Same				R = 60"	
16						P.I.D = 0.0	( 15'-20' )
—							
17							
18							
-							
19							
_							
20						<b></b>	
	20'-2	5': Same				R = 56"	
21						P.I.D = 0.00	( 20'-25' )
						:	
22							
23							
24							
25			·				
25							
26	25.0-2	28.4': Red-brov	n fine SILT, trace Gra	ivel, trace Sand		R= 53"	
۷						P.I.D = 0.00	( 25'-30' )
27_							
21 —							
28							
Time:	11:00 an	1 – 3:48 pm		SES	SI CONSULT	ING ENG	INEERS, PC

PRO	JECT NO.	7180	PROJECT	Concord	GEOPRO	GEOPROBE NO.	
LOC	ATION	SEE FIGURE 1	APPROX. ELEV.	1484.58' <u>+</u>	INSPECT	ED BY	RF/CDM
WAT	ER OBSE	RVATION	Not Encountered	·	DATE EX	CAVATED	9/4/2008
DEPTH FT.		DES	CRIPTION / SOIL CLA	ASSIFICATION		RECOVER	Y / P.I.D.(ppm)
28							
29	28.4	-29.4': Weather	ed Shale				
	Samj	ple: OU-1C-1B(	(27.5-28.3')11:45				
30	- <b></b>			·		- <b>-</b>	
31	End	of Geo-Probe at	30 Feet		•		
32							
<u> </u>					i		
33							
34							
35							
_							
36		•			•		
37						i	
38—							
39		·					
40							
-							
41							
42							
Time:	11:00 ar	n - 3:48 pm		SE	SI CONSULT	ING ENG	INEERS, PC

Fig.

PROJECT NO. 7180		PROJECT	Concord	GEOPRO	BE NO.	OU-1C-5	
LOC	ATION	SEE FIGURE 1	APPROX. ELEV.	1484.58' <u>+</u>	INSPECT	ED BY	RF/CDM
WATER OBSERVATION Not Encountered DATE EXCAVATED		CAVATED	9/4/2008				
DEPTH FT.		DES	CRIPTION / SOIL CLA	SSIFICATION		RECOVER	Y / P.I.D.(ppm)
0		-				R = 33"	
	0-2.7	'5': Red-brown S	SILT, little fine Sand, l	ittle Gravel		P.I.D = 0.00	(0'-5')
1							·
2_							
2	Samp	ole: OU-1C-5A	(1.6-2.75') 10:15				
—							
34							
4							
5		· -					
	5-7.9	': Same				R = 35"	
6— 7— 						P.I.D = 0.00	[5'-10')
7_							
							٠
8							
9							
_							
10						·	
_	10-12	2.5': Same				R = 60"	
11						P.I.D = 0.0(	10'-12.5')
							ĺ
12							
13	12.5-1	15': Red-brown	fine SAND, little Silt,	little Gravel		P.I.D. = 0.0	(12.5-15')
14	<del>-</del>					·	
Time:	11:00 an	1-3:48 pm		SE	SI CONSULT	ING ENG	NEERS, PC

Fig.

Time:

PRO	JECT NO.	7180	PROJECT	Concord	GEOPRO	DBE NO.	OU-1C-5
LOC	ATION	ED BY	RF/CDM				
WAT	ER OBSE	RVATION	Not Encountered		DATE EX	CAVATED	9/4/2008
DEPTH FT.		DES	CRIPTION / SOIL CLA	SSIFICATION		RECOVER	Y / P.I.D.(ppm)
14							
4.5							
15							
16	15-1	625" Red-brow	n SILT, little fine Sand	i little Consul		R = 45"	
_[	15-1	0.25 . ICCC-010W.	i bili, mae ine ban	1, IIIIe Gravel		P.1.D. = 0.0	( 15'-16.75' )
17	16.2:	5-18.75': Red-br	own fine SAND, little	Silt, little Gravel with	Boulder	P.J.D. = 0.0	(16.25-18.75')
							,
18							
19							
20							
_						R = 50"	
21	20'-2	1.2': Red-brown	SILT, little fine Sand,	little Gravel		P.I.D. = 0.0	( 20'-21.2' )
22				·			
	21.2-	24.2': Red-brow	n fine SAND, some Si	lt, little Gravel		P.I.D. = 0.0	(21.2-24.2')
23							
_							
24							
25							
26	35.07	06 7h D - 4 '	- E - G1375	to that in a		R= 52"	
	25.0-2	.o./: Ked-browi	ı fine SAND, some Si	It, little Gravel		P.I.D. = 0.00	( 25'-26.7' )
27	26.7-2	28.3': Red-brown	ı fine SILT, little fine	Sand, little Gravel		P.I.D. = 0.0	(26.7-28.21)
_			, <del></del>	,		z .z.D, — 0,0	(20.1-20.3)
28							İ
Time:	11:00 an	1-3:48 pm		SESI	CONSULT	ING ENGI	NEERS, PC

PRO.	JECT NO.	7180	PROJECT	Concord	GEOPRO	DBE NO.	OU-1C-5
LOC	ATION	SEE FIGURE 1	APPROX. ELEV.	1484.58' <u>+</u>	INSPECT	ED BY	RF/CDM
WAT	ER OBSE	RVATION	Not Encountered		DATE EX	CAVATED	9/4/2008
DEPTH FT,		DES	CRIPTION / SOIL CLA	SSIFICATION		RECOVER	RY / P.I.D.(ppm)
28					······································		
29							
30	Sam	ple: OU-1C-5B(	(27.7-28.5')10:30				
30	<del></del>			<del>-</del>	<del>-</del>	<del> </del>	
31	End e	of Geo-Probe at	: 30 Feet				110
32							
33							
33—							
34							
35							
_							
36							
37							
_							
38		•					
_							
39							
40							
41							
42		<del> </del>		· <u> </u>			
Time:	11:00:11	n - 3:48 pm		SESI	CONSULT	ING ENG	INEERS, PC

PRO.	JECT NO.	7180	PROJECT	Concord	GEOPRO	BE NO.	OU-1C-9
LOC	ATION S	EE FIGURE 1	APPROX. ELEV.	1490.03' <u>+</u>	INSPECT	ED BY	RF/CDM
WAT	ER OBSERV	/ATION	Not Encountered		DATE EX	CAVATED	9/4/2008
DEPTH FT.		DES	CRIPTION / SOIL CLA	SSIFICATION		RECOVER	Y / P.I.D.(ppm)
0	0-6": T	opsoil				R = 50"	<del></del>
_	4"-4.2':	Red-brown r	n-f SAND, little Silt, l	little Gravel		P.I.D = 0.06	( 0'-5' )
1							
2	Sample	:: OU-1C-1A	(6"-2.1') 17:00				
3							
4							
5			********				
	5-9.8': I	Red-brown c-	f SAND, little Silt, lit	tle Gravel with small (	Cobbles	R = 58"	
6						P.I.D = 0.00	(5'-10')
7							
,							
8							
9							
10							
	10-14':	Same				R = 59"	
11						P.I.D = 0.00	(10'-14')
12							
<u> </u>							
13							
14							

Time:

PRO.	JECT NO.	7180	PROJECT	Concord	GEOPRO	BE NO.	OU-1C-9
LOC	ATION	SEE FIGURE 1	APPROX. ELEV.	1490.03' <u>+</u>	INSPECT	ED BY	RF/CDM
WAT	ER OBSER	RVATION	Not Encountered		DATE EX	CAVATED	9/4/2008
DEPTH FT.		DES	SCRIPTION / SOIL CLA	ASSIFICATION		RECOVER	Y / P.I.D.(ppm)
14							
<u> </u>	14-19	: Same w/ Co	bbles			R= 53"	ĺ
15						P.I.D = 0.00	(14'-19')
_							2
16							
]							
17							
<u> </u>							
18							
-							
19							
	19-23	': Red-brown r	n-f SAND, little Silt, l	ittle Gravel with Cobl	oles	R = 59"	
20						P.I.D = 0.00	[ 19-23' )
21—							
22							
23							
_	23-24.	.1': Same				R= 44"	
24						P.I.D = 0.00	[23-26')
	24.1-2	6': Red-brown	m-f SAND, some Sil	t, little Gravel with Co	obbles		
25							
26							
27	26-30'	: Same w/ Col	bbles			R=50"	
27						P.I.D = 0.0(	26-30')
<u></u>							
28		<del></del>					
Time:	<del></del> .		······································	SES	I CONSULT	ING ENG	NEERS, PC

LOCATION   SEE FIGURE 1   APPROX. ELEV.   1490.03±   INSPECTED BY   RF/CDM   WATER OBSERVATION   Not Encountered   DATE EXCAVATED   9/4/2008	PRO	JECT NO.	7180	PROJECT	Concord	GEOPROBE NO		OU-1C-9
DEPTH FT. DESCRIPTION / SOIL CLASSIFICATION RECOVERY / P.I.D. (ppm)  28 —	LOC	ATION	SEE FIGURE 1	APPROX. ELEV.	1490.03' <u>+</u>	INSPECTE	ED BY	RF/CDM
FT. DESCRIPTION / SOIL CLASSIFICATION RECOVERY / P.I.D. (ppm)  28 —	WAT	ER OBSE	RVATION	Not Encountered		DATE EXC	CAVATED	9/4/2008
28— ———————————————————————————————————			DES	CRIPTION / SOIL CLA	SSIFICATION		RECOVER	(Y / P.I.D.(ppm)
	1		<del></del>	<del>.</del>				
Sample: OU-1C-9B(28.9-30)17:05								
30	29							
30		Sam	ple: OU-1C-9B	(28.9-30')17:05				٠.
32— 33— 33— 34— 35— 35— 36— 37— 38— 40— 40— 41— 41— 42—	30					<b></b> _		
32— 33— 33— 34— 35— 35— 36— 37— 38— 40— 40— 41— 41— 42—	<b>-</b>							
33— — 34— — 35— — 36— — 37— — 38— — 40— — 41— — 41— — 42—	31	End	of Geo-Probe a	t 30 Feet				
33— — 34— — 35— — 36— — 37— — 38— — 40— — 41— — 41— — 42—								
	32							
	33.							
	34							
	_							
	35							
38— 39— 40— 41— 41— 42—	36							
38— 39— 40— 41— 41— 42—								
	3/							
	38							
	39							
 41  42								
42	40							
42								
	41							
		<del></del>						

PRO	JECT NO. 7180 PROJECT Concord	GEOPRO	BE NO.	OU-1C-10
LOC	ATION SEE FIGURE 1 APPROX. ELEV. 1487.46	INSPECT	ED BY	JZ/RF/CDM
WAT	ER OBSERVATION Not Encountered	DATE EX	CAVATED	9/3/2008
DEPTH FT.	DESCRIPTION / SOIL CLASSIFICATION		RECOVER	Y / P.I.D.(ppm)
0	05': Topsoil		R = 58"	
			P.I.D. = 0.0	( 0'-5' )
1	.5'-4.8': Red-Brown medium to fine SAND, little Silt, little Gravel			
2				
3				
4	Sample: OU-1C-10A (.5'-2.5') 16:45			
5_				
			D 600	
6	5'-10': Same w/ Cobbles at 8.4 - 9.1'		R = 59" P.I.D. = 0.0	(55.101)
	Cobbles at 8.4 - 9.1'		1.1.1.2. 0.0	(3-10)
7				į
8	•			
9 -				
"_				
10				
			R = 56"	
11	10'-15': Same		P.I.D. = 0.0	(10'-15')
_				
12				
13		:		
	·			
14				
Time:	SESI	CONSULT	ING ENG	INEERS, PC

Fig. #

PRO	JECT NO.	7180	PROJECT	Concord	GEOPRO	BE NO.	OU-1C-10
LOC	ATION	SEE FIGURE 1	APPROX. ELEV.	1487.46	INSPECT	ED BY	JZ/RF/CDM
WAT	ER OBSER	RVATION	Not Encountered		DATE EXC	CAVATED	9/3/2008
DEPTH FT.		DES	CRIPTION / SOIL CLA	SSIFICATION		RECOVER	Y / P.I.D.(ppm)
14						·	
15			~~~~~				·
16	15'-20	)': Same w/ Col	obles			R = 53"	•.
10					:	P.I.D. = 0.0	)( 15'-20' )
17							
_							
18							
 19							
20							
21	20'-25	: Same				R = 53"	
_	22.5-2		e Encountered			P.I.D. = 0.0	0(20'-25')
22							
						•	
23							
24							
25							
_	25'-30'	': Same	<b></b>		<b></b>	R = 35"	
26		: Moisture Enc	ountered		•	A 33	
27							
					i		
28							
Time:				SE	SICONSULT	ING ENG	INEERS, PC

Fig. #

PRO	JECT NO.	7180	PROJECT	Concord	GEOPRO	BE NO.	OU-1C-10
LOC	ATION	SEE FIGURE 1	APPROX. ELEV.	1487.46	INSPECT	ED BY	JZ/RF/CDM
WAT	ER OBSEI	RVATION	Not Encountered	<u> </u>	DATE EX	CAVATED	9/3/2008
DEPTH FT.		DES	CRIPTION / SOIL CLA	SSIFICATION		RECOVER	Y / P.l.D.(ppm)
28					·····		
29	~						Ï
30	Samp	ole: OU-1C-10E	3 (26.6'-28') 16:50				
	End o	of Geo-Probe at	30 Feet		<del></del>	<del></del> -	
31							
32							
33							
						•	
34							
					;		
35							
_							
36							
37							1
_				·			
38							
-	٠						
39							
40							
41							
42		<u> </u>					
Time:				SESI	CONSULT	ING ENGI	NEERS, PC

PRO	JECT NO. 7180 PROJECT Concord	GEOPRO	BE NO.	OU-1C-11
LOC	ATION SEE FIGURE 1 APPROX. ELEV. 1488.67	INSPECTI	ED BY	RF
WAT	ER OBSERVATION Not Encountered	DATE EX	CAVATED	9/2/2008
DEPTH FT.	DESCRIPTION / SOIL CLASSIFICATION		RECOVER	Y / P.I.D.(ppm)
0	0-7" Topsoil		R = 59"	<del></del> -
	7"-4.9': Red brown medium to fine SAND, little Gravel, little Silt		P.I.D = 0.00	(0-5')
1			<u> </u>	
2	Sample: OU-1C-11A(7"-1.5') 9:00			
3				
_				
4	,			
5				
	5-9.9': Same		R = 59"	
6—			P.I.D = 0.0	(5-10')
7				
<i>'</i> —				
8				
8				
9				
9—				
10				
	10.14.0k.Garara			
11	10-14.9': Same		R = 58"	
'			P.I.D = 0.0	(10-15')
12				
, <b></b>				
13				
14				
Time:		GONSULT	I ING ENG	INEERS, PC

PRO	JECT NO.	7180	PROJECT	Concord	GEOPRO	DBE NO.	OU-1C-11
Loc	ATION	SEE FIGURE 1	APPROX. ELEV.	1488.56	INSPECT	ED BY	RF
WAT	ER OBSE	RVATION	Not Encountered	<del></del>	DATE EX	CAVATED	9/2/2008
DEPTH FT.		DES	CRIPTION / SOIL CLA	SSIFICATION		RECOVER	Y / P.I.D.(ppm)
14							
15	16.1	0.61.0					
16	12-1	8.6': Same				R= 43"	
	Sam	ple: OU-1C-11	B(15.3-18.3) 9:15			P.I.D = 0.0	(15-19')
17	•	•	,				
						!	
18							
19							
19—	End	of Geo-Probe a	t 10 East				
20		UASL AT 19 F					
21							
22						1	
23							
24							
25							
26							
27							
28							
-Time:	8:00-an	n-9:1 <del>5 an</del>		SE	SI-CONSUL	TING ENC	INEERS, PC

PRO	ECT NO. 7180 PROJECT Concord GEOPE	ROBE NO.	OU-1C-12
LOC	ATION SEE FIGURE 1 APPROX. ELEV. 1488.56 INSPEC	TED BY	RF
WAT	ER OBSERVATION Not Encountered DATE E	XCAVATED	9/2/2008
DEPTH FT.	DESCRIPTION / SOIL CLASSIFICATION	RECOVE	RY / P.I.D.(ppm)
0	0-3": Topsoil	R = 33"	
	3"-2.75': Red-brown medium to fine SAND, little Silt, little Gravel	P.I.D = 0.0	(0-2.75')
1—			
2	Sample: OU-1C-12A(3"-2.25") 16:25		
3			
T			
5			
	5-10': Red-brown medium to fine SAND, little Silt, little Gravel w/ Cobbles	n	
6	3-10. Red-blown medium to line SAND, little Silt, little Gravel W Copples	R = 60"	0(5 100
_		P.I.D = 0.0	n(2-10.)
7			
_			
8			
9			:
_			
10			
	10-15': Red-brown medium to fine SAND, little Silt, little Gravel	R = 60"	
11		P.I.D = 0.0	)(10-15')
12	Sample: OU-1C-12B(13.75-15') 16:30		
13			
14—			
Time:	13:30 pm - 16:30 pm SESI-CONSU	LTING EN	3INEERS. PC

PRO	JECT NO	7180	PROJECT	Concord	GEOPRO	BE NO.	OU-1C-12
LOC	ATION SEI	E FIGURE 1	APPROX. ELEV.	1488.56	INSPECTE	D BY	RF
WAT	ER OBSERVA	ATION	Not Encountered		DATE EXC	AVATED	9/2/2008
DEPTH FT.		DES	CRIPTION / SOIL CLA	SSIFICATION		RECOVER	Y / P.I.D.(ppm)
14							
15							
 16	Sample:	15-15.5': OI	U-1C-12B(45-60") 16:	30		R= 0"	
10—						P.I.D = 0.0	(15-15.5')
17			15 Feet 6 Inches EET 6 INCHES				
	KELOSA	TEAL 13 C	EET O INCHES	·			
18							
19							
20							
21							
_							
22							
23							
20—			•				
24							
25							
_							
26							
_							
27							
28							
Time:	13:30 pm - 16	:30 pm		SE	SHOONSULT	ING-ENG	INEERS, PC

PRO	JECT NO.	7180	PROJECT	Concord	GEOPRO	DBE NO.	OU-1C-13
LOC	CATION	SEE FIGURE 1	APPROX. ELEV.	1484.85' <u>+</u>	INSPECT	ED BY	RF/CDM
TAW	TER OBSE	RVATION	Not Encountered		DATE EX	CAVATED	9/4/2008
DEPTH FT.		DES	SCRIPTION / SOIL CLA	SSIFICATION		RECOVER	Y / P.I.D.(ppm)
0	0-6":	Topsoil				R = 32"	
-	6"-2.	.6': Red-brown	SILT, little fine Sand, t	race Gravel		P.I.D = 0.00	( 0'-5.0' )
1						ļ	,
							, <b>;</b>
2	Sam	ole: OU-1C-13	A (6"-1.5') 9:00				
	<u> </u>						
3							
4	[						
	}						
5							
<u></u>	5-10'	: Red-brown SI	LT, little fine Sand, tra	ce Gravel with Bould	erc	R = 60"	
6			and Durin, Mu	oo diavel with bound	CIS	P.I.D = 0.0(	5'-10' \
 7 <u></u> -						1.1.0 0.0(	3-10 )
7							
8							
_							
9							
10							
	10.14			·		· <del></del>	
11_	10-14	': Same				R = 44"	
					ļ	P.I.D = 0.0(	10'-14')
12							
13							
14		·		·	<b>-</b>		
Time:			-	SES	CONSULT	ING ENGI	NEERS, PC

PRO.	JECT NO.	7180	PROJECT	Concord	GEOPRO	BE NO.	OU-1C-13
LOC	ATION SEI	E FIGURE 1	APPROX. ELEV.	1484.85' <u>+</u>	INSPECT	ED BY	RF/CDM
WAT	ER OBSERVA	ATION	Not Encountered		DATE EX	CAVATED	9/4/2008
DEPTH FT.		DES	CRIPTION / SOIL CL	ASSIFICATION		RECOVER	RY / P.I.D.(ppm)
14							
15	14-18': S	ame				R= 44"	
13						P.I.D = 0.0	( 14'-18' )
16							
17							
18						<del> </del>	
	18-20.2':	Same				R = 40"	
19						P.I.D = 0.0	( 18-22' )
20	20.0.00		<b></b>				
21	20.2-23.3	s': Red-brov	m fine SAND, some	Silt, little Gravel with	Boulders		
_							
22						<u> </u>	
_	22-23.2':	Red-brown	SILT, some fine Sar	ıd, little Gravel		R= 36"	
23						P.I.D = 0.0	( 22-26' )
	23.2-26':	Red-brown	fine SAND, some S	ilt, little Gravel with B	Soulders		
24							
25							
26							
	26 27 51.	Ped brown	SIIT little fine Son	d little Gravel with De	v.1d	R=50"	
27	20-21.3:	von-010.MII	om, mue me san	d, little Gravel with Bo	onidet8	R=50 $P.I.D=0.0$	( 26-30) )
						J	(20:00)
28			×				
Time:				SES	SI CONSULT	TING FNC	INEERS, PC

PRO	JECT NO.	7180	PROJECT	Concord	GEOPRO	BE NO.	OU-1C-13
LOC	ATION	SEE FIGURE 1	APPROX. ELEV.	1484.85' <u>+</u>	INSPECT	ED BY	RF/CDM
WAT	ER OBSE	RVATION	Not Encountered	<del></del>	DATE EX	CAVATED	9/4/2008
DEPTH FT.		DES	CRIPTION / SOIL CLA	SSIFICATION		RECOVER	Y / P.I.D.(ppm)
28					·		····-
29							
	Sam	ple: OU-1C-13E	3(27.7-28.5')9:15				
30							
31	End	of Geo-Probe at	t 30 Feet				
32							
33							
-							
34							
35							
36							
~							
37							
38							
30							
39							
40							
41						  -  -	
42							

Time:

PRO	JECT NO.	7180	PROJECT	Concord	GEOPRO	BE NO.	OU-1C-14
LOC	ATION	SEE FIGURE 1	APPROX. ELEV.	1484.23	INSPECT	ED BY	JZ/GP
WAT	ER OBSEI	RVATION	Not Encountered		DATE EX	CAVATED	9/3/2008
DEPTH FT.		DES	CRIPTION / SOIL CLA	SSIFICATION		RECOVER	Y / P.I.D.(ppm)
0—	0-3":			-		R = 36"	
	3"-3'	: Red brown Si	lt, some fine Sand, littl	e Gravel		P.I.D = 0.0	(0-5')
1							:
_	Sam	ple OU-1C-14A	8"-1.6') 15:30				
2							
_							
3							
4							
5							
—	5-10'	: Same				R = 60"	
6						P.I.D = 0.0	(5-10')
-							
7							
							,
8							
_							
9							
_							
10						·	<b></b>
	10-14	4': Same				R = 48"	
11						$P.I.D \approx 0.0$	(10-14)
12							:
13							
14						· <del> </del>	
Time:	3:30 pr	m - 5:45 pm		SES	SI CONSUL	TING ENG	INEERS, PC

PROJ	IECT NO.	7180	PROJECT	PROJECT Concord		GEOPROBE NO.	
LOC	ATION	SEE FIGURE 1	APPROX. ELEV.	1484.23	INSPECTI	ED BY	JZ/GP
WAT	ER OBSEI	RVATION	Not Encountered		DATE EX	CAVATED	9/3/2008
DEPTH FT.	<u></u>	DES	CRIPTION / SOIL CLA	SSIFICATION		RECOVER	RY / P.I.D.(ppm)
14							
	14-1	8': Same				R = 42"	
15						P.I.D = 0.0	(14-18')
—							
16							
17							
						i	
18							<b>-</b>
	18-2	3': Same				R = 52"	
19						P.I.D = 0.0	(18-23')
_							
20							
21							
21							
22							
23							
	23.2	8': Same				R = 58"	
24	25-2	o. Bank				P.I.D = 0.0	1/22 201
						1.1.D – 0.0	7 (23-26)
25							
_							,
26							
-							
27							
-							
28						ļ <b></b>	
Time:	3:30 p	m - 5:45 pm		SE	SI CONSUL	TING EN	SINEERS, PC

PRO.	JECT NO.	7180	PROJECT	Concord	GEOPROBE NO.	OU-1C-14
LOC	ATION	SEE FIGURE 1	APPROX. ELEV.	1484.23	INSPECTED BY	JZ/GP
1A/AT	ER OBSER	PVATION	Not Encountered		—	
VVAI	EK OBSER	WATION	Not Encountered		DATE EXCAVATED	9/3/2008
DEPTH		DES	CRIPTION / SOIL CL	ASSIFICATION	RECOVE	RY / P.I.D.(ppm)
FT.						
28	<b></b>					
	28-33	': Same			$R = 48^{\circ}$	
29					P.I.D = 0.	0 (28-33')
						,
30						
31	:	•				
						1
32						
33						
	33_37	": Same			R = 49"	
34	] ""	. baine				0 (22 25)
					P.I.D = 0.	0 (33-37')
35						
30						
36	 					
37	<b> </b>		<b></b>			
	37-40	': Same			R = 60"	
38						
39						
	Samp	le: OU-1C-14]	B(38-39.75') 15:45			
40			<u> </u>			
	End o	f Geoprobe at	40 Feet			
41		ISAL AT 40 F				
42						
	1					

LOCATION   SEE FIGURE 1   APPROX. ELEV.   1483.08   INSPECTED BY   JZ/GP	PROJ	IECT NO.	7180	PROJECT	Concord	GEOPRO	BE NO.	OU-1C-15
DEPTH   DESCRIPTION / SOIL CLASSIFICATION   RECOVERY / P.I.D.(ppm)	LOCA	ATION	SEE FIGURE 1	APPROX. ELEV.	1483.08	INSPECTI	ED BY	JZ/GP
FT. DESCRIPTION 3 Solt CLASSIPICATION  0	WAT	ER OBSE	 RVATION	Not Encountered		DATE EX	CAVATED	9/3/2008
FT. DESCRIPTION 3 Solt CLASSIPICATION  0	······································							
1.1-1.75': Red brown Silt, some fine Sand, little gravel  Sample: OU-1C-15A(0-1.75') 1400  Sample: OU-1C-15A(0-1.75') 1400  Sample: OU-1C-15A(0-1.75') 1400  Sample: OU-1C-15A(0-1.75') 1400  Sample: OU-1C-15A(0-1.75') 1400  Sample: OU-1C-15A(0-1.75') 1400  R = 60" P.I.D = 0.0 (5-10')  R = 60" P.I.D = 0.0 (10-15')			DES	CRIPTION / SOIL CLA	ASSIFICATION		RECOVER	RY / P.I.D.(ppm)
1 —	0	0-1.1	l': Gray -brown	Silt, some fine Sand, l	ittle Gravel		R = 21"	
		1.1-1	1.75': Red brow	n Silt, some fine Sand,	little gravel		P.I.D = 0.0	(0-5')
3— 4— 4— 5— 5-10': Red brown Sand and Silt, little Gravel R=60" P.I.D = 0.0 (5-10')  7— 8— 9— 10— 10-15': Same R=60" P.I.D = 0.0 (10-15')	1							
3— 4— 4— 5— 5-10': Red brown Sand and Silt, little Gravel R=60" P.I.D = 0.0 (5-10')  7— 8— 9— 10— 10-15': Same R=60" P.I.D = 0.0 (10-15')	_						ļ	
4— 4— 5— 5-10': Red brown Sand and Silt, little Gravel R = 60" P.I.D = 0.0 (5-10')  8— 9— 10— 10- 10-15': Same R = 60" P.I.D = 0.0 (10-15')	2—	Sam	ple: OU-1C-15	A(0-1.75') 1400			1	
4— 4— 5— 5-10': Red brown Sand and Silt, little Gravel R = 60" P.I.D = 0.0 (5-10')  8— 9— 10— 10- 10-15': Same R = 60" P.I.D = 0.0 (10-15')	_							
5— 5— 5-10': Red brown Sand and Silt, little Gravel  R = 60" P.I.D = 0.0 (5-10')  R = 60" P.I.D = 0.0 (10-15')  R = 60" P.I.D = 0.0 (10-15')	1							
5— 5— 5-10': Red brown Sand and Silt, little Gravel  R = 60" P.I.D = 0.0 (5-10')  R = 60" P.I.D = 0.0 (10-15')  R = 60" P.I.D = 0.0 (10-15')				,				
	4							
6— P.I.D = 0.0 (5-10')  7— 7— 8—		5 10	V. Dad brown S	and and Silt little Grav	val		R = 60"	
7	6	3-10	, Ked blown 3	and and one, indic Grav	voi		ļ	) (5-10')
	-							, (5 10)
	7						ļ	
9	_	 						
	8							
	9							
11 	10	}			·		-	
12 <u> </u>		10-1	15': Same				R = 60"	
	11						P.I.D = 0.	0 (10-15')
13	12							
	13							
Time: 2:00 pm - 3:00 pm SESI CONSULTING ENGINEERS, PC	14—	<u> </u>			<u> </u>	el conelli	TING EN	CINEEDS BO

PRO	PROJECT NO. 7180		PROJECT Concord		GEOPRO	BE NO. OU-1C-15
LOC	ATION	SEE FIGURE 1	APPROX. ELEV.	1483.08	INSPECTI	ED BY JZ/GP
WAT	ER OBSE	RVATION	Not Encountered		DATE EX	CAVATED 9/3/2008
					<u> </u>	
DEPTH FT.		DES	CRIPTION / SOIL CLA	SSIFICATION		RECOVERY / P.I.D.(ppm)
14						
15						
_	15-1	9.25': Same				R = 51"
16	-					P.I.D = 0.0 (15-20')
4-7	·					
17						
18						
19						
20						
<u> </u>	20-2	23.5': Same				R = 43"
21	!					P.I.D = 0.0 (20-25')
—						
22						
<u> </u>						
23						
24	1					
25						
	15 1	28.1': Same				R = 38"
26	13-2	20.1 . Danie				P.I.D = 0.0 (25-30')
_						( )
27						
28						
Time:	2:00 [	oni - 3:00 pm		SE	SI CONSUL	TING ENGINEERS, PC

PROJI	ECT NO.	7180	PROJECT	Concord	GEOPRO	BE NO. OU-1C-15
LOCA	TION	SEE FIGURE 1	APPROX. ELEV.	1483.08	INSPECTE	D BY JZ/GP
		RVATION	Not Encountered		DATE EXC	DAVATED 9/3/2008
DEPTH FT.		DES	SCRIPTION / SOIL CL	ASSIFICATION		RECOVERY / P.I.D.(ppm)
28					'	
-						
29						
30				~~~~~~~		
_	30-3	4.5': Same				R = 24"
31	55.5					P.I.D = 0.0 (30-34.5')
	Sam	ple: OU-1C-1:	5B(31-32') 15:00			
32						
33						
34						
• . —     —						
35	End	l of Geoprobe	@ 34 Feet 6 Inches			
_	RE	FUSAL @ 34 ]	FEET 6 INCHES			
36	<u> </u>					
37						
38						
<b>_</b>						
39						
-						
40						
	1			•		
41—						
42						
Time:	2:00	o.m 3:00 p.m.		<u> </u>	SESI CONSU	LTING ENGINEERS, P

Fig. #

PRO	JECT NO.	7180	GEOPRO	DBE NO.	OU-1A-10			
LOC	ATION	SEE FIGURE 1	APPROX. ELEV.	1498.62 <u>+</u>	INSPECT	ED BY	RF/CDM	
WAT	ER OBSEI	RVATION	Not Encountered		DATE EX	CAVATED	8/19/2008	
DEPTH FT.		DES	CRIPTION / SOIL CLA	ASSIFICATION		RECOVERY / P.I.D.(ppm)		
0	0-4":	Fill: Concrete	Debris	<del></del>		R=30"		
	4"-2.	5': Red-brown i	P.I.D. = 0.0	) (0-5')				
1	Weat	thered Sandston	ne from 1.7-2.5'				,	
_								
2	Sam	ple: OU-1A-10A	A (4"-1') 14:45					
<b> </b>								
3								
_								
4								
5						<b></b>		
_			SILT, little medium to		el w/	R=43"		
6			e from 6.4-6.9', 7.2-7.			P.I.D. = 0.0	) (5-7.25')	
	7.25-	8.6': Brown coa	erse to fine SAND, litt	le Silt, little Gravel w/	oil present	P.I.D. = 14	7.0 (7. <b>7</b> ')	
'-	_					P.I.D. = 10	. ,	
_			3 (5.8-6.4') 15:10			P.I.D. = 43	.1 (8.6')	
	Samp	ole: OU-1A-100	C (7.5-8.2') 15:15					
9								
10								
<u> </u>	10-16	).3': Red-brown	coarse to fine SAND,	little Silt little Grave	.]	R=52"		
11			vn Silt, some mottled i			P.I.D. = 10	8 0 (10 2')	
			d Sandstone from 14.1		OII	P.I.D. = 58	, ,	
12	1	,	A TOTAL A TOTA			P.I.D. = 23		
	Samr	ole: OU-1A-10I	0 (12.8-13.8') 15:20			P.I.D. = 17	•	
13	•			P.I.D. = 2.3	· · · · · ·			
						P.I.D. = 0.2	•	
14							· · /	
/ <del></del>						<del></del>	<del></del>	

Time:

2:30p.m.-3:25p.m.

PRO.	PROJECT NO. 7180		PROJECT	PROJECT Concord GE		BE NO.	OU-1A-10
LOC	ATION	SEE FIGURE 1	APPROX. ELEV.	1498.62 <u>+</u>	INSPECTI	ED BY	RF/CDM
WAT	ER OBSEI	RVATION	Not Encountered		DATE EX	CAVATED	8/19/2008
DEPTH FT.		DES	CRIPTION / SOIL CLA	SSIFICATION		RECOVER	Y / P.I.D.(ppm)
14				· · · · · · · · · · · · · · · · · · ·			
	<del></del>						
15			t 14 Feet 9 Inches				
16	REF	USAL ON ROC	CK AT 14 FEET 9 INC	CHES			-
10							
17							
18							
					·		
19							
_	٠						
20							
21							
21 —							
22							
23							
24							
25							
—							
26							
27							
27							
28	•						
Time:	2:30p.n	n3:25p.m.		S	ESI CONSULT	ING ENG	INEERS. PC

PRO	JECT NO.	7180	PROJECT	Concord	GEOPRO	DBE NO.	OU-1A-11
LOC	ATION	SEE FIGURE 1	APPROX. ELEV	·1497.51±	INSPECT	ED BY	RF/CDM
WAT	ER OBSER	VATION	Not Encountered		DATE EX	CAVATED	8/19/2008
DEPTH FT.		DES	CRIPTION / SOIL CL	ASSIFICATION		RECOVER	Y / P.I.D.(ppm)
0	0-1': I	Fill: Concrete	and Asphalt Debris w	/ Weathered Sandstone		R=25"	
-	1-2.1'	: Black-brown	coarse to fine SAND	, little Silt, little Gravel	l, oil present	P.I.D. = 0.0	) (0-5')
1							
2	Sampi	le: OU-1A-11.	A (1.3-2.1') 17:20				
_							
3							
4							
5							
_	•	Same				R=45"	
6	i			fine SAND, little Silt, li		P.I.D. = 0.0	(5-10')
7	1			D, little Silt, little Grave			
/	Weath	nered Sandstor	ne from 7.1-7.3' and V	Veathered Siltstone from	m 8.4-8.75'		
— 8 <u>—</u> 8							
0—							
9							
_							
10							
	10-11	7'- Same w/ W	Veathered Sandstone	from 11-11-4		R=24"	
11				to fine Sand, trace Gra	vel	$R=24^{\circ}$ $P.I.D.=0.0$	) (10, 120
		_ , 1104 010 111	- ~-wa, more mounting	so mic band, trace Gra	.voi	1 .1.D. – 0.0	(10-12)
12	Samol	e: OU-1A-111	3 (11.7-12') 17:25				
	<b></b>	4	· · · · · · · · · · · · · · · · · · ·				
13	End of	f Geo-Probe a	t 12 Feet 9 Inches		<u></u> .	<u> </u>	
<u> </u>			CK AT 12 FEET 9 IN	CHES			
14							

Time:

4:45p.m.-5:25p.m.

PRO.	JECT NO.	GEOPRO	BE NO.	I17-1			
LOC	ATION	SEE FIGURE 1	APPROX. ELEV	1496.46' <u>+</u>	INSPECTI	ED BY	JZ/RF/CDM
WAT	ER OBSE	RVATION	Not Encountered		DATE EX	CAVATED	8/11/2008
DEPTH FT.		DES	CRIPTION / SOIL CL	ASSIFICATION		RECOVER	Y / P.I.D.(ppm)
0	03':	: Concrete and I	Misc. Debris			R = 52"	
_	<u> </u>	P.I.D. = 0.0	( 0.0'-5.0' )				
1	.3'-2.						
	Sand	lstone at (1.8'-2.	.0') and (2.3'-2.4')		!		
2							
3	1			ne SAND, trace Gravel			
4	!	stone at (4.2'-4.	•				
*		IA (.3'-1.2') 11:					
5	117-1	IB (3.8'-4.3') 11	1:50				
	552	s': Concrete and	Miss Debrie			7. 6011	
6			thered Sandstone			R = 60"	V 51 101)
_			hered Sandstone			P.I.D. = 0.0	(3-10')
7		-		D, little Gravel, trace S	Silt		
				ND, little Silt, little Gra			
8				,, 22			
	8.0'-8	8.5': Brown coa	rse to fine SAND, so	me Gravel, little Silt			
9							
	8.5'-1	10.0': Red-Brov	vn medium to fine SA	ND, little Silt, little Gr	avel		
10						<del> </del>	
	10.0'	-12.9': Brown n	nedium to fine SAND	, little Silt, little Grave	1	R = 58"	:
11			P.I.D. = 0.0	( 10'-14' )			
-	12.95						
12		little Gr	m odor				
	I170:	1C (13.4'-14.4')	12:07				
13	050	DD 0 D M	O. I. T. A. D. A. A. S. S. S. S. S. S. S. S. S. S. S. S. S.				
<del></del>			SAL AT 14.5 FEET	m			
,'7	GEO	PROBE COM	PLETE AT 14.5 FEE	<u>T</u>			

Time:

11:00a.m.-12:10p.m.

PRO.	JECT NO.	7180	PROJECT	Concord	GEOPRO	BE NO.	K-18-1
LOC	ATION	SEE FIGURE 1	APPROX. ELEV.	1487.94 <u>+</u>	INSPECT	ED BY	
WAT	ER OBSE	RVATION	Not Encountered		DATE EX	CAVATED	9/5/2008
DEPTH FT.		DES	CRIPTION / SOIL CLA	SSIFICATION		RECOVER	Y / P.I.D.(ppm)
0						R = 39"	-
1		CONCRETE				P.I.D = 0.0	( 0-5' )
' <u></u>	.9'-3.	25': Brown me	edium to fine SAND, lit	tle Gravel, little Si	ilt		
2							
_							
3	Samp	ole: K-18-1A (	.9'-1.83') 9:15				
4_	Samp	ole: K-18-1B (	1.83'-2.9') 9:30				
4—							
5				.==============		 	
_						R = 50"	
6	5'-5.6	5': Same				P.I.D = 0.0	(5'-10')
			red SANDSTONE (coa	rse grained)			
7	Samp	ole: K-18-1C (	5.6'-6.2') 9:45				
88							
	End o	of Geo-Probe a	t 8 Feet				
9		USAL AT 8 FE					
10							
11							
_							
12							
_							
13							
14							·
Time:	9:10	0 - 9:45		<u> </u>	ESI CONSUL	     TING ENG	INEERS. PC

PRO	JECT NO.	7180	PROJECT	Concord	TEST PIT	NO.	L-18-1
LOC	ATION	SEE FIGURE 1	APPROX. ELEV.	1491.89	INSPECT	ED BY	JZ/MZ
WAT	ER OBSE	RVATION	Not Encountered		DATE EX	CAVATED	9/4/2008
DEPTH FT.		DES	CRIPTION / SOIL CLA	ASSIFICATION	<u> </u>	RECOV	ERY / P.I.D.
0		-				R = 37"	
_	03'	:Fill: CONCRE	TE and Misc. Debris			P.I.D.=0.0(	0-5')
1	.3'-2	.2': Brown coar	se to fine SAND, little	Gravel, little Silt			
_							·
2							
3	2.2'-	3.1': Light Brov	wn medium to fine SA	ND, trace Gravel, trac	e Silt		
٥			51 4 651 \ 45 66				
		ple: L-18-1A ( .	•				
<u> </u>	Sam	ple: L-18-1B ( 2	2.1 - 2.9" ) 15:45				
5	<b></b>						
						R = 33"	
6	5'-5.	67': Same				P.I.D. = 0.0	)( 5'-10' )
	5.67	'-6.67': Brown !	SILT, trave Gravel, tra	ice Silt			(, ,,
7	6.67	'-7.75': Brown i	medium to fine SAND	, little Gravel, trace S	ilt		:
-							
8							
_							
9—							
10							
11	0-11	.9': Brown SIL'	T, little Gravel, trace S	Silt with Boulders		R = 23"	
' '	g	mlo. T 10 10 / 1	1.351.11.013.16-00			P.I.D. = 0.0	)( 10'-12' )
12	Sam	рю: r-18-1С ( 1	1.25'-11.9' ) 16:00				
	End	of Geo-Probe a	t 12 Feet				
13		USAL AT 12 F					
		<b></b>					
14							

PRO	JECT NO.	7180	PROJECT	Concord	GEOPRO	BE NO.	L19
LOC	ATION	SEE FIGURE 1	APPROX. ELE	v	INSPECT	ED BY	JZ
WAT	ER OBSE	RVATION	Not Encountered		DATE EX	CAVATED	8/21/2008
DEPTH FT.		DES	SCRIPTION / SOIL C	CLASSIFICATION		RECOVER	Y / P.I.D.(ppm)
0	0-3.6	: Fill: Brown c	oarse to fine SAND,	some medium to f	ine Gravel, little Silt,	R = 44"	
	w/ we	eathered Sands	tone, Concrete, Bric	k		P.I.D = 0.0	
1							
<u> </u>							·
2	L-19-	·1A (6"-1.5') 9	:30 am				
	L-19-	·1B (3-3.6') 9:4	15 am	•		: :	
3							
-							
4							
_							
5						 	
_	5-6':	Same as above				R = 30"	
6	6-6.6'	: Gray coarse to	fine Sand, some coars	se to fine weathered	Sandstone, trace Silt	P.I.D = 0.0	
	6.6-7	.5': Fractured/	weathered Sandstone	e			
7	L-19-	1C(5-6') 10:00	) am				
8							
]							
9							
10							
11							
12							
13							
-							
14				<u> </u>			
Time:	8:30 am	- 10:00 am			SESI CONSULT	ING ENG	INEERS, PC

Time:



CONTRACTOR, LABOR & EQUIPMENT		
QUANTITY	DESCRIPTION	
1	Crew for Excavation-Tano	
1	Crew for Footings-Skyline Concrete	

DATE: 9/12/2008	<b>JOB NO. 5485</b>	
PROJECT: Concord		
LOCATION: Lake Kiames	ha, NY	
contractor: Fuller		
owner: Capelli		
WEATHER: Sunny TEMP: Low 60's		
PRESEN	IT AT SITE	
Oscar Nordstrom/Brian	Tetor-Fuller	
George Rivera-Skyline	Concrete	
Eric Beck-Tano	·	

CHARGABLE EQUIPMENT USED	
Nuclear Density Gauge	

## **GEOTECH INSPECTION REPORT**

- Excavated 2 to 3 feet below existing finished floor subgrade to a firm natural footing subgrade or solid bedrock for Casino Footings C724, C339, C717, C350 to C356 and compacted with double-drum sheepsfoot roller. Footings C717, C350, C353 to C355 needed rock removal to achieve subgrade. Visual observation and probing of the footing subgrades verified a footing subgrade of 4 TSF bearing capacity.
- Excavated material from footings was taken to the roadway north of Sediment Basin #2 and one 12-inch thick loose lift were placed from elevation ± 13' above former tennis court elevation in the southern part of the north of Sediment Basin #2 roadway, and compacted with single smooth drum 10-ton roller and single drum sheepsfoot 10-ton roller. Nuclear density testing, visual observation and probing of the subgrade verified the lift was placed in accordance with design specifications.
- Casino Footings C210 to C212, C221 to C223, C232 to C234, C245 to C246, C255 to C258, C264 to C270, C277 to C283, C900, C288 to C294, C891, C304 to C305, C880 were backfilled with two 12-inch thick lifts of 1-½-inch crushed bedrock and recycled concrete aggregate. Casino Footings C231, C241 to C244, C253 to C254, C262, to C263, C273 to C276, C284 to C287 were backfilled with one 12-inch thick lift of 1-½-inch crushed bedrock and recycled concrete aggregate and will be completed on 9/15/08. Footings C210 to C212, C221 to C223, C231 to C232, C241 to C244 had soft material at bottom of excavation removed before placement of fill materials. Nuclear density testing, visual observation and probing of the subgrade verified the lifts were placed in accordance with design specifications. Pending a proof-roll of 2<sup>nd</sup> lift footings backfill is approved.

Page 1 of 1 Inspector: Robert Fioretti



CONTRACTOR, LABOR & EQUIPMENT			
QUANTITY	DESCRIPTION		
1	Crew for Excavation-Tano		
1	Crew for Footings-Skyline Concrete		

DATE: 9/3/2008	<b>JOB NO. 5485</b>	
PROJECT: Concord		
LOCATION: Lake Kiames	sha, NY	
contractor: Fuller		
owner: Capelli		
WEATHER: Sunny TEMP: Mid 80's		
PRESE	NT AT SITE	
Oscar Nordstrom/Al D	estefano-Fuller	
George Rivera-Skyline	Concrete	
Eric Beck-Tano		
Mark Skellet-MHE (T	ownship Representative)	
Mark Skellet-Mile (1	ownship Representative)	

CHARGABLE EQUIPMENT USED	
Nuclear Density Gauge	
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# **GEOTECH INSPECTION REPORT**

- Excavated 2 to 3 feet below existing finished floor subgrade to a firm natural footing subgrade or
  previously approved compacted fill footing subgrade for Casino Footings C246 to C250, C927, C235 to
  C239, C933, C224 to C228, C938, C213 to C217, C941 and compacted with doubledrum sheepsfoot
  roller. Visual observation and probing of the footing subgrades verified a footing subgrade of 4 TSF
  bearing capacity.
- The excavated material from the footings was taken to the node line 13.5 to 14.5 from J to H and two 12-inch thick loose lifts of natural on-site soil was placed and compacted with single drum sheepsfoot 10-ton roller and a single smooth drum 10-ton roller. Nuclear density testing, visual observation and probing of the subgrade verified the lift was placed in accordance with design specifications. Some excavated material was placed in a stockpile nearby the footings approved and poured 9/2/08 for backfill tomorrow.
- Soil excavated during the construction of Sediment Basin #1 was placed in a 12-inch loose lift of on-site fill material in roadway to the north of Sediment Basin #2, but when placed noticeable amounts of large debris was visible. Lift was not approved. Al Beck informed us the soil would be removed and sorted before use.

Inspector:	Robert	Fioretti
Signature		



CONTRACTOR, LABOR & EQUIPMENT			
QUANTITY	DESCRIPTION		
1	Crew for Excavation-Tano		
1	Crew for Footings-Skyline Concrete		
	1/ABL		

~ 1	
ркојест: Concord	
LOCATION: Lake Kiam	esha, NY
CONTRACTOR: Fuller	
owner: Capelli	<del></del>
weather: Sunny	темр: Mid 70's
PRES	SENT AT SITE
Oscar Nordstrom/Bri	an Tetor-Fuller
George Rivera-Skylin	ne Concrete
Eric Beck-Tano	·

CHARGABLE EQUIPMENT USED	
Nuclear Density Gauge	
	-
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	· -

## GEOTECH INSPECTION REPORT

- Excavated 2 to 3 feet below existing finished floor subgrade to a firm natural footing subgrade for Casino Footings C711, C361, C362, C370, C371, C817 and compacted with double-drum sheepsfoot roller. Footing C371, C817 was over-excavated 6 to 12-inches due to slight pumping and backfilled with recycled concrete aggregate. Visual observation and probing of the footing subgrades verified a footing subgrade of 4 TSF bearing capacity.
- Excavated material from footings, from node line 18 from approximate node line K to L and from node line 17 from approximate node line L to N was taken to the roadway north of Sediment Basin #2 and two 12-inch thick loose lift were placed from elevation ±14' to ±15' above the former tennis court elevation, on southern end of roadway section and compacted with single smooth drum 10-ton roller. Nuclear density testing, visual observation and probing of the subgrade verified the lift was placed in accordance with design specifications.
- Casino Footings C774, C251 to C252, C756, C295 to C303, C749, C306 to C312, C741, C317 to C323, C732, C328 to C333, C724, C339 to C341 were backfilled with two 12-inch thick lifts of recycled concrete aggregate. Casino Footings C779, C240 to C244, C253 to C254, C769, C262 to C263, C765, C273 to C276, C761, C284 to C287, C313, C324, C334 to C335 were backfilled with one 12-inch thick lift of recycled concrete aggregate. Nuclear density testing, visual observation and probing of the subgrade verified the lifts were placed in accordance with design specifications. Pending a proof-roll of 2<sup>nd</sup> lift the footing backfill in this area is approved.

Inspector: Robert Fioretti	
Signature	



CONTRACTOR, LABOR & EQUIPMENT	
QUANTITY	DESCRIPTION
1	Crew for Excavation-Tano
1	Crew for Footings-Skyline Concrete
<del></del>	

date: 9/9/2008	JOB NO. 5485	
PROJECT: Concord		
LOCATION: Lake Kiame	sha, NY	
CONTRACTOR: Fuller	-	
owner: Capelli		
WEATHER: Rain TEMP: High 60's		
PRESENT AT SITE		
Oscar Nordstrom/Brian Tetor-Fuller		
George Rivera-Skyline Concrete		
Eric Beck-Tano		

CHARGABLE EQUIPMENT USED	

## GEOTECH INSPECTION REPORT

- Excavated 2 to 3 feet below existing finished floor subgrade to a firm natural footing subgrade for Casino Footings C315 and compacted with double-drum sheepsfoot roller. Three Casino Footings C314, C302, and C303 were re-inspected from yesterday because they were not poured and it was raining at the time of pour. Footings remained approved. Visual observation and probing of the footing subgrades verified a footing subgrade of 4 TSF bearing capacity.
- A rock outcrop located at approximate node H-16 was jack hammered. All rock was brought to the crusher and crushed into 1-½-inch stone. The stone will be reused tomorrow during backfill of footings if the bottoms are moist.

Page 1 of 1

Inspector: Robert Fioretti



CONTRACTOR, LABOR & EQUIPMENT	
QUANTITY	DESCRIPTION
1	Crew for Excavation-Tano
1	Crew for Footings-Skyline Concrete
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DATE: 9/8/2008	<b>јов no.</b> 5485
PROJECT: Concord	
LOCATION: Lake Kiamesh	ıa, NY
contractor: Fuller	
owner: Capelli	
WEATHER: Sunny TEMP: Mid 70's	
PRESEN	Γ AT SITE
Oscar Nordstrom/Brian	Tetor-Fuller
George Rivera-Skyline	Concrete
Eric Beck-Tano	

CHARGABLE EQUIPMENT USED
Nuclear Density Gauge

## GEOTECH INSPECTION REPORT

- Excavated 2 to 3 feet below existing finished floor subgrade to a firm natural footing subgrade for Casino Footings C749, C306 to C314, C756, C295 to C304 and compacted with double-drum sheepsfoot roller. Three footings were over-excavated 6 to 12 inches, C296, C306, and C303 and filled with natural on-site material due to pumping during compaction. Visual observation and probing of the footing subgrades verified a footing subgrade of 4 TSF bearing capacity.
- Excavated material from footings and from node line 16 and 17 from approximate node line J to M was taken to the roadway north of Sediment Basin #2 and three 12-inch thick loose lifts were placed from elevation ± 13' to ± 16' above former tennis court elevation, and compacted with single smooth drum 10-ton roller and single drum sheepsfoot 10-ton roller. Nuclear density testing, visual observation and probing of the subgrade verified the lift was placed in accordance with design specifications.

Inspector: Robert Fioretti



CONTRACTOR, LABOR & EQUIPMENT	
QUANTITY	DESCRIPTION
1	Crew for Excavation-Tano
1	Crew for Footings-Skyline Concrete

date: 9/11/2008	<b>ЈОВ NO. 5485</b>	
PROJECT: Concord		
LOCATION: Lake Kiamesha, NY		
CONTRACTOR: Fuller		
owner: Capelli		
WEATHER: Sunny TEMP: Low 70's		
PRESENT AT SITE		
Oscar Nordstrom/Brian Tetor-Fuller		
George Rivera-Skyline Concrete		
Eric Beck-Tano		
	Lyla i	

CHARGABLE EQUIPMENT USED	
Nuclear Density Gauge	,
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### GEOTECH INSPECTION REPORT

- Excavated 2 to 3 feet below existing finished floor subgrade to a firm natural footing subgrade or solid bedrock for Casino Footings C340 to C349, C837, C357 to C360, C826 and compacted with double-drum sheepsfoot roller. Footings C343 to C345 needed rock removal to achieve subgrade. Visual observation and probing of the footing subgrades verified a footing subgrade of 4 TSF bearing capacity.
- Excavated material from footings was taken to the roadway north of Sediment Basin #2 and two 12-inch thick loose lifts were placed from elevation ± 11' to ± 12' above former tennis court elevation in the southern part of the north of Sediment Basin #2 roadway, and compacted with single smooth drum 10-ton roller and single drum sheepsfoot 10-ton roller. Nuclear density testing, visual observation and probing of the subgrade verified the lifts were placed in accordance with design specifications.
- Casino Footings C213 to C217, C941, C224 to C228, C938, C235 to C239, C933, C247 to C250, C927, C259 to C261, C919, C271 to C272, C909 were backfilled with two 12-inch thick lifts of 1-½-inch crushed bedrock and recycled concrete aggregate. Footings C249 to C250, C927, C238 to C239, C933, C224 to C228, C938, C213 to C217, C941 had soft material at bottom of excavation removed before placement of fill materials. Nuclear density testing, visual observation and probing of the subgrade verified the lifts were placed in accordance with design specifications. Pending a proof-roll of 2<sup>nd</sup> lift footings backfill is approved.

Page 1 of 1

Inspector: Robert Fioretti



CONTRACTOR, LABOR & EQUIPMENT	
QUANTITY	DESCRIPTION
1	Crew for Excavation-Tano
1	Crew for Footings-Skyline Concrete

NY		
VY		
мр: High 80's		
PRESENT AT SITE		
Oscar Nordstrom/Al Destefano-Fuller		
George Rivera-Skyline Concrete		
Mark Skellet-MHE (Township Representative)		

CHARGABLE EQUIPMENT USED	
Nuclear Density Gauge	
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### GEOTECH INSPECTION REPORT

- Excavated 2 to 3 feet below existing finished floor subgrade to a firm natural footing subgrade for Casino Footings C761, C284 to C289, C765, C273 to C278, C769, C262 to C267, C774, C251 to C256 and compacted with doubledrum sheepsfoot roller. Visual observation and probing of the footing subgrades verified a footing subgrade of 4 TSF bearing capacity. Other footings were excavated and will be expected tomorrow when forming begins.
- Excavated material from footings was placed in a 12-inch thick loose lift of natural on-site soil along the future location of footings C781, C785, C787, C203, C202, C381, C384, and C385, to achieve finished floor subgrade, and compacted with single drum sheepsfoot 10-ton roller. Nuclear density testing, visual observation and probing of the subgrade verified the lift was placed in accordance with design specifications.
- Remaining excavated material from the footings and from the recently cleared 16 node line from approximate node line J to K was taken to the south of Sediment Basin #1 and stockpiled for construction of the Sediment Basin #1 berms.
- Unapproved 12-inch think loose lift placed yesterday at elevation ±9' above former tennis court in Sediment Basin #2 was sorted removing all debris and compacted with a single smooth drum 10-ton roller. Nuclear density testing, visual observation and probing of the subgrade verified that lift was placed in accordance with design specifications.

Page 1 of 1	Inspector: Robert Fioretti
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CONTRACTOR, LABOR & EQUIPMENT		
QUANTITY	DESCRIPTION	
1	Crew for Excavation-Tano	
1	Crew for Footings-Skyline Concrete	

date: 9/5/2008	JOB NO. 5485	
PROJECT: Concord		
LOCATION: Lake Kiamesha, NY		
CONTRACTOR: Fuller		
owner: Capelli		
weather: Sunny	TEMP: Mid 80's	
PRESENT AT SITE		
Oscar Nordstrom/Al Destefano-Fuller		
George Rivera-Skyline Concrete		
Eric Beck-Tano		
Mark Skellet-MHE (Township Representative)		

CHARGABLE EQUIPMENT U	SED
Nuclear Density Gauge	
100 1	

### GEOTECH INSPECTION REPORT

- Excavated 2 to 3 feet below existing finished floor subgrade to a firm natural footing subgrade for Casino Footings C290 to C294, C891, C279 to C283, C900, C268 to C272, C909, C257 to C261, C919 and compacted with double-drum sheepsfoot roller. Visual observation and probing of the footing subgrades verified a footing subgrade of 4 TSF bearing capacity. Some footings above were previously excavated yesterday 9/4/08.
- Excavated material from cleared node lines 16 and 17 from approximate node line J to M was placed in 12-inch thick loose lifts of natural on-site soil around Casino Footings. Two lifts were placed and compacted with a double-drum sheepsfoot roller around Casino Footings C788, C207 to C210, C786, C218 to C220, C782, C229 to C231. One lift was placed and compacted with a double-drum sheepsfoot roller around Casino Footings C779, C240 to C244, C232 to C233, C221 to C222, C211. Nuclear density testing, visual observation and probing of the subgrade verified the lift was placed in accordance with design specifications.
- Remaining excavated material from node line 16 and 17 from approximate node line J to M, and the excavated footing material was taken to the roadway north of Sediment Basin #2 and three 12-inch thick loose lifts were placed from elevation ± 10' to ± 13' above former tennis court elevation, and compacted with single smooth drum 10-ton roller. Nuclear density testing, visual observation and probing of the subgrade verified the lift was placed in accordance with design specifications.

Inspector: Robert Fioretti	
Signature	



CONTRACTOR, LABOR & EQUIPMENT		
QUANTITY	DESCRIPTION	
1	Crew for Excavation-Tano	
1	Crew for Footings-Skyline Concrete	

DATE: 9/10/2008	<b>JOB NO. 5485</b>
PROJECT: Concord	
LOCATION: Lake Kiames	sha, NY
contractor: Fuller	
owner: Capelli	
weather: Sunny	TEMP: Mid 60's
PRESE	NT AT SITE
Oscar Nordstrom/Brian	n Tetor-Fuller
George Rivera-Skyline	e Concrete
Eric Beck-Tano	<del></del>

CHARGABLE EQUIPMENT USEI	<b>—</b>	
Nuclear Density Gauge	-	

### GEOTECH INSPECTION REPORT

- Excavated 2 to 3 feet below existing finished floor subgrade to a firm natural footing subgrade or solid bedrock for Casino Footings C305, C880, C316, C869, C741, C317 to C327, C858, C732, C328 to C338, C847 and compacted with double-drum sheepsfoot roller. One Casino Footings C304 was re-inspected from 3/8/08 because it was not poured and it rained. Footing C316 was over-excavated 6 to 12-inches due to slight pumping and backfilled with 1-½-inch crush bedrock. Footings C741, C317, C319 and C320 were over-excavated 4 to 6-inches due to slight pumping and backfilled with on-site material and compacted with double-drum sheepsfoot roller. Footings C321 to C232 needed rock removal to achieve subgrade. Visual observation and probing of the footing subgrades verified a footing subgrade of 4 TSF bearing capacity.
- Excavated material from footings and from node line 17 and 17.5 from approximate node line L to M was taken to the roadway north of Sediment Basin #2 and three 12-inch thick loose lifts were placed from elevation ± 8' to ± 10' above former tennis court elevation in the southern part of the north of Sediment Basin #2 roadway, and compacted with single smooth drum 10-ton roller and single drum sheepsfoot 10-ton roller. Nuclear density testing, visual observation and probing of the subgrade verified the lifts were placed in accordance with design specifications.

Page 1 of 1

Inspector: Robert Fioretti



CONTRACTOR, LABOR & EQUIPMENT	
QUANTITY	DESCRIPTION
1	Crew for Excavation-Tano
1	Crew for Footings-Skyline Concrete
	=

DATE: 9/2/2008	<b>J</b> ов no. 5485	
PROJECT: Concord		
LOCATION: Lake Kiamesha, NY		
contractor: Fuller		
owner: Capelli		
WEATHER: Sunny	темр: Mid 80's	
PRESENT AT SITE		
Oscar Nordstrom-Fuller		
George Rivera-Skyline	Concrete	
Eric Beck-Tano		
Mark Skellet-MHE (Township Representative)		

CHARGABLE EQUIPMENT USED		
Nuclear Density Gauge		
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- Placed final 12-inch thick loose lift in area of Congress Hotel with natural on-site soil, originating from excavation of node line 16 to 17 from K to half way between M and N to finished floor subgrade, and compacted with single drum sheepsfoot 10-ton roller and single drum smooth 10-ton roller. Nuclear density testing, visual observation and probing of the subgrade verified the lift was placed in accordance with design specifications. Ganesh Persaud performed density testing.
- Excavated 2 to 3 feet below existing finished floor subgrade to a firm natural footing subgrade or previously approved compacted fill footing subgrade for Casino Footings C779, C240 to C245, C782, C229 to C234, C786, C218 to C223, C788, C207 to C212 and compacted with walk-behind double drum sheepsfoot roller. Visual observation and probing of the footing subgrades verified a footing subgrade of 4 TSF bearing capacity. Ganesh Persaud performed footing inspection.
- The additional excavated material, from the footings and excavated material from the excavation of node line 16 to 17 from K to half way between M and N, was taken to node line 13 to 14.5 from J to I. Two 12-inch thick loose lifts of natural on-site soil was placed and compacted with a single drum sheepsfoot 10-ton roller and a single smooth drum 10-ton roller. Nuclear density testing, visual observation and probing of the subgrade verified the lift was placed in accordance with design specifications. Ganesh Persaud performed density testing.

Inspector: Robert Fioretti	
Signature	



CONTRACTOR, LABOR & EQUIPMENT	
QUANTITY	DESCRIPTION
1	Crew for Excavation-Tano
1	Crew for Footings-Skyline Concrete

DATE: 9/25/2008	<b>JOB NO.</b> 5485
PROJECT: Concord	
LOCATION: Lake Kiame	esha, NY
CONTRACTOR: Fuller	
owner: Cappelli	
weather: Sunny	темр: High 60's
PRES	ENT AT SITE
Oscar Nordstrom/Bria	an Tetor-Fuller
George Rivera-Skylin	ne Concrete
Eric Beck-Tano	
-,4	

CHARGABLE EQUIPMENT USED	
Nuclear Density Gauge	
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- Excavated 5 to 6 feet below existing finished floor subgrade to a firm natural footing subgrade for Casino Footing C882, however the elevation of the footing bottom changed to 2 to 3 feet below existing finished floor subgrade and two 12-inch thick lifts of recycled concrete aggregate were placed and compacted with double-drum sheepsfoot roller to bring footing to subgrade. Visual observation and probing of the footing subgrades verified a footing subgrade of 4 TSF bearing capacity.
- As part of the changed footing subgrades, Casino Footings C860 and C871 were removed.
- Excavated material from footing excavation, from hotel excavation and from node lines 17 to 18 from approximate node line L to N was taken to the tennis court area northeast of Sediment Basin #2 and two 12-inch thick loose lift were placed from elevation +1' to +3' above the former tennis court and compacted with single smooth drum 10-ton roller. Nuclear density testing, visual observation and probing of the subgrade verified the lift was placed in accordance with design specifications.
- Excavation continued for hotel towers in the northeast portion of the hotel. The excavation will be going to sound rock for a 10 TSF bearing capacity. A +8' cut was excavated today.
- Placement of coarse R.C.A. from crushed demolition concrete stockpile continued in previously proofrolled area for slab-on-grade from Casino Footings C788/C207 to C217/C941 to C371/C817 to C711/C361. Placed 7-inches of R.C.A. and compacting with single smooth drum 10-ton roller.

e 1 of 2	Inspector: Robert Fioretti
	Signature

- Excavation for UST-12 removal was backfill with three 12-inch thick lifts of natural on-site material This completed the backfill of this excavation. Visual observation and probing of the subgrade verified the lifts were placed in accordance with design specifications.
- Excavation for UST-8 removal was backfilled today, due to approaching rain storm. No visual observation was performed on backfill operation. This excavation is located directly under the hotel tower and will be removed to bedrock at a later date.
- Casino Footings C371/C817, C360/C826, C349/C837, C338/C847, C327/C858 were excavated on their north side on 9/23/08 for an expansion of concrete. The footings were backfilled due to approaching rainstorm. Footing expansions will be excavated again at a later date.
- Casino Footings C881 and C892 were backfilled with two 12-inch thick lifts of natural on-site material to top of footings. Nuclear density testing, visual observation and probing of the subgrade verified the lifts were placed in accordance with design specifications. Pending a proof-roll of 2<sup>nd</sup> lift the footing backfill in this area is approved.

Page 2 of 2

Inspector: Robert Fioretti



CONTRACTOR, LABOR & EQUIPMENT	
QUANTITY	DESCRIPTION
1	Crew for Excavation-Tano
1	Crew for Footings-Skyline Concrete

DATE: 9/30/2008	<b>ЈОВ NO. 5485</b>	
PROJECT: Concord		
LOCATION: Lake Kiamesha, NY		
contractor: Fuller	12-7-	
owner: Cappelli	·	
WEATHER: P. Cloudy	темр: Mid 70's	
PRESENT AT SITE		
Oscar Nordstrom/Brian Tetor-Fuller		
George Rivera-Skyline Concrete		
Eric Beck-Tano	<u> </u>	

CHARGABLE EQUIPMENT USED	
Nuclear Density Gauge	
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- Excavated 2 to 3 feet below existing finished floor subgrade to a firm natural footing subgrade for Casino Footings C871 and C883 to a firm natural footing subgrade. Visual observation and probing of the footing subgrades verified a footing subgrade of 4 TSF bearing capacity. Casino Footing C882, previously approved 9/25/08, was re-approved.
- As part of the changed footing subgrades, Casino Footings C872 was removed and excavated 6 to 7 feet below existing finished floor subgrade to rock subgrade. Groundwater is present at the bottom 2 to 3 feet. A sump pump was set up to allow for forming and pouring. Footing subgrade will be approved prior to pour.
- During excavation of Casino Footing C860, contamination was found at footing subgrade elevation.
   Environmental testing was performed and footing approval will remain pending until results are attained.
   All excavated material after contamination was found was brought to the Temporary Contaminated Stockpile.
- Excavated material from footing excavations and from node lines 16 to 18 from approximate node line L to N was taken to the tennis court area east of Sediment Basin #2 and three 12-inch thick loose lift were placed from elevation ±1' to ±4' above the former tennis court and compacted with single smooth drum 10-ton roller. Nuclear density testing, visual observation and probing of the subgrade verified the lift was placed in accordance with design specifications.

Inspector: Robert Fioretti	
Signature	

- Casino Footings C861 and C873 were backfilled with two 12-inch thick lifts of recycled concrete aggregate and C862, C863, C874 and C875 were backfilled with one 12-inch thick lifts of recycled concrete aggregate. Nuclear density testing, visual observation and probing of the subgrade verified the lifts were placed in accordance with design specifications. Pending a proof-roll of 2<sup>nd</sup> lift the footing backfill in this area is approved.
- Excavated 2 to 3 feet below existing finished floor subgrade to a previously approved firm natural footing subgrade for footing expansion on north side of Casino Footings C371/C817, C360/C826, C349/C837, C338/C847 and C327/C858. The footings were changed and need to be expanded. Visual observation and probing of the expansion area of footing subgrades verified a footing subgrade of 4 TSF bearing capacity.

Inspector: Robert Fioretti



CONTRACTOR, LABOR & EQUIPMENT	
QUANTITY	DESCRIPTION
1	Crew for Excavation-Tano
1	Crew for Footings-Skyline Concrete

DATE: 9/19/2008	<b>JOB NO.</b> 5485			
PROJECT: Concord	PROJECT: Concord			
LOCATION: Lake Kiamesha, NY				
CONTRACTOR: Fuller				
owner: Cappelli	<u> </u>			
WEATHER: Sunny	TEMP: Low 60's			
PRES	PRESENT AT SITE			
Oscar Nordstrom/Brian Tetor-Fuller				
George Rivera-Skylin	ne Concrete			
Eric Beck-Tano				
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CHARGABLE EQUIPMENT USI	ED
Nuclear Density Gauge	

- Excavated 2 to 3 feet below existing finished floor subgrade to a firm natural footing subgrade for Casino Footings C859 to C860, C870 to C871 and compacted with double-drum sheepsfoot roller. Visual observation and probing of the footing subgrades verified a footing subgrade of 4 TSF bearing capacity. Deo Persaud performed inspections.
- Excavation began for hotel towers in the northeast portion of the hotel. The excavation will be going to sound rock for a 10 TSF bearing capacity. A  $\pm 8$ ° cut was excavated today.
- Excavated material from footing excavations, from hotel excavation, from node line 18 from approximate node line K to L and from node line 17 from approximate node line L to N was taken to the roadway north of Sediment Basin #2 and one 12-inch thick loose lift were placed from elevation ±18' to ±19' above the former tennis court elevation on southern end of roadway section and compacted with single smooth drum 10-ton roller. This elevation is subgrade for the roadway. The remainder of material was placed on the slope of the roadway in 12-inch thick loose lifts and compacted with single smooth drum 10-ton roller. Nuclear density testing, visual observation and probing of the subgrade verified the lift was placed in accordance with design specifications. Deo Persaud and Rob Fioretti performed inspections.
- Placement of coarse R.C.A. from crushed demolition concrete stockpile continued in previously proof-rolled area for slab-on-grade from Casino Footings C788, C207 to C761, C284 to C217, C941 to C294, C891. Placed 7-inches of R.C.A. and compacting with single smooth drum 10-ton roller.

Page 1 of 1	Inspector: Robert Fioretti
	Signature



CONTRACTOR, LABOR & EQUIPMENT		
QUANTITY	DESCRIPTION	
1	Crew for Excavation-Tano	
1	Crew for Footings-Skyline Concrete	

DATE: 9/24/2008	<b>ЈОВ NO. 5485</b>		
PROJECT: Concord			
LOCATION: Lake Kiames	na, NY		
CONTRACTOR: Fuller			
owner: Cappelli			
WEATHER: Sunny	темр: High 60's		
PRESENT AT SITE			
Oscar Nordstrom/Brian Tetor-Fuller			
George Rivera-Skyline Concrete			
Eric Beck-Tano			

CHARGABLE EQUIPMENT USED	
Nuclear Density Gauge	<u>,</u>
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- Excavated 2 to 3 feet below existing finished floor subgrade to a firm natural footing subgrade for Casino Footings C861 to C863 and C872 to C875 and compacted with double-drum sheepsfoot roller. Visual observation and probing of the footing subgrades verified a footing subgrade of 4 TSF bearing capacity.
- Excavated material from footing excavations, from hotel excavation and from node lines 17 to 18 from approximate node line L to N was taken to the tennis court area northeast of Sediment Basin #2 and one 12-inch thick loose lift was placed from elevation ±0' to ±1' above the former tennis court and compacted with single smooth drum 10-ton roller. Nuclear density testing, visual observation and probing of the subgrade verified the lift was placed in accordance with design specifications.
- Excavation continued for hotel towers in the northeast portion of the hotel. The excavation will be going to sound rock for a 10 TSF bearing capacity. A ±8' cut was excavated today.
- Placement of coarse R.C.A. from crushed demolition concrete stockpile continued in previously proofrolled area for slab-on-grade from Casino Footings C788/C207 to C217/C941 to C371/C817 to C711/C361. Placed 7-inches of R.C.A. and compacting with single smooth drum 10-ton roller.
- Excavation for UST-12 removal was backfill with four 12-inch thick lifts of natural on-site material.
   Remainder will be performed tomorrow. Visual observation and probing of the subgrade verified the lifts were placed in accordance with design specifications.

Page 1 of 1	Inspector: Robert Fioretti	
	Signature	



CONTRACTOR, LABOR & EQUIPMENT	
QUANTITY	DESCRIPTION
1	Crew for Excavation-Tano
1	Crew for Footings-Skyline Concrete

DATE: 9/26/2008	<b>ЈОВ NO. 5485</b>		
PROJECT: Concord			
LOCATION: Lake Kiamesl	LOCATION: Lake Kiamesha, NY		
CONTRACTOR: Fuller			
owner: Cappelli			
weather: Rain	TEMP: Low 60's		
PRESENT AT SITE			
Oscar Nordstrom/Brian Tetor-Fuller			
George Rivera-Skyline Concrete			
Eric Beck-Tano			

CHARGABLE EQUIPMENT USED	
Nuclear Density Gauge	
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## **GEOTECH INSPECTION REPORT**

- Excavated material from node lines 17 to 18 from approximate node line L to N was stockpiled at upper east tennis court area for use when condition permit.
- Placement of coarse R.C.A. from crushed demolition concrete stockpile continued in previously proof-rolled area for slab-on-grade from Casino Footings C788/C207 to C217/C941 to C371/C817 to C711/C361. Placed 7-inches of R.C.A. and compacting with single smooth drum 10-ton roller.
- No other construction work was performed due to rain.

Page	l of 1	
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Inspector: Robert Fioretti



CONTRACTOR, LABOR & EQUIPMENT		
QUANTITY	DESCRIPTION	
1	Crew for Excavation-Tano	
1	Crew for Footings-Skyline Concrete	
-		
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date: 9/16/2008	JOB NO. 5485	
PROJECT: Concord		
LOCATION: Lake Kiames	ha, NY	
CONTRACTOR: Fuller		
owner: Capelli		
WEATHER: P. Cloudy	темр: Mid 60's	
PRESENT AT SITE		
Oscar Nordstrom/Brian	Tetor-Fuller	
George Rivera-Skyline Concrete		
Eric Beck-Tano		

CHARGABLE EQUIPMENT USED
Nuclear Density Gauge
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### **GEOTECH INSPECTION REPORT**

- Excavated material from node line 18 from approximate node line K to L and from node line 17 from approximate node line L to N was taken to the roadway north of Sediment Basin #2 and two 12-inch thick loose lift were placed from elevation ±17' to ±18' above the former tennis court elevation on northern end of roadway section and compacted with single smooth drum 10-ton roller. Nuclear density testing, visual observation and probing of the subgrade verified the lift was placed in accordance with design specifications.
- Casino Footings C314 to C316, C869, C325 to C327, C858, C336 to C338, C847, C346 to C349, C837, C359 to C360, C826, C370 to C371, C817 were backfilled with two 12-inch thick lifts of recycled concrete aggregate. Casino Footings C313, C324, C334 to C335 were backfilled with one 12-inch thick lift of recycled concrete aggregate. Nuclear density testing, visual observation and probing of the subgrade verified the lifts were placed in accordance with design specifications. Pending a proof-roll of 2<sup>nd</sup> lift the footing backfill in this area is approved.

e 1 of 1	Inspector: Robert Fioretti
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CONTRACTOR, LABOR & EQUIPMENT	
QUANTITY	DESCRIPTION
1	Crew for Excavation-Tano
1	Crew for Footings-Skyline Concrete
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date: 9/23/2008	<b>JOB NO.</b> 5485	
PROJECT: Concord		
LOCATION: Lake Kiame	esha, NY	
contractor: Fuller		
owner: Cappelli		
weather: Sunny	темр: High 60's	
PRESI	ENT AT SITE	
Oscar Nordstrom/Bria	an Tetor-Fuller	
George Rivera-Skylin	ne Concrete	
Eric Beck-Tano		

CHARGABLE EQUI	PMENT USED
Nuclear Density Gauge	

- Excavated 2 to 3 feet below existing finished floor subgrade to a previously approved firm natural footing subgrade for Casino Footings C371/C817, C360/C826, C349/C837, C338/C847, C327/C858 and compacted with double-drum sheepsfoot roller. The footings were changed and need to be expanded. Visual observation and probing of the expansion area of footing subgrades verified a footing subgrade of 4 TSF bearing capacity.
- Excavated material from node lines 17 to 18 from approximate node line L to N was taken to the roadway north of Sediment Basin #2 and one 12-inch thick loose lift were placed from elevation ±20' to ±22' above the former tennis court elevation on southern end of roadway section and compacted with single smooth drum 10-ton roller. This elevation is subgrade for this roadway section. Visual observation and probing of the subgrade verified the lift was placed in accordance with design specifications. Deo Persaud and Rob Fioretti performed inspections.
- Placement of coarse R.C.A. from crushed demolition concrete stockpile continued in previously proofrolled area for slab-on-grade from Casino Footings C788/C207 to C217/C941 to C371/C817 to C711/C361. Placed 7-inches of R.C.A. and compacting with single smooth drum 10-ton roller.
- Casino Footings C818 to C819, C827 to C829, C838 to C841, C848 to C851, C859 to C860, C870 to C871 were backfilled with one 12-inch thick lifts of natural on-site material to top of footings. Visual observation and probing of the subgrade verified the lifts were placed in accordance with design specifications. Pending a proof-roll of 2<sup>nd</sup> lift the footing backfill in this area is approved.

Page 1 of 1	Inspector: Robert Fioretti
	Signature



CONTRACTOR, LABOR & EQUIPMENT		
QUANTITY	DESCRIPTION	
1	Crew for Excavation-Tano	
1	Crew for Footings-Skyline Concrete	

DATE: 9/18/2008	<b>ЈОВ NO. 5485</b>
PROJECT: Concord	
LOCATION: Lake Kiam	esha, NY
contractor: Fuller	
owner: Cappelli	· · · · · · · · · · · · · · · · · · ·
weather: Sunny	темр: High 60's
PRES	SENT AT SITE
Oscar Nordstrom/Bri	an Tetor-Fuller
George Rivera-Skylin	ne Concrete
Eric Beck-Tano	- <del> </del>

CHARGABLE EQUIPMENT USED	
Nuclear Density Gauge	
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- Excavated 2 to 3 feet below existing finished floor subgrade to a firm natural footing subgrade for Casino Footings C818 to C819, C827 to C829, C838 to C841, C848 to C851 and compacted with double-drum sheepsfoot roller. Footing C818 was over-excavated 2 feet below subgrade and C819 was over-excavated 3 feet below subgrade due to uncontrolled fills, and C828 was over-excavated 6 inches due to slight pumping. All over-excavated footings were backfilled with compacted lifts of recycled concrete aggregate (R.C.A.). Visual observation and probing of the footing subgrades verified a footing subgrade of 4 TSF bearing capacity. Deo Persaud and Rob Fioretti performed inspections.
- Excavated material from footing excavations, from node line 18 from approximate node line K to L and from node line 17 from approximate node line L to N was taken to the roadway north of Sediment Basin #2 and one 12-inch thick loose lift were placed from elevation ±19' above the former tennis court elevation on northern end of roadway section and compacted with single smooth drum 10-ton roller. This elevation is subgrade for the roadway. The remainder of material was placed on the slope of the roadway in 12-inch thick loose lifts and compacted with single smooth drum 10-ton roller. Nuclear density testing, visual observation and probing of the subgrade verified the lift was placed in accordance with design specifications. Deo Persaud and Rob Fioretti performed inspections.
- Placement of coarse R.C.A. from crushed demolition concrete stockpile continued in previously proof-rolled area for slab-on-grade from Casino Footings C788, C207 to C761, C284 to C217, C941 to C294, C891. Placed 7-inches of R.C.A. and compacting with single smooth drum 10-ton roller.

Page 1 of 1	Inspector: Robert Fioretti
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CONTRACTOR, LABOR & EQUIPMENT	
QUANTITY	DESCRIPTION
1	Crew for Excavation-Tano
1	Crew for Footings-Skyline Concrete

DATE: 9/29/2008	<b>JOB NO. 5485</b>	
PROJECT: Concord		
LOCATION: Lake Kiames	ha, NY	
CONTRACTOR: Fuller		
owner: Cappelli		
WEATHER: P. Cloudy	TEMP: Low 70's	
PRESEN	IT AT SITE	
Oscar Nordstrom/Brian	Tetor-Fuller	
George Rivera-Skyline	Concrete	
Eric Beck-Tano	<u> </u>	
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CHARGABLE EQUIPMENT USED	
Nuclear Density Gauge	
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- Excavated 2 to 3 feet below existing finished floor subgrade for Casino Footings C883 to C886. However, during excavation the depth of footings changed and excavation will continue tomorrow.
- Excavated material from footing excavation and from node lines 16 to 18 from approximate node line L to N was taken to the tennis court area east of Sediment Basin #2. All asphalt in tennis court was broken to allow for drainage and left in place. A 8-inch thick loose lift and a 12-inch thick loose lift was placed above the tennis court from elevation 0' to  $\pm 1$ ' and compacted with single smooth drum 10-ton roller. Nuclear density testing, visual observation and probing of the subgrade verified the lift was placed in accordance with design specifications.
- Casino Footings C862, C863, C874 and C875 were backfilled with one 12-inch thick lifts of recycled concrete aggregate. Visual observation and probing of the subgrade verified the lifts were placed in accordance with design specifications.
- Placement of coarse R.C.A. from crushed demolition concrete stockpile was completed in previously proof-rolled area for slab-on-grade from Casino Footings C788/C207 to C217/C941 to C371/C817 to C711/C361. Placed 7-inches of R.C.A. and compacting with single smooth drum 10-ton roller.

1 of 1	Inspector: Robert Fioretti
	Signature



	CONTRACTOR, LABOR & EQUIPMENT	
QUANTITY	DESCRIPTION	
1	Crew for Excavation-Tano	
1	Crew for Footings-Skyline Concrete	
	-	

DATE: 9/22/2008	<b>ЈОВ NO. 5485</b>
PROJECT: Concord	
LOCATION: Lake Kiame	sha, NY
CONTRACTOR: Fuller	<u></u>
owner: Cappelli	<del>"</del>
WEATHER: Sunny	TEMP: High 60's
PRESE	NT AT SITE
Oscar Nordstrom/Bria	n Tetor-Fuller
George Rivera-Skylin	e Concrete
Eric Beck-Tano	

CHARGABLE EQUIPMENT USED	
Nuclear Density Gauge	

- Excavated 2 to 3 feet below existing finished floor subgrade to a firm natural footing subgrade for Casino Footings C881 and C892 and compacted with double-drum sheepsfoot roller. Visual observation and probing of the footing subgrades verified a footing subgrade of 4 TSF bearing capacity.
- Excavated material from footing excavations, and from node lines 17 to 18 from approximate node line L to N was taken to the tennis court area northeast of Sediment Basin #2. All asphalt in tennis court was broken to allow for drainage and left in place. A 8-inch thick loose lift was placed above the tennis court elevation and compacted with single smooth drum 10-ton roller. Visual observation and probing of the subgrade verified the lift was placed in accordance with design specifications. Deo Persaud and Rob Fioretti performed inspections.
- Placement of coarse R.C.A. from crushed demolition concrete stockpile continued in previously proofrolled area for slab-on-grade from Casino Footings C788, C207 to C761, C284 to C217, C941 to C294, C891. Placed 7-inches of R.C.A. and compacting with single smooth drum 10-ton roller
- Proof-rolling performed from Casino Footings C756, C295 to C305, C880 to C711, C361 to C371, C817.
   Area was rolled with a single smooth drum 10-ton roller. The area is approved for slab-on-grade preparation.
- Casino Footings C818 to C819, C827 to C829, C838 to C841, C848 to C851, C859 to C860, C870 to C871 were backfilled with one 12-inch thick lifts of on-site material. Nuclear density testing, visual observation and probing of the subgrade verified the lifts were placed in accordance with design specifications.

Page 1 of 1	Inspector: Robert Fioretti	
	Signature	



CONTRACTOR, LABOR & EQUIPMENT	
QUANTITY	DESCRIPTION
1	Crew for Excavation-Tano
1	Crew for Footings-Skyline Concrete

DATE: 9/17/2008	<b>ЈОВ NO. 5485</b>	
PROJECT: Concord		
LOCATION: Lake Kiamesl	ıa, NY	
CONTRACTOR: Fuller		
owner: Capelli	**	
weather: Sunny	TEMP: High 60's	
PRESEN	T AT SITE	
Oscar Nordstrom/Brian	Tetor-Fuller	
George Rivera-Skyline Concrete		
Eric Beck-Tano		

CHARGABLE EQUIPMENT US	ED
Nuclear Density Gauge	
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#### **GEOTECH INSPECTION REPORT**

- Excavated material from node line 18 from approximate node line K to L and from node line 17 from approximate node line L to N was taken to the roadway north of Sediment Basin #2 and two 12-inch thick loose lift were placed from elevation ±16' to ±17' above the former tennis court elevation on southern end of roadway section and compacted with single smooth drum 10-ton roller. Nuclear density testing, visual observation and probing of the subgrade verified the lift was placed in accordance with design specifications. Deo Persaud and Rob Fioretti performed inspections.
- Casino Footings C342 to C344, C717, C350 to C355, C711, C361 to C362 were backfilled with two 12-inch thick lifts of recycled concrete aggregate. Nuclear density testing, visual observation and probing of the subgrade verified the lifts were placed in accordance with design specifications. Pending a proof-roll of 2<sup>nd</sup> lift the footing backfill in this area is approved. Deo Persaud and Rob Fioretti performed inspections.
- Proof-rolling performed from Casino Footings C788, C207 to C761, C284 to C217, C941 to C294, C891. Area was rolled with a single smooth drum 10-ton roller, and soft areas were removed and backfilled with recycled concrete aggregate. The area is approved for slab-on-grade preparation. Began placing 7-inches of coarse recycled concrete aggregate from crushed demolition concrete stockpile and compacting with single smooth drum 10-ton roller.

Page 1 of 1 Inspector: Robert Fioretti



CONTRACTOR, LABOR & EQUIPMENT	
QUANTITY	DESCRIPTION
1	Excavating/Backfilling Crew - Tano
<u> </u>	

date: 10/9/2008	<b>ЈОВ NO. 5485</b>
рројест: Concord	
LOCATION: Lake Kiamesh	ıa, NY
CONTRACTOR: Fuller, JM	Associates, Tano
owner: Cappelli	
WEATHER: P. Cloudy	TEMP: Low 70's
PRESENT	Γ AT SITE
Al Destefano/Oscar Nor	dstrom-Fuller
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CHARGABLE EQ	QUIPMENT USED

CHARGABLE EQUIPMENT USER	)
Nuclear Density Gauge	<u>"</u>
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## **GEOTECH FIELD REPORT**

• UST-11 excavation, approximately 9 feet below existing grade, was backfilled with three feet of chipped stone bedrock and six 12-inch thick loose lifts of natural on-site material to the existing grade. The stone bedrock was placed in three lifts and compacted with the excavator bucket, and the natural on-site material was compacted with a double-drum sheepsfoot roller. Nuclear density testing, visual observation and probing of the subgrade verified the lift was placed in accordance with design specifications.

Page 1 of 1

Inspector: Robert Fioretti



	CONTRACTOR, LABOR & EQUIPMENT
QUANTITY	DESCRIPTION
2	Crew for Excavation-Tano
	-
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DATE: 10/15/2008	<b>JOB NO.</b> 5485
PROJECT: Concord	
LOCATION: Lake Kiames	ha, NY
contractor: Fuller	
owner: Cappelli	
WEATHER: P. Cloudy	темр: Mid 60's
PRESEN	T AT SITE
Oscar Nordstrom-Fuller	<u> </u>
Eric Beck-Tano	
·	

CHARGABLE EQUIPMENT USED	
Nuclear Density Gauge	,

## **GEOTECH INSPECTION REPORT**

- Placed two 12-inch thick loose lift of natural on-site material in existing excavation for hotel and
  compacted with a single smooth drum 10-ton roller. Due to design changes the existing hotel excavation
  is being filled, because the designed location of the hotel has moved. Nuclear density testing, visual
  observation and probing of the subgrade verified the lifts were placed in accordance with design
  specifications.
- Due to design changes of the slab-on-grade elevation of the Casino Area the previously approved slab-on-grade subgrade will be raised. Placed one 12-inch thick loose lift of R.C.A. from Casino Footings C771/C361 to C363 to C357 to C302 to C756/C259 and compacted with a single smooth drum 10-ton roller. Pending a proof-roll in this area the slab subgrade will be approved visually.

Page 1 of 1

Inspector: Robert Fioretti



	CONTRACTOR, LABOR & EQUIPMENT
QUANTITY	DESCRIPTION
1	Excavating/Backfilling Crew - Tano
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•	

date: 10/10/2008	<b>јов no.</b> 5485	
PROJECT: Concord		
LOCATION: Lake Kiames	ha, NY	
CONTRACTOR: Fuller		
owner: Cappelli		_
weather: Sunny	TEMP: Mid 60's	
PRESEN	T AT SITE	
Al Destefano/Oscar No	rdstrom-Fuller	
CHADCABLE D	OTTORAGENIE TIEGEN	

CHARGABLE EQUIPMENT	USED
Nuclear Density Gauge	<del></del>
	<del></del>

## **GEOTECH FIELD REPORT**

• UST-16 excavation, approximately 5 feet below existing grade, was backfilled with five 12-inch thick loose lifts of natural on-site material to the existing grade and compacted with a double-drum sheepsfoot roller. Nuclear density testing, visual observation and probing of the subgrade verified the lift was placed in accordance with design specifications.

Page 1 of 1

Inspector: Robert Fioretti



·-	CONTRACTOR, LABOR & EQUIPMENT	
QUANTITY	DESCRIPTION	
1	Excavating/Backfilling Crew - Tano	
•	A-1-1-1-1-1-1-1-1-1-1-1-1-1-1-1-1-1-1-1	
-		

DATE: 10/8/2008	<b>JOB NO. 5485</b>
PROJECT: Concord	
LOCATION: Lake Kiamesl	na, NY
CONTRACTOR: Fuller, JM	Associates, Tano
owner: Cappelli	
WEATHER: P. Cloudy	TEMP: Low 60's
PRESEN	Γ AT SITE
Al Destefano/Oscar Nor	rdstrom-Fuller
Erick Beck-Tano	<del>-</del>

CHARGABLE EQUIPMENT	USED
Nuclear Density Gauge	

## **GEOTECH FIELD REPORT**

- Remainder of node excavation M-18, approximately 3 feet below existing grade and 30 feet diameter, was backfilled with three 12-inch thick loose lifts of natural on-site material to the existing grade and compacted with a double-drum sheepsfoot roller. Nuclear density testing, visual observation and probing of the subgrade verified the lift was placed in accordance with design specifications.
- Node excavation O-18, approximately 5 feet below existing grade and 30 feet diameter, was excavated
  one additional foot to remove all soft material to a firm natural subgrade, and was backfilled with six 12inch thick loose lifts of natural on-site material to the existing grade and compacted with a double-drum
  sheepsfoot roller. Nuclear density testing, visual observation and probing of the subgrade verified the lift
  was placed in accordance with design specifications.

Page 1 of 1

Inspector: Robert Fioretti



CONTRACTOR, LABOR & EQUIPMENT	
QUANTITY	DESCRIPTION
1	Crew for Excavation-Tano
1	Crew for Footings-Skyline Concrete
<u>-</u>	
_	3.00

DATE: 10/2/2008	<b>JOB NO.</b> 5485			
PROJECT: Concord	PROJECT: Concord			
LOCATION: Lake Kiamesha, NY				
CONTRACTOR: Fuller				
owner: Cappelli				
WEATHER: P. Cloudy/Windy TEMP: High 50's				
PRESENT AT SITE				
Oscar Nordstrom/Brian Tetor-Fuller				
George Rivera-Skyline Concrete				
Eric Beck-Tano				

CHARGABLE EQUIPMENT USED		
Nuclear Density Gauge		

- Excavated 6 to 7 feet below existing finished floor subgrade to a bedrock footing subgrade for Casino Footings C884 to C886. Groundwater is present at the 6 to 7 feet below existing finished floor subgrade. Sump pumps were set up to allow for forming and pouring. Visual observation and probing of the footing subgrades verified a footing subgrade of 4 TSF bearing capacity.
- Excavated material from footing excavations and from node lines 15 to 18 from approximate node line L to N was taken to the tennis court area northeast and east of Sediment Basin #2 and one 12-inch thick loose lift were placed at elevation ±7' above the former tennis court and compacted with single smooth drum 10-ton roller. Nuclear density testing, visual observation and probing of the subgrade verified the lift was placed in accordance with design specifications.
- Casino Footings C872 was backfilled with two 12-inch thick lifts of recycled concrete aggregate and compacted with a double-drum sheepsfoot roller. Nuclear density testing, visual observation and probing of the subgrade verified the lifts were placed in accordance with design specifications.
- Placement of 1 to 2-inches of fine R.C.A. from crushed demolition concrete stockpile was performed on low areas of previously graded slab subgrade and compacted with a single smooth drum 10-ton roller.
- Under slab drainage pipe trenches were excavated 2 to 5 feet below existing slab subgrade in general areas between Casino Footings C219 to C221 to C265 to C263 and C329 to C331 to C353 to C351.

Page 1 of 2	Inspector: Robert Fioretti	
	Signature	

		Page 2 of 2	Inspector: Robert Fioretti Signature
	footings were changed and need to be footing subgrades verified a footing s	e expanded. Visual obser	e, C779/C240, C782/C229, C786/C218. The vation and probing of the expansion area of g capacity.
•	subgrade for footing expansion on no	rth side of Casino Footin	o a previously approved firm natural footing gs C724/C339, C732/C328, C741/C317,



CONTRACTOR, LABOR & EQUIPMENT		
QUANTITY	DESCRIPTION	
1	Crew for Excavation-Tano	
1	Crew for Footings-Skyline Concrete	

date: 10/6/2008	<b>JOB NO.</b> 5485	
PROJECT: Concord		
LOCATION: Lake Kiamesh	ıa, NY	
CONTRACTOR: Fuller		
owner: Cappelli		
WEATHER: P. Cloudy/Windy TEMP: Mid 50's		
PRESENT AT SITE		
Oscar Nordstrom-Fuller		
George Rivera-Skyline Concrete		
Eric Beck-Tano		

CHARGABLE EQUIPMENT USE	D
Nuclear Density Gauge	<del>-</del>
	N.

### **GEOTECH INSPECTION REPORT**

- Excavated 4 to 5 feet below existing finished floor subgrade to a bedrock footing subgrade for Casino Footings C901, C910, C920, C928, and C934. Visual observation and probing of the footing subgrades verified a footing subgrade of 4 TSF bearing capacity. However, before footings were poured work was stopped on site due to major changes to design.
- Stockpiled material in upper east tennis court area from 9/26/08 (from node lines 17 to 18 from approximate node line L to N) was taken to the tennis court area northeast and east of Sediment Basin #2 and one 12-inch thick lift was placed and compacted with single smooth drum 10-ton roller. Additional material from stockpile was taken to the Hotel excavation and two 12-inch thick lifts were placed on the west side of the excavation and compacted with single smooth drum 10-ton roller. Nuclear density testing, visual observation and probing of the subgrade verified the lift was placed in accordance with design specifications.
- Under slab drainage pipe trenches were excavated 2 to 5 feet below existing slab subgrade in general areas between Casino Footings C302 to C304 to C359 to C357.

Page 1 of 1

Inspector: Robert Fioretti



CONTRACTOR, LABOR & EQUIPMENT		
QUANTITY	DESCRIPTION	
2	Crew for Excavation-Tano	

date: 10/14/2008	<b>ЈОВ NO. 5485</b>	
PROJECT: Concord		
LOCATION: Lake Kiames	ha, NY	
contractor: Fuller	· · · ·	
owner: Cappelli		
weather: P. Cloudy	TEMP: Low 70's	
PRESENT AT SITE		
Oscar Nordstrom-Fulle	r	
Eric Beck-Tano	<del>~ ~ ~</del>	
7		

CHARGABLE EQUIPMENT USED	$\neg$
Nuclear Density Gauge	

#### GEOTECH INSPECTION REPORT

- Placed one 12-inch thick loose lift of natural on-site material in existing excavation for old hotel location and compacted with a single smooth drum 10-ton roller. Due to design changes the existing hotel excavation is being filled with natural on-site material, because the designed location of the hotel has moved. Nuclear density testing, visual observation and probing of the subgrade verified the lift was placed in accordance with design specifications.
- An excavation created during footing demolition from node lines J to K and 17 to 18 approximately 8 feet below existing grade was pumped of water. Two 12-inch thick loose lifts of chipped stone were placed and compacted with the excavator bucket, and six 12-inch thick loose lifts of natural on-site material were placed and compacted with a single smooth drum 10-ton roller. Nuclear density testing, visual observation and probing of the subgrade verified the lifts were placed in accordance with design specifications.

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Inspector: Robert Fioretti



CONTRACTOR, LABOR & EQUIPMENT		
QUANTITY	DESCRIPTION	
1	Excavating/Backfilling Crew - Tano	

<b>DATE:</b> 10/7/2008	<b>ЈОВ NO. 5485</b>		
PROJECT: Concord			
LOCATION: Lake Kiamesha, NY			
contractor: Fuller, JM Associates, Tano			
owner: Cappelli			
weather: Sunny	темр: Mid 60's		
PRESENT AT SITE			
Al Destefano/Oscar Nordstrom-Fuller			
Erick Beck-Tano			

CHARGABLE EQUIPMENT USED	
Nuclear Density Gauge	
<u></u>	

## **GEOTECH FIELD REPORT**

• Node excavation M-18, approximately 10 feet below existing grade and 30 feet diameter, was backfilled with seven lifts. Five 12-inch thick loose lifts of natural on-site material and two lifts of crushed sandstone bedrock that was excavated from the southern most point of the parking garage excavation (Approx. grid node lines 16 to 18 and along gridline N). The soil was compacted with a double-drum sheeps-foot roller. Nuclear density testing, visual observation and probing of the subgrade verified the lift was placed in accordance with design specifications.

Page 1 of 1

Inspector: Chris Mazur



	CONTRACTOR, LABOR & EQUIPMENT		
QUANTITY	DESCRIPTION		
1	Crew for Excavation-Tano		
1	Crew for Footings-Skyline Concrete		

DATE: 10/3/2008	<b>ЈОВ NO. 5485</b>	
PROJECT: Concord		
LOCATION: Lake Kiamesha, NY		
contractor: Fuller		
owner: Cappelli		
WEATHER: P. Cloudy/Windy TEMP: Mid 50's		
PRESENT AT SITE		
Oscar Nordstrom/Brian Tetor-Fuller		
George Rivera-Skyline Concrete		
Eric Beck-Tano		
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CHARGABLE EQUIPMENT US	ED	•
Nuclear Density Gauge		

#### GEOTECH INSPECTION REPORT

- Excavated 4 to 5 feet below existing finished floor subgrade to a bedrock footing subgrade for Casino Footings C893 to C897. Visual observation and probing of the footing subgrades verified a footing subgrade of 4 TSF bearing capacity.
- Excavated material from footing excavations and from node lines 15 to 18 from approximate node line L to N was taken to the tennis court area north of Sediment Basin #2 and one 6-inch thick lift of chipped bedrock was placed on slopes and compacted with single smooth drum 10-ton roller. Visual observation and probing of the subgrade verified the lift was placed in accordance with design specifications.
- Expansion Casino Footings C724/C339, C732/C328, C741/C317, C749/C306, C756/C259, C761/C284, C765/C273, C769/C262, C774/C251, C782/C229, C786/C218, C371/C817, C360/C826, C349/C837, C338/C847, C327/C858, C316/C869, C305/C880, C294/C891, C283/C900, C272/C909 were backfilled with two 12-inch thick lifts, one of recycled concrete aggregate and one of natural on-site material, and compacted with a double-drum sheepsfoot roller. Nuclear density testing, visual observation and probing of the subgrade verified the lifts were placed in accordance with design specifications. It was discovered that C774/C251 was expanded instead of C779/C240. Report dated 10/2/08 shows C779/C240 approval and it should be noted that the expansion really took place on C774/C251. Site supervisors were notified of mistake.
- Under slab drainage pipe trenches were excavated 2 to 5 feet below existing slab subgrade in general areas between Casino Footings C219 to C221 to C265 to C263 and C329 to C331 to C353 to C351.

Page 1 of 2	Inspector: Robert Fioretti

•	Stockpile of on-site natural material (from previous excavations at node line 16 from K to L) located to the southeast of Sediment Basin #1 was used to place two 12-inch thick loose lifts to the southeast of Sediment Basin #1. The area was proofrolled before the lifts were placed and upon placement compacted with a single smooth drum 10-ton roller. Nuclear density testing, visual observation and probing of the subgrade verified the lifts were placed in accordance with design specifications.  Excavated five test pits approximately 25 feet below existing grade in racetrack area. Test pits excavated to verify depth to rock.
	Porce 2 of 2
	Page 2 of 2 Inspector: Robert Fioretti Signature



CONTRACTOR, LABOR & EQUIPMENT		
QUANTITY	DESCRIPTION	
1	Crew for Excavation-Tano	
1	Crew for Footings-Skyline Concrete	

јов по. 5485		
LOCATION: Lake Kiamesha, NY		
CONTRACTOR: Fuller		
owner: Cappelli		
WEATHER: P. Cloudy TEMP: Mid 60's		
NT AT SITE		
n Tetor-Fuller		
George Rivera-Skyline Concrete		

CHARGABLE EQUIPMENT USED	)	
Nuclear Density Gauge		•
		_
		_

## **GEOTECH INSPECTION REPORT**

- Previously excavated 6 to 7 feet below existing finished floor subgrade to a bedrock footing subgrade for Casino Footings C872. Groundwater is present at the 4 to 5 feet below existing finished floor subgrade. A sump pump was set up to allow for forming and pouring. Visual observation and probing of the footing subgrades verified a footing subgrade of 4 TSF bearing capacity.
- Excavated material from footing excavations and from node lines 16 to 18 from approximate node line L to N was taken to the tennis court area northeast and east of Sediment Basin #2 and two 12-inch thick loose lift were placed from elevation ±4' to ±6' above the former tennis court and compacted with single smooth drum 10-ton roller. Nuclear density testing, visual observation and probing of the subgrade verified the lift was placed in accordance with design specifications.
- Casino Footings C871, C882 and C883 were backfilled with two 12-inch thick lifts of recycled concrete aggregate and compacted with a double-drum sheepsfoot roller. Nuclear density testing, visual observation and probing of the subgrade verified the lifts were placed in accordance with design specifications. Pending a proof-roll of 2<sup>nd</sup> lift the footing backfill in this area is approved.
- Placement of 1 to 2-inches of fine R.C.A. from crushed demolition concrete stockpile was performed on low areas of previously graded slab subgrade and compacted with a single smooth drum 10-ton roller.

of 2	Inspector: Robert Fioretti	
	Signature	

Page 1 of 2

•	subgrade for footing expansion on nor	th side of Casino Footing ings were changed and r	a previously approved firm natural footing gs C316/C869, C305/C880, C294/C891, need to be expanded. Visual observation d a footing subgrade of 4 TSF bearing
		Page 2 of 2	Inspector: Robert Fioretti



CONTRACTOR, LABOR & EQUIPMENT		
QUANTITY	DESCRIPTION	
2	Crew for Excavation-Tano	

date: 10/23/2008	<b>ЈОВ NO. 5485</b>
PROJECT: Concord	
LOCATION: Lake Kiam	iesha, NY
CONTRACTOR: Fuller	
owner: Cappelli	
WEATHER: Sunny	TEMP: Low 50's
PRES	SENT AT SITE
Oscar Nordstrom-Fu	ller
Eric Beck-Tano	
<u> </u>	

CHARGABLE EQUIPMENT USED	
Nuclear Density Gauge	.=

- Placed five 12-thick loose lifts of fine grained R.C.A. from approximate elevation ±1481' to ±1485' on northern 1/3 of hotel footprint (including 25' east of footprint) and two 12-thick lifts from approximate elevation ±1481' to ±1482' on the remaining sourthern 2/3 of hotel footprint (including 25' east of footrpint) and compacted with a single smooth drum 10-ton roller. Nuclear density testing, visual observation and probing of the subgrade verified the lifts were placed in accordance with design specifications.
- One 12-thick loose lift of 6-minus R.C.A. to approximate elevation ±1478' on northern 1/3, 25 feet west of hotel footprint placed 10/22/08 was compacted with a single smooth drum 10-ton roller. Placed five 12-thick loose lifts of fine grained and 4-inch minus R.C.A. from approximate elevation ±1479' to ±1483' on northern 1/3, 25 feet west of hotel footprint and compacted with a single smooth drum 10-ton roller. Placed two 12-thick loose lifts of fine grained and 4-inch minus R.C.A. from approximate elevation ±1478' to ±1479' on southern 2/3, 25 feet west of hotel footprint and compacted with a single smooth drum 10-ton roller. Nuclear density testing, visual observation and probing of the subgrade verified the lifts were placed in accordance with design specifications.
- One 12-thick loose lift of on-site natural material, from previously compacted material north of Sediment Basin #2, from 25 to 50 feet east and west of hotel footprint and compacted with a single smooth drum 10-ton roller on 10/22/08. Placed three 12-thick loose lifts of on-site natural material, from excavation southwestern most tennis court south of Sediment Basin #2, from 25 to 50 feet east and west of hotel footprint and compacted with a single smooth drum 10-ton roller. The lifts are sloped away from the building footprint at an approximate 1:2 ratio. Nuclear density testing, visual observation and probing of the subgrade verified the lifts were placed in accordance with design specifications.

Page 1 of 1	Inspector: Robert Fioretti
	Signature



CONTRACTOR, LABOR & EQUIPMENT	
QUANTITY	DESCRIPTION
2	Crew for Excavation-Tano
	-
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date: 10/24/2008	<b>ЈОВ NO. 5485</b>
PROJECT: Concord	
LOCATION: Lake Kiames	ha, NY
CONTRACTOR: Fuller	
owner: Cappelli	
WEATHER: P. Cloudy	темр: High 50's
PRESEN	NT AT SITE
Oscar Nordstrom-Fulle	r
Eric Beck-Tano	
	<u></u>
CII I DO I DI DI	

CHARGABLE EQUIPMENT USED	
Nuclear Density Gauge	

#### **GEOTECH INSPECTION REPORT**

- Placed two 12-thick loose lifts of fine grained R.C.A. from approximate elevation ±1486' to ±1487' on northern 1/3 of hotel footprint (including 25' east of footprint) and compacted with a single smooth drum 10-ton roller. Nuclear density testing, visual observation and probing of the subgrade verified the lifts were placed in accordance with design specifications.
- Placed four 12-thick loose lifts of fine grained and 4-inch minus R.C.A. from approximate elevation ±1484' to ±1487' on northern 1/3, 25 feet west of hotel footprint and compacted with a single smooth drum 10-ton roller. Placed one 12-thick loose lift of fine grained and 4-inch minus R.C.A. to approximate elevation ±1480' on southern 2/3, 25 feet west of hotel footprint and compacted with a single smooth drum 10-ton roller. Nuclear density testing, visual observation and probing of the subgrade verified the lifts were placed in accordance with design specifications.
- Placed three 12-thick loose lifts of on-site natural material, from excavation southwestern most tennis court south of Sediment Basin #2, from 25 to 50 feet east and west of hotel footprint and compacted with a single smooth drum 10-ton roller. The lifts are sloped away from the building footprint at an approximate 1:2 ratio. Nuclear density testing, visual observation and probing of the subgrade verified the lifts were placed in accordance with design specifications.

Inspector: Robert Fioretti
Signature

Page 1 of 1



CONTRACTOR, LABOR & EQUIPMENT	
QUANTITY	DESCRIPTION
1	Crew for Backfill-Tano
	-

DATE: 11/13/2008	<b>ЈОВ NO. 5485</b>
PROJECT: Concord	
Location: Lake Kiamesh	ıa, NY
CONTRACTOR: Fuller	
owner: Cappelli	<u> </u>
weather: Rain	TEMP: Mid 40's
PRESENT AT SITE	
Oscar Nordstrom-Fuller	
Eric Beck-Tano	
7-10-1	
CHARGABLE EC	QUIPMENT USED

## **GEOTECH INSPECTION REPORT**

- Earthwork had been previously stopped after 10/27/08 because SWPPP program was being revised and installed.
- Due to design changes of the slab-on-grade elevation of the Casino Area previously approved slab-on-grade subgrade will be raised. Placed one 12-inch thick loose lift of crushed bedrock, from parking garage #1 excavation, from Casino Footings 253 to C231 to C235 to C213 to C210 and compacted with a single smooth drum 10-ton roller. Work was performed in accordance with our recommendations, and upon a proof-roll in this area the slab subgrade will be given final approved.

Page 1 of 1

Inspector: Robert Fioretti



CONTRACTOR, LABOR & EQUIPMENT	
QUANTITY	DESCRIPTION
2	Crew for Excavation-Tano
1	

DATE: 10/22/2008	JOB NO. 5485	
PROJECT: Concord		
LOCATION: Lake Kiamesl	na, NY	
CONTRACTOR: Fuller	<u></u>	
owner: Cappelli		
WEATHER: P. Cloudy/Windy	TEMP: Low 40's	
PRESENT AT SITE		
Oscar Nordstrom-Fuller		
Eric Beck-Tano	*	
-	<u></u>	
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CHARGABLE EQUIP	MENT USED
Nuclear Density Gauge	
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## **GEOTECH INSPECTION REPORT**

- Placed three 12-thick loose lifts of fine grained R.C.A. from approximate elevation ±1478' to ±1480' over entire hotel footprint (including 25' east of footprint) and compacted with a single smooth drum 10-ton roller. Nuclear density testing, visual observation and probing of the subgrade verified the lifts were placed in accordance with design specifications.
- Placed one 12-thick loose lift of 6-minus R.C.A., 25 feet west of hotel footprint, however the lift was not compacted. Placed one 12-thick loose lift of on-site natural material, from previously compacted material north of Sediment Basin #2, from 25 to 50 feet east and west of hotel footprint and compacted with a single smooth drum 10-ton roller. Tests will be performed tomorrow.

Page 1 of 1

Inspector: Chris Mazur



<u>,                                     </u>	CONTRACTOR, LABOR & EQUIPMENT	
QUANTITY	DESCRIPTION	
1	Crew for Backfill-Tano	
	7	
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<u> </u>		

DATE: 11/17/2008	<b>ЈОВ NO. 5485</b>
PROJECT: Concord	
LOCATION: Lake Kiam	esha, NY
CONTRACTOR: Fuller	
owner: Cappelli	
weather: Sunny	темр: High 30's
PRES	SENT AT SITE
Oscar Nordstrom-Ful	ller
Eric Beck-Tano	
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CHARGABLI	E EQUIPMENT USED
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#### **GEOTECH INSPECTION REPORT**

- No Geotechnical Work performed on 11/14/08.
- Due to design changes of the slab-on-grade elevation of the Casino Area the previously approved slab-ongrade subgrade will be raised. Placed one 12-inch thick loose lift of crushed bedrock, from parking garage #1 excavation, from Casino Footings 2774/C251 to C252 to C219 to C220 to C209 to C786/C207 and compacted with a single smooth drum 10-ton roller. Work was performed in accordance with our recommendations, and upon a proof-roll in this area the slab subgrade will be given final approved.

Inspector: Robert Fioretti



CONTRACTOR, LABOR & EQUIPMENT		
QUANTITY	DESCRIPTION	
2	Crew for Excavation-Tano	
	<u>-</u>	

DATE: 10/17/2008	<b>JOB NO. 5485</b>
PROJECT: Concord	
LOCATION: Lake Kiame	esha, NY
CONTRACTOR: Fuller	
owner: Cappelli	***
weather: Sunny	темр: High 60's
PRESI	ENT AT SITE
Oscar Nordstrom-Full	ler
Eric Beck-Tano	
744.1	

CHARGABLE EQUIPA	MENT USED
Nuclear Density Gauge	
	<u></u>

## GEOTECH INSPECTION REPORT

- Due to design changes of the slab-on-grade elevation of the Casino Area the previously approved slab-on-grade subgrade will be raised. Placed one 12-inch thick loose lift of natural on-site material (from soil previously placed in compacted lifts north of Sediment Basin #2) from Casino Footings C357 to C358 to C327/C858 to C261/C919 to C257 and compacted with a double drum sheepsfoot roller. Upon end of the day, the area was compacted with a single smooth drum 10-ton roller. Pending a proof-roll in this area the slab subgrade will be approved.
- Due to design changes the hotel location has been moved. Excavated approximately 15 feet in the northeast corner of the building footprint and proceeded south, because the contractor initially thought they needed to go to bedrock. However, due to further revision hotel will be placed on compacted lifts of R.C.A. material. A 2 foot by 2 foot by 2 foot excavation at the bottom of the excavation was excavated to locate bedrock. This excavation was backfilled with two 12-inch thick loose lifts of R.C.A. material and compacted with the excavator bucket. Nuclear density testing, visual observation and probing of the subgrade verified the lifts were placed in accordance with design specifications.

Page 1 of 1

Inspector: Chris Mazur



CONTRACTOR, LABOR & EQUIPMENT		
QUANTITY	DESCRIPTION	
2	Crew for Excavation-Tano	
•	•	

DATE: 10/21/2008	<b>јов no.</b> 5485
PROJECT: Concord	<u> </u>
LOCATION: Lake Kiamesh	ıa, NY
CONTRACTOR: Fuller	
owner: Cappelli	
WEATHER: P. Cloudy/Showers	TEMP: Mid 40's
PRESENT	F AT SITE
Oscar Nordstrom-Fuller	
Eric Beck-Tano	<del> </del>
***************************************	
CHARGABLE EC	QUIPMENT USED
Nuclear Density Gauge	<del>-</del>

#### **GEOTECH INSPECTION REPORT**

• Surveyed location of UST-8 that was not excavated and compacted previously on 10/20/08 was excavated 5 feet below existing grade to a firm natural material and proofrolled with a single drum 10-ton roller. Placed five 12-thick loose lifts of 6-inch minus R.C.A. material and compacted with a single smooth drum 10-ton roller. Contractor then placed proofrolled firm natural material in south end of hotel footprint and placed three 12-inch thick loose lifts of 6-inch minus R.C.A material and compacted with a single smooth drum 10-ton roller. Finally, placed one 12-inch thick loose lift of 6-inch minus R.C.A. material over entire hotel footprint to approximate elevation ±1477' and compacted with a single smooth drum 10-ton roller. Nuclear density testing, visual observation and probing of the subgrade verified the lift was placed in accordance with design specifications.

Inspector: Robert Fioretti	
Signature	



CONTRACTOR, LABOR & EQUIPMENT	
QUANTITY	DESCRIPTION
1	Crew for Backfill-Tano

DATE: 11/13/2008	<b>JOB NO.</b> 5485
PROJECT: Concord	
LOCATION: Lake Kiamesh	ia, NY
CONTRACTOR: Fuller	
owner: Cappelli	<u>.                                      </u>
weather: Rain	темр: Mid 40's
PRESENT	T AT SITE
Oscar Nordstrom-Fuller	
Eric Beck-Tano	
	<del></del>
	neral.
CHARGABLE EQ	QUIPMENT USED
	_ <del></del>

#### **GEOTECH INSPECTION REPORT**

- Placed one 12-thick loose lift of fine grained R.C.A. from approximate elevation ±1482' to ±1483' on southern 2/3 of hotel footprint (including 25' east of footprint) and compacted with a single smooth drum 10-ton roller. Placed one 12-thick loose lift of fine grained R.C.A. from approximate elevation ±1480' to 1481' on southern 2/3 of hotel footprint, 25 feet west of hotel footprint and compacted with a single smooth drum 10-ton roller. Nuclear density testing, visual observation and probing of the subgrade verified the lifts were placed in accordance with design specifications.
- Began demolition of the upper-most tennis courts. The courts are located on top of a 4-in. to 6-in. thick concrete pad. The first ±100-ft. (from south to north) of concrete contains rebar, post-tension rods and electrical conduit piping running throughout. The remaining concrete appears to just have a wire mat that is located in the bottom 1-in. to 2-in. of the pad.

Page 1 of 1

Inspector: Robert Fioretti



CONTRACTOR, LABOR & EQUIPMENT		
QUANTITY	DESCRIPTION	
2	Crew for Excavation-Tano	

date: 10/16/2008	<b>јов no.</b> 5485	
PROJECT: Concord		
Location: Lake Kiamesh	ia, NY	
contractor: Fuller	<del>"</del>	
owner: Cappelli		
WEATHER: P. Cloudy	темр: Mid 60's	_
PRESENT	Γ AT SITE	
Oscar Nordstrom-Fuller		_
Eric Beck-Tano	****	_
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GW - D G - D V D TO		ー コ
CHARGABLE EQ	UIPMENT USED	_
	<del></del>	-
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# **GEOTECH INSPECTION REPORT**

- Minor grading with bulldozer occurred in area north of footing grid line CO8. Area was rolled firm with single smooth drum 10-ton roller.
- Due to design changes of the slab-on-grade elevation of the Casino Area the previously approved slab-on-grade subgrade will be raised. Placed one 12-inch thick loose lift of R.C.A. from Casino Footings C756/C259 to C302 to C269 to C268 to C246 to C243 to C769/C262 and compacted with a single smooth drum 10-ton roller. Pending a proof-roll in this area the slab subgrade will be approved.

Page 1 of 1

Inspector: Robert Fioretti



CONTRACTOR, LABOR & EQUIPMENT	
QUANTITY	DESCRIPTION
2	Crew for Excavation-Tano
<u></u>	

DATE: 10/20/2008	<b>ЈОВ NO. 5485</b>	_	
PROJECT: Concord	<u></u>		
LOCATION: Lake Kiamo	esha, NY		
contractor: Fuller	<u> </u>		
owner: Cappelli			
weather: Sunny	TEMP: Low 50's		
PRESENT AT SITE			
Oscar Nordstrom-Fuller			
Eric Beck-Tano	<u> </u>		
	<u> </u>	_	
	<u> </u>		

CHARGABLE EQUIPMENT USED	
Nuclear Density Gauge	
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# GEOTECH INSPECTION REPORT

- Due to design changes of the slab-on-grade elevation of the Casino Area the previously approved slab-on-grade subgrade will be raised. Placed one 12-inch thick loose lift of natural on-site material (from soil previously placed in compacted lifts north of Sediment Basin #2) from Casino Footings C257 to C261/C919 to C217/C941 to C213 and compacted with a single smooth drum 10-ton roller. Pending a proof-roll in this area the slab subgrade is approved.
- Due to design changes the hotel location has been moved. Previously excavated northern side of the building footprint (approx. location of UST-8) was proofrolled on firm natural material. Placed eight 12-inch thick loose lifts of 6-inch minus R.C.A material and compacted with a single smooth drum 10-ton roller. Nuclear density testing, visual observation and probing of the subgrade verified the lifts were placed in accordance with design specifications. Geod surveyed the exact location of UST-8 and five feet to the north and five feet to the west needs to be excavated and proofrolled before any compacted lifts are placed. This area requires excavation as it was filled previously with uncontrolled backfill with the thought that the building subgrade would be excavated to bedrock.

Page 1 of 1	Inspector: Robert Fioretti
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CONTRACTOR, LABOR & EQUIPMENT		
QUANTITY	DESCRIPTION	
1	Crew for Backfill-Tano	
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DATE: 11/24/2008	<b>ЈОВ NO. 5485</b>	
PROJECT: Concord		
LOCATION: Lake Kiamesha, NY		
CONTRACTOR: Fuller		
owner: Cappelli		
WEATHER: Windy	темр: High 30's	
PRESENT AT SITE		
Oscar Nordstrom-Fuller		
Eric Beck-Tano		
CHARGABLE EQUIPMENT USED		

CHARGABLE EQUIPMENT USED		
Nuclear Density Gauge		

## **GEOTECH INSPECTION REPORT**

• Ten inches of frost present in morning. Contractor removed frost and placed in frost stockpile on tennis court prior to placement of fill. Placed three 12-inch thick loose lift of 3-inch minus R.C.A. and 3-inch minus crushed bedrock (from parking garage #1 excavation) from approximate elevation ±1475' to ±1477' along entire hotel footprint width including 20 feet outside of footprint and from column line T-2.5 to T-4.5 and compacted with a single smooth drum 10-ton roller. Nuclear density testing, visual observation and probing of the subgrade verified the lifts were placed in accordance with design specifications. Deo Persaud performed inspection.

Page 1 of 1

Inspector: Robert Fioretti



CONTRACTOR, LABOR & EQUIPMENT	
QUANTITY	DESCRIPTION
1	Crew for Backfill-Tano

DATE: 11/20/2008	<b>ЈОВ NO. 5485</b>	
PROJECT: Concord	<u> </u>	
LOCATION: Lake Kiamesh	ıa, NY	
CONTRACTOR: Fuller		
owner: Cappelli		
weather: P. Cloudy	темр: High 20's	
PRESENT AT SITE		
Oscar Nordstrom-Fuller		
Eric Beck-Tano		
<del>-</del>		
CHARGABLE EQUIPMENT USED		
Marilan D. V. C.		

CHARGABLE EQUIPMENT USED  Nuclear Density Gauge	

# **GEOTECH INSPECTION REPORT**

- Two to three inches of frost present in morning. Contractor used dozer to track and break the frost and compacted with a single smooth drum 10-ton roller. Placed three 12-inch thick loose lift of 3-inch minus R.C.A. from approximate elevation ±1462' to ±1464' along entire hotel footprint width including 20 feet outside of footprint and from 20 feet south of footprint to column line T-2.5 and compacted with a single smooth drum 10-ton roller. Nuclear density testing, visual observation and probing of the subgrade verified the lifts were placed in accordance with design specifications. Deo Persaud performed inspection.
- Two to three inches of frost present in morning. Contractor used dozer to track and break the frost and compacted with a single smooth drum 10-ton roller. Placed five 12-inch thick loose lifts of natural on-site material on subgrade of Casino Footings C385 to C996 to C990 to C984 approximately 7 feet below slab-on-grade elevation and compacted with a single smooth drum 10-ton roller. Natural on-site material came from the excavation of Sediment Basin #4 and parking garage #1 excavation. This backfills the former hotel location that had been excavated and is being filled with natural on-site material because the designed location of the hotel has been moved. Nuclear density testing, visual observation and probing of the subgrade verified the lift was placed in accordance with design specifications. Deo Persaud performed inspection.

Page 1 of 1

Inspector: Robert Fioretti



CONTRACTOR, LABOR & EQUIPMENT	
QUANTITY	DESCRIPTION
1	Crew for Backfill-Tano

DATE: 11/19/2008	<b>ЈОВ NO. 5485</b>	
PROJECT: Concord		
LOCATION: Lake Kiamesha, NY		
CONTRACTOR: Fuller		
owner: Cappelli		
weather: Windy	темр: High 20's	
PRESENT AT SITE		
Oscar Nordstrom-Fuller		
Eric Beck-Tano		
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CHARGABLE EQUIPMENT USE	 D
Nuclear Density Gauge	
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## GEOTECH INSPECTION REPORT

- Veteran's Day Holiday 11/18/08. No work performed.
- A coffer dam was constructed across Sediment Basin #1 and the contractor pumped dry eastern half of Sediment Basin #1. Bottom of basin was mucked of all high moist silts and material was taken to the temporary topsoil stockpile. Placed 18 to 24-inch thick loose lift of 12-inch minus R.C.A. and heeled with excavator bucket. Another 18 to 24-inch thick loose lift of 12-inch minus R.C.A. was placed and compacted with a single smooth drum 10-ton roller. This achieved top of Sediment Basin#1 berm subgrade.
- Contractor removed onsite natural soil berm at southern end of hotel backfill pad due to design changes extending the hotel to the south. The soil was removed to a firm natural material and compacted with a single smooth drum 10-ton roller. Placed one 12-inch thick loose lift of 3-inch minus R.C.A. along entire hotel footprint width including 20 feet outside of footprint and from 20 feet south of footprint to column line T-2.5 and compacted with a single smooth drum 10-ton roller. Nuclear density testing, visual observation and probing of the subgrade verified the lifts were placed in accordance with design specifications.
- Two to three inches of frost present in morning. Contractor used dozer to track and break the frost and compacted with a single smooth drum 10-ton roller. Placed two 12-inch thick loose lifts of natural on-site material on subgrade of Casino Footings C385 to C996 to C990 to C984 approximately 9 feet below slabon-grade elevation and compacted with a single smooth drum 10-ton roller. Natural on-site material came from the excavation of Sediment Basin #4 and parking garage #1 excavation. This backfills the former hotel location that had been excavated and is being filled with natural on-site material because the

Inspector: Robert Fioretti Signature

Page 1 of 2

 the subgrade verified the lift was place	ed in accordance with de-	sign specifications.
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	Page 2 of 2	Inspector: Dobort Figurett:
	1 450 2 01 2	Inspector: Robert Fioretti Signature
		~-Briming 4



	CONTRACTOR, LABOR & EQUIPMENT
QUANTITY	DESCRIPTION
2	Crew for Backfill-Tano

DATE: 11/25/2008	JOB NO. 5485	
PROJECT: Concord		
LOCATION: Lake Kiamesh	ıa, NY	
contractor: Fuller		
owner: Cappelli		
weather: Snow	темр: Mid 30's	
PRESEN	T AT SITE	
Oscar Nordstrom-Fuller	•	
Eric Beck-Tano		
- ""		

CHARGABLE EQUIPMENT	USED
Nuclear Density Gauge	

#### **GEOTECH INSPECTION REPORT**

- Five inches of frost present in morning. Contractor removed frost and placed in frost stockpile on tennis court prior to placement of fill. Placed four 12-inch thick loose lift of 3-inch minus R.C.A. and 3-inch minus crushed bedrock (from parking garage #1 excavation) from approximate elevation ±1478' to ±1481' along entire hotel footprint width including 20 feet outside of footprint and from column line T-2.5 to T-4.5 and compacted with a single smooth drum 10-ton roller. Nuclear density testing, visual observation and probing of the subgrade verified the lifts were placed in accordance with design specifications. Deo Persaud performed inspection.
- Working mat of concrete approximately 3 to 9-inches thick was poured from the south end of the hotel pad to column line T-2.5.
- Utility trench in north to south direct in second tier tennis court was excavated to a firm natural subgrade to the north of the future hotel pad. Utility trench from east to west in second tier tennis court was excavated to a firm natural subgrade to west of the above utility trench. Placed four 12-inch thick loose lifts of 6-inch minus crushed bedrock and compacted with a double drum sheepsfoot roller. Visual observation and probing of the subgrade verified the lifts were placed in accordance with design specifications. Deo Persaud performed inspection.

Page 1 of 1

Inspector: Robert Fioretti



	CONTRACTOR, LABOR & EQUIPMENT	
QUANTITY	DESCRIPTION	
2	Crew for Backfill-Tano	

DATE: 12/1/2008	<b>ЈОВ NO. 5485</b>
PROJECT: Concord	
LOCATION: Lake Kiames	sha, NY
CONTRACTOR: Fuller	# · · · · · · · · · · · · · · · · · · ·
owner: Cappelli	
WEATHER: P. Cloudy	темр: High 40's
PRESE	NT AT SITE
Oscar Nordstrom-Fulle	er
Eric Beck-Tano	
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CHARGABLE E	EQUIPMENT USED
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### GEOTECH INSPECTION REPORT

- Two to three inches of frost present in morning. Contractor used dozer to track and break the frost and compacted with a single smooth drum 10-ton roller. Placed one 12-inch thick loose lift of 3-inch minus crushed bedrock (from parking garage #1 excavation) from approximate elevation ±1478' to ±1479' along entire hotel footprint width including 20 feet outside of footprint to the east and from column line T-5 to T-7 and compacted with a single smooth drum 10-ton roller. Visual observation and probing of the subgrade verified the lifts were placed in accordance with design specifications.
- Contractor excavated approximately 8 feet of natural soil placed from 25 feet to 50 feet outside of former hotel footprint from column line T-4 to T-8 to a firm natural soil subgrade and compacted with a single smooth drum 10-ton roller. This location is approximately from the west wall of the new hotel footprint to 20 feet west of the hotel footprint. Placed two 12-inch thick loose lifts of 12-inch minus R.C.A. and from approximate elevation ±1471' to ±1472' and compacted with a single smooth drum 10-ton roller. Placed three 12-inch thick loose lifts of 3-inch minus crushed bedrock from approximate elevation ±1473' to ±1475' and compacted with a single smooth drum 10-ton roller. Visual observation and probing of the subgrade verified the lifts were placed in accordance with design specifications.

Inspector: Robert Fioretti
Signature



	CONTRACTOR, LABOR & EQUIPMENT	
QUANTITY	DESCRIPTION	
2	Crew for Backfill-Tano	

DATE: 12/2/2008	<b>ЈОВ NO. 5485</b>
PROJECT: Concord	·
LOCATION: Lake Kiamesh	ıa, NY
CONTRACTOR: Fuller	
owner: Cappelli	
weather: Sunny	темр: Mid 30's
PRESEN	Γ AT SITE
Oscar Nordstrom-Fuller	
Eric Beck-Tano	
	<u> </u>

CHARGABLE EQUIPMENT I	USED	
Nuclear Density Gauge		

# **GEOTECH INSPECTION REPORT**

- No frost present today. Placed two 12-inch thick loose lift of 3-inch minus R.C.A. from approximate elevation ±1482' to ±1484' along entire hotel footprint width including 20 feet outside of footprint from column line T-5 to T-8 and compacted with a single smooth drum 10-ton roller. Nuclear density testing, visual observation and probing of the subgrade verified the lifts were placed in accordance with design specifications.
- No frost present today. Placed three 12-inch thick loose lifts of 3-inch minus crushed concrete from 20' to 60' west of hotel footprint in area of indoor pool structure from approximate elevation ±1475' to ±1478' and compacted with a single smooth drum 10-ton roller. No density testing performed on crushed bedrock due to lack amount of gravel in material. Visual observation and probing of the subgrade verified the lifts were placed in accordance with design specifications.
- Contractor excavated approximately 10 feet of undisturbed existing grade fill material to a firm natural soil subgrade to elevation ± 1463'. This was performed from 60' to 90' west of hotel footprint in area of indoor pool structure. The soil was stockpiled in the frost stockpile on the tennis courts as it was highly moist. In area 75' to 90 west of hotel footprint concrete slab from tennis court was broken and a manhole and footing drain pipe were discovered. The manhole and footing drain were removed.

Inspector:	Robert Fioretti	
Signature		

•	Area in Casino footprint (to the east of the former ice rink in former building footprint) for approximate Casino Footings C12 to C15 to C772 to C53 was previously cut 3 to 4 feet to a firm natural soil subgrade to elevation $\pm$ 1475' and proofrolled with a single smooth drum 10-ton roller. Placed three 12-inch thick loose lifts of natural onsite material, from excavation 20 to 100 feet south of OU-1B in racetrack area, from approximate elevation $\pm$ 1475' to $\pm$ 1478' and compacted with a single smooth drum 10-ton roller. Nuclear density testing, visual observation and probing of the subgrade verified the lifts were placed in accordance with design specifications.

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Inspector: Robert Fioretti Signature



CONTRACTOR, LABOR & EQUIPMENT			
QUANTITY DESCRIPTION		QUANTITY	DESCRIPTION
2	Crew for Backfill-Tano		
	14.14		

-		
LOCATION: Lake Kiamesha, NY		
CONTRACTOR: Fuller		
owner: Cappelli		
PRESENT AT SITE		
Oscar Nordstrom-Fuller		
-		

CHARGABLE EQUIPMENT L	JSED
Nuclear Density Gauge	

## GEOTECH INSPECTION REPORT

- No frost present today. Area previously filled 11/28/08 from 25 feet to 50 feet outside of former hotel footprint from column line T-4 to T-8 (approx. from 5 feet inside to 20 feet outside of the west wall of new hotel). Placed four 12-inch thick loose lifts of 3-inch minus crushed bedrock, from parking garage #1 excavation, from approximate elevation ±1475' to ±1479' and compacted with a single smooth drum 10ton roller. This brought area up to grade of the remaining areas on the hotel footprint fill. No density testing performed on crushed bedrock due to size of particles in material. Visual observation and probing of the subgrade verified the lifts were placed in accordance with design specifications.
- No frost present today. Placed three 12-inch thick loose lift of 3-inch minus R.C.A. and 3-inch minus crushed bedrock, from parking garage #1 excavation, from approximate elevation ±1479' to +1482' along entire hotel footprint width including 20 feet outside of footprint from column line T-5 to T-8 and compacted with a single smooth drum 10-ton roller. Nuclear density testing, visual observation and probing of the subgrade verified the lifts were placed in accordance with design specifications.
- Chris Zwingle had a meeting onsite to discuss frost issues. It was decided that the remainder of the hotel footprint will have 22-inches of \(^3\)-inch clean crushed stone placed on it to subgrade to avoid the effects of frost. The areas already with a mud mat of concrete need to be covered and heated when temperature overnight are below freezing. A letter will be issued by SESI regarding these procedures.

Inspector: Robert Fioretti Signature

with design specifications.	probing of the subgrade	venned the lifts were placed in accordance
natural material from stripped material a 250 feet southwest of the gas station and approximate elevation $\pm 1470^{\circ}$ to $\pm 1475^{\circ}$	and racetrack excavation d swale excavation sout ' and compacted with a	ns (of the new tracking pad approximately hwest of sediment basin #3T) from single smooth drum 10-ton roller. Nucle
Contractor excavated approximately 10 elevation ± 1470°. This was performed	from 20' to 60' west of	hotel footprint in area of indoor pool
	elevation ± 1470'. This was performed structure. The soil was stockpiled in a matural material from stripped material 250 feet southwest of the gas station an approximate elevation ±1470' to ±1475 density testing, visual observation and p	elevation $\pm$ 1470'. This was performed from 20' to 60' west of structure. The soil was stockpiled in a location close by for reunatural material from stripped material and racetrack excavation 250 feet southwest of the gas station and swale excavation sout approximate elevation $\pm$ 1470' to $\pm$ 1475' and compacted with a density testing, visual observation and probing of the subgrade



CONTRACTOR, LABOR & EQUIPMENT		
QUANTITY	DESCRIPTION	
1	Crew for Backfill-Tano	

DATE: 11/22/2008	<b>JOB NO. 5485</b>		
PROJECT: Concord			
LOCATION: Lake Kiamesha, NY			
CONTRACTOR: Fuller			
owner: Cappelli			
weather: Windy	TEMP: Low 20's		
PRESENT AT SITE			
Oscar Nordstrom-Fuller			
Eric Beck-Tano			

CHARGABLE EQUIPMENT USED  Nuclear Density Gauge		
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## GEOTECH INSPECTION REPORT

• Six inches of frost present in morning. Contractor removed frost and placed in frost stockpile on tennis court prior to placement of fill. Placed three 12-inch thick loose lift of 3-inch minus R.C.A. and 3-inch minus crushed bedrock (from parking garage #1 excavation) from approximate elevation ±1472' to ±1474' along entire hotel footprint width including 20 feet outside of footprint and from 20 feet south of footprint to column line T-2.5 and compacted with a single smooth drum 10-ton roller. Nuclear density testing, visual observation and probing of the subgrade verified the lifts were placed in accordance with design specifications.

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Inspector: Deo Persaud



CONTRACTOR, LABOR & EQUIPMENT		
QUANTITY DESCRIPTION		
2	Crew for Backfill-Tano	
	-	
	*	

date: 11/26/2008	<b>јов no.</b> 5485	
ркојест: Concord		
LOCATION: Lake Kiamesh	ıa, NY	
CONTRACTOR: Fuller		
owner: Cappelli	<u> </u>	
WEATHER: P. Cloudy	темр: Mid 30's	
PRESENT AT SITE		
Oscar Nordstrom-Fuller		
Eric Beck-Tano	<u> </u>	
CHARGABLE EC	QUIPMENT USED	

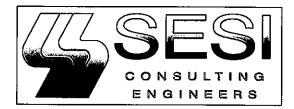
CHARGABLE EQUIPMENT USED			
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	<del>-</del>	<del></del> .	<u> </u>
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## GEOTECH INSPECTION REPORT

- Two to three inches of frost present in morning. Contractor used dozer to track and break the frost and compacted with a single smooth drum 10-ton roller. Placed four 12-inch thick loose lift of 3-inch minus crushed bedrock (from parking garage #1 excavation) from approximate elevation ±1482' to ±1485' along entire hotel footprint width including 20 feet outside of footprint and from column line T-3.5 to T-4.5 and compacted with a single smooth drum 10-ton roller. Visual observation and probing of the subgrade verified the lifts were placed in accordance with design specifications. Deo Persaud performed inspection.
- Utility trench from north to south in second tier tennis court was excavated to a firm natural subgrade to in the future hotel pad. Placed four 12-inch thick loose lifts of 6-inch minus crushed bedrock and compacted with a double drum sheepsfoot roller. Visual observation and probing of the subgrade verified the lifts were placed in accordance with design specifications. Deo Persaud performed inspection.

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Inspector: Robert Fioretti



CONTRACTOR, LABOR & EQUIPMENT		
QUANTITY	DESCRIPTION	
ĺ	Crew for Backfill-Tano	
<u></u>		

DATE: 11/21/2008	<b>ЈОВ NO. 5485</b>		
PROJECT: Concord			
LOCATION: Lake Kiamesha, NY			
CONTRACTOR: Fuller			
owner: Cappelli			
weather: P. Cloudy	темр: High 20's		
PRESENT AT SITE			
Oscar Nordstrom-Fuller			
Eric Beck-Tano			

CHARGABLE EQUIPMENT USED	 )
Nuclear Density Gauge	
	<u></u>

## **GEOTECH INSPECTION REPORT**

• Two to three inches of frost present in morning. Contractor used dozer to track and break the frost and compacted with a single smooth drum 10-ton roller. Placed six 12-inch thick loose lift of 3-inch minus R.C.A. and 3-inch minus crushed bedrock (from parking garage #1 excavation) from approximate elevation ±1465' to ±1471' along entire hotel footprint width including 20 feet outside of footprint and from 20 feet south of footprint to column line T-2.5 and compacted with a single smooth drum 10-ton roller. Nuclear density testing, visual observation and probing of the subgrade verified the lifts were placed in accordance with design specifications. Deo Persaud performed inspection.

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Inspector: Robert Fioretti



CONTRACTOR, LABOR & EQUIPMENT		
QUANTITY	DESCRIPTION	
3	Crew for Backfill-Tano	

date: 12/8/2008	<b>JOB NO. 5485</b>		
PROJECT: Concord			
LOCATION: Lake Kiamesh	ıa, NY		
contractor: Fuller			
owner: Cappelli	-		
WEATHER: Sunny TEMP: Low 20's			
PRESENT AT SITE			
Oscar Nordstrom-Fuller			
Eric Beck-Tano			
	, ,		

CHARGABLE EQUIPMENT USED		
Nuclear Density Gauge		
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# **GEOTECH INSPECTION REPORT**

- Twelve inches of frost present in morning. Contractor removed frost and placed in frost stockpile on to the north of Sediment Basin #1 prior to placement of fill. Placed four 12-inch thick loose lifts of natural onsite material, from racetrack excavation 20 to 100 feet south of OU-1B, from 20' to 60' west of hotel footprint in area of indoor pool structure, from column lines T-1 to T-7, from approximate elevation ±1478' to ±1482' and compacted with a single smooth drum 10-ton roller. Nuclear density testing, visual observation and probing of the subgrade verified the lifts were placed in accordance with design specifications. Deo Persaud performed inspection.
- Rat slab poured in hotel footprint from column line T-3.5 to T-7 except for elevator pit area which is completed.
- Twelve inches of frost present in morning. Contractor removed frost and placed in frost stockpile on to the north of second tier tennis court prior to placement of fill. Placed two 12-inch thick loose lifts of natural onsite material, from racetrack excavation 20 to 100 feet south of OU-1B, from approximate elevation ±1465' to ±1467', inside of Ballroom footprint on the north side of second tier tennis court and compacted with a single smooth drum 10-ton roller. Nuclear density testing, visual observation and probing of the subgrade verified the lifts were placed in accordance with design specifications. Deo Persaud performed inspection.
- Twelve inches of frost present in morning, eight inches removed. Contractor removed frost and took it to the temporary topsoil stockpile. Placed stabilization fabric between Casino column line CF to CD and C17 to C19. Placed 8-inches of ¾-inch clean crushed stone as working surface for upper level construction. Area will be prooffolled in spring prior to slab-on-grade approval.

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Inspector: Robert Fioretti



CONTRACTOR, LABOR & EQUIPMENT			
	CONTRACTOR, LABOR & EQUIPMENT		
QUANTITY	DESCRIPTION		
1	Crew for Backfill-Tano		
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	-			
DATE: 12/12/2008	јов no. 5485			
PROJECT: Concord				
LOCATION: Lake Kiamesh	ia, NY			
CONTRACTOR: Fuller				
owner: Cappelli				
WEATHER: P. Cloudy	темр: Mid 30's			
PRESENT AT SITE				
Oscar Nordstrom-Fuller				
Eric Beck-Tano				
CHARGABLE EQUIPMENT USED				

CHARGABLE EQUIPMENT USED			
			<del>-</del>
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# **GEOTECH INSPECTION REPORT**

No frost present in morning. Contractor removed 8-inches of crushed bedrock material and stockpiled for future use. Placed stabilization fabric between Casino column line CF to CB and C14 to C15. Placed 8inches of ¾-inch clean crushed stone as working surface for upper level construction. Area will be prooffolled in spring prior to slab-on-grade approval.

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Inspector: Robert Fioretti Signature



CONTRACTOR, LABOR & EQUIPMENT		
QUANTITY	DESCRIPTION	
3	Crew for Backfill-Tano	

date: 12/6/2008	<b>ЈОВ NO. 5485</b>
PROJECT: Concord	
LOCATION: Lake Kiames	ha, NY
contractor: Fuller	
owner: Cappelli	
WEATHER: P. Cloudy	темр: High 20's
PRESEN	T AT SITE
Oscar Nordstrom-Fulle	r
Eric Beck-Tano	
OW A DOLLAR DE D	
CHARGABLE E	QUIPMENT USED

# **GEOTECH INSPECTION REPORT**

• No frost present in areas to be worked this morning. 18-inches of ¾-inch clean crushed stone placed on hotel footprint from T-3.5 to T-12 except for elevator pit area which is already complete. Stone was compacted with a single drum 10-ton roller.

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Inspector: Deo Persaud



CONTRACTOR, LABOR & EQUIPMENT		
QUANTITY	DESCRIPTION	
3	Crew for Backfill-Tano	

DATE: 12/4/2008	<b>JOB NO. 5485</b>		
PROJECT: Concord			
LOCATION: Lake Kiames	LOCATION: Lake Kiamesha, NY		
CONTRACTOR: Fuller			
owner: Cappelli			
WEATHER: Rain TEMP: Mid 30's			
PRESENT AT SITE			
Oscar Nordstrom-Fuller			
Eric Beck-Tano			

CHARGABLE EQUIPMENT USED		
Nuclear Density Gauge		
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# **GEOTECH INSPECTION REPORT**

- No frost present this morning. Placed eight 12-inch thick loose lifts of 3-inch minus crushed bedrock, in hotel elevator pit area, from the west foundation wall to column line TA1.5 and column line T-9 to T-12 from approximate elevation ±1473' to ±1481' and compacted with a single smooth drum 10-ton roller. Area previously excavated approximately 8 feet to a firm natural soil subgrade to approximate elevation ±1473'. 22-inches of ¾-inch clean crushed stone placed from the west foundation wall to column line TA2 and column line T-9 to T-12 to approximate elevation ±1481'. No density testing performed on crushed bedrock due to amount of gravel in material. Visual observation and probing of the subgrade verified the lifts were placed in accordance with design specifications. Deo Persaud performed inspection.
- No frost present this morning. Placed 12 to 18-inches of ¾-inch clean crushed stone over entire hotel footprint area from column lines T-3.5 to T-12 and compacted with a single smooth drum 10-ton roller. Area was either at approximate elevation ±1485' or was stripped, if necessary, 12 to 18-inches to approximate elevation ±1485'. This was done to allow for 22-inches of ¾-inch clean crushed stone and a 2-inch concrete rat slab before achieving elevation ±1487' which is footing subgrade. Placement of ¾-inch clean crushed stone will continue as required to achieve 22-inches. No density testing performed on crushed bedrock due to amount of gravel in material. Visual observation and probing of the subgrade verified the lifts were placed in accordance with design specifications. Deo Persaud performed inspection.

Inspector: Robert Fioretti

•	No frost present this morning. Placed three 12-inch thick loose lifts of natural onsite material, from racetrack excavation 20 to 100 feet south of OU-1B, from approximate elevation ±1463' to ±1466', inside of Ballroom footprint on the north side of second tier tennis court and compacted with a single smooth drum 10-ton roller. Tennis court concrete was previously broken, tracked with a dozer and left in place with large pieces of metal removed. Nuclear density testing, visual observation and probing of the subgrade verified the lifts were placed in accordance with design specifications. Deo Persaud performed inspection.

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Inspector: Robert Fioretti Signature



CONTRACTOR, LABOR & EQUIPMENT	
QUANTITY	DESCRIPTION
1	Crew for Backfill-Tano
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DATE: 12/15/2008	<b>ЈОВ NO. 5485</b>	
PROJECT: Concord		
LOCATION: Lake Kiamesha, NY		
CONTRACTOR: Fuller		
owner: Cappelli		
WEATHER: P. Cloudy	темр: Mid 50's	
PRESEN	Γ AT SITE	
Oscar Nordstrom-Fuller		
Eric Beck-Tano		

CHARGABLE EQUIPMENT USED	_
Nuclear Density Gauge	

### **GEOTECH INSPECTION REPORT**

- No frost present in morning. Placed stabilization fabric between Casino column line CF to CD and C12.5 to C14. Placed 8-inches of ¾-inch clean crushed stone as working surface for upper level construction. Area will be proofrolled in spring prior to slab-on-grade approval.
- No frost present in morning. Placed one 12-inch thick loose lifts of natural onsite material, from racetrack sediment basin C excavation, from approximate elevation ±1463' to ±1464', inside of Ballroom footprint on the north side of second tier tennis court on the east side of the haul road in the center of the tennis court and compacted with a single smooth drum 10-ton roller. Nuclear density testing, visual observation and probing of the subgrade verified the lifts were placed in accordance with design specifications.
- No frost present in morning. Placed two 12-inch thick loose lifts of R.C.A. material from 0' to 20' west of hotel footprint in area of indoor pool structure, from column lines T-3.5 to T-11, from approximate elevation ±1482' to ±1484' and compacted with a single smooth drum 10-ton roller. Nuclear density testing, visual observation and probing of the subgrade verified the lifts were placed in accordance with design specifications.
- No frost present in morning. Contractor removed 6-inches of mud and brought it to the temporary topsoil stockpile. Placed two 12-inch thick loose lifts of natural onsite material, from racetrack sediment basin C excavation, from approximate elevation ±1482' to ±1484', approximately 0' to 20' east of hotel footprint in area for roadway and compacted with a single smooth drum 10-ton roller. Nuclear density testing, visual observation and probing of the subgrade verified the lifts were placed in accordance with design specifications.

Page 1

of 2	Inspector: Robert Fioretti	
	Signature	

	Page	2 of 2	Inspector: Robert Fioretti Signature
	for C387 and C392. No frost is expected and	1 looding subgrade	will be checked again tomorrow.
	and C980/C994. It was discovered later in the and water the subgrade for C387 and C392 d 1 foot and 3/4-inch clean crushed stone was placed subgrades verified a footing subgrade of 4.5	eteriorated. The fo aced. Visual obser TSF bearing capaci	otings were over excavated approximate vation and probing of the footing ty. All footings were poured today exce
•	subgrade for Casino Footings C387, C392, C Excavated 3 to 4 feet below existing subgrad	977 and compacted to bedrock for Ca	with double-drum sheepsfoot roller. sino Footings C997/C992, C968/C993
•	Everyoted 3 to 4 foot heless switting out	0 to 0 fam.	de bookstod og dagen at 10 gr



CONTRACTOR, LABOR & EQUIPMENT		
QUANTITY	DESCRIPTION	
1	Crew for Backfill-Tano	
•		
	<u> </u>	

DATE: 12/18/2008	<b>јов no.</b> 5485	
PROJECT: Concord		
LOCATION: Lake Kiamesh	a, NY	
CONTRACTOR: Fuller		
owner: Cappelli		
WEATHER: Sunny TEMP: Mid 30's		
PRESENT AT SITE		
Oscar Nordstrom-Fuller		
Eric Beck-Tano		
	· · · · · · · · · · · · · · · · · · ·	
12.5	<u> </u>	

CHARGABLE EQUIPMENT USED	
Nuclear Density Gauge	
	<u>-</u>

### GEOTECH INSPECTION REPORT

- Placed 12-inches ¾-inch clean crushed stone, from column lines T-3.5 to T-11, from 0' to 20' west of hotel footprint in area of indoor pool structure, to approximate elevation ±1489'. Now 24-inches of ¾-inch clean crushed stone in this area. Deo Persaud performed inspection.
- Two to three inches of frost present in morning. Contractor used dozer to track and break the frost and compacted with a single smooth drum 10-ton roller. Placed two 12-inch thick loose lifts of natural onsite material, from racetrack sediment basin C excavation, from 20' to 40' west of hotel footprint in area of indoor pool structure, from column lines T-3.5 to T-11, from 0' to 40' west of hotel footprint, from approximate elevation ±1486' to ±1488', from column line T-2 to T-3.5, from approximate elevation ±1476' to ±1478'. Nuclear density testing, visual observation and probing of the subgrade verified the lifts were placed in accordance with design specifications. Deo Persaud performed inspection.
- Two to three inches of frost present in morning. Contractor removed 2 feet of material from column line C11 to C13 from CW to CZ, that was deteriorated due to high traffic from concrete pour yesterday. Placed one 12-inch thick loose lift of 12-inch minus R.C.A. and one 12-inch think loose lift of 3-inch minus R.C.A., from approximate elevation ±1488' to ±1490' and compacted with a single smooth drum 10-ton roller. No density testing performed due to high gravel content. Visual observation and probing of the subgrade verified the lifts were placed in accordance with design specifications. Deo Persaud performed inspection.

2 Inspector: Robert Fioretti Signature

Page 1 of 2

•	- voing one grant for the country of the country in the country to come of the country in the co
	above was reapproved to elevation ±1490'. Footings were covered with blankets to protect the subgrade
	from frost overnight. Excavated 2 to 3 feet below existing subgrade to a firm previously backfilled and
	approved footing subgrade for Casino Footings C988, C989 and C990 and to bedrock for Casino Footing
	C991/C995. Footings C988 and C990 were compacted with a double drum sheepsfoot roller and C989
	was over-excavated 12-inches due to moist material and 3-inch minus R.C.A. was placed and compacted
	with a double drum sheepsfoot roller. Casino Footing C387 previously rejected was removed ¾-inch
	stone was added to areas were stone was previously removed. Visual observation and probing of the
	footing subgrades verified a footing subgrade of 4.5 TSF bearing capacity. All footings were poured
	today. All pours were covered with blankets to protect the subgrade and footings.

•	MW-BSEA-5 was removed today in area on north of second tier tennis court.	The monitoring well was
	backfilled with bentonite pellets and the pvc pipe and steel casing removed.	_

Inspector: Robert Fioretti Signature Page 2 of 2



CONTRACTOR, LABOR & EQUIPMENT		
QUANTITY	DESCRIPTION	
1	Crew for Backfill-Tano	

<b>JOB NO.</b> 5485		
a, NY		
TEMP: Low 50's		
T AT SITE		
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777		
QUIPMENT USED		

GFC	TECH	INSPECTION	RFPORT
$\cup$	<i>,</i>	HAOL FOLIOIA	ILLLOIVI

• No frost present in morning. Contractor removed 8-inches of crushed bedrock material and stockpiled for future use. Placed stabilization fabric between Casino column line CF to CB and C16 to C17. Placed 8-inches of ¾-inch clean crushed stone as working surface for upper level construction. Area will be proofrolled in spring prior to slab-on-grade approval. Deo Persaud performed inspection.

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Inspector: Robert Fioretti



CONTRACTOR, LABOR & EQUIPMENT		
QUANTITY	DESCRIPTION	
1	Crew for Backfill-Tano	
<u></u>		

<b>јов no. 5485</b>			
ia, NY			
TEMP: Mid 20's			
PRESENT AT SITE			
<u></u>			

CHARGABLE EQUIPMENT USE	)
Nuclear Density Gauge	

## **GEOTECH INSPECTION REPORT**

• It was determined that work would be stopping for two weeks due to weather and holidays. Four to five inches of frost present in morning. Contractor did not remove frost due to pending work stoppage informed SESI representative that any unsatisfactory areas would be addressed later. Placed four 12-inch thick loose lifts of natural onsite material, from racetrack sediment basin C excavation around Casino Footings C377, C965, and C975 which were covered overnight and had no frost. Placed two 12-inch thick loose lifts of natural onsite material, from racetrack sediment basin C excavation from approximate elevation ±1492' to ±1494' from column lines C14 to C15 from CW to CZ, and one 12-inch thick loose from elevation ±1492' to ±1494' C15 to C19 from CW to CZ. Nuclear density testing, visual observation and probing of the subgrade verified the lifts were placed in accordance with design specifications. Due to frosty slab-on-grade subgrade that will need to be addressed prior to pour.

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Inspector: Robert Fioretti



CONTRACTOR, LABOR & EQUIPMENT		
QUANTITY DESCRIPTION		
1 Crew for Backfill-Tano		

DATE: 12/16/2008	<b>јов no.</b> 5485	
PROJECT: Concord		
LOCATION: Lake Kiamesh	ia, NY	
CONTRACTOR: Fuller		
owner: Cappelli		
WEATHER: P. Cloudy	темр: Mid 40's	
PRESENT AT SITE		
Oscar Nordstrom-Fuller		
Eric Beck-Tano		
****		

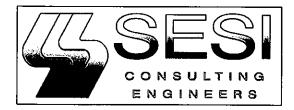
CHARGABLE EQUIPMENT USED		
Nuclear Density Gauge		

## **GEOTECH INSPECTION REPORT**

- No frost present in morning. Placed stabilization fabric between Casino column line CF to CD and C11 to C112.5. Placed 8-inches of 34-inch clean crushed stone as working surface for upper level construction. Area will be proofrolled in spring prior to slab-on-grade approval.
- No frost present in morning. Placed two 12-inch thick loose lifts of natural onsite material, from racetrack sediment basin C excavation, from approximate elevation ±1464' to ±1466', inside of Ballroom footprint on the north side of second tier tennis court on the east side of the haul road in the center of the tennis court and compacted with a single smooth drum 10-ton roller. Nuclear density testing, visual observation and probing of the subgrade verified the lifts were placed in accordance with design specifications.
- No frost present in morning. Placed three 12-inch thick loose lifts of natural onsite material, from racetrack sediment basin C excavation, from approximate elevation ±1476' to ±1479', around the first tier of the hotel on the southern end backfilling the foundation and protecting it from frost. The lifts were compacted with a single smooth drum 10-ton roller. Nuclear density testing, visual observation and probing of the subgrade verified the lifts were placed in accordance with design specifications.
- Excavated 3 to 4 feet below existing subgrade to a firm previously backfilled and approved footing subgrade for Casino Footings C386, C966, C976 and compacted with double-drum sheepsfoot roller. Footing subgrade for Casino Footings C387 and C392 was approved yesterday and was approved again today. Footing C217/C941 was removed due to design changes and subgrade remained approved. Visual observation and probing of the footing subgrades verified a footing subgrade of 4.5 TSF bearing capacity. All footings were poured today. No frost is expected and footing subgrade will be checked again tomorrow.

Page 1 of 2	Inspector: Robert Fioretti
	Signature

	Page 2 of	2 Inspecto Signatu	or: Robert Fioretti re
	approximate elevation ±1481' to ±1482', approximate roadway and compacted with a single smooth dructushed bedrock due to size of material. Visual of were placed in accordance with design specification	m 10-ton roller. No description and probing	ensity testing performed on
•	No frost present in morning. Contractor removed topsoil stockpile. Placed one 12-inch thick loose.		



CONTRACTOR, LABOR & EQUIPMENT		
QUANTITY	DESCRIPTION	
3	Crew for Backfill-Tano	
·		

DATE: 12/3/2008	<b>ЈОВ NO. 5485</b>
PROJECT: Concord	<del></del>
LOCATION: Lake Kiam	esha, NY
CONTRACTOR: Fuller	
owner: Cappelli	
weather: Sunny	темр: Mid 30's
PRES	SENT AT SITE
Oscar Nordstrom-Ful	ller
Eric Beck-Tano	
	<del></del>

CHARGABLE EQUIPMENT USED				
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			•	
 			_	<del>-</del>
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### GEOTECH INSPECTION REPORT

- Four inches of frost present in morning. Contractor removed frost and placed in frost stockpile on tennis court prior to placement of fill. Placed one 12-inch thick loose lift of 3-inch minus crushed bedrock, from parking garage #1 excavation, from approximate elevation ±1484' to ±1485' along entire hotel footprint width including 20 feet outside of footprint from column line T-5 to T-6 and compacted with a single smooth drum 10-ton roller. No density testing performed on crushed bedrock due to amount of gravel in material. Visual observation and probing of the subgrade verified the lifts were placed in accordance with design specifications.
- Area for hotel elevation pit from column lines TA1.5 to TA2 and column line T-9 to T-12 was excavated approximately 3 feet from slab-on-grade subgrade. Area from the west foundation wall to column line TA1.5 and column line T-9 to T-12 was excavated approximately 8 feet to a firm natural soil subgrade to approximate elevation ±1473' because area was previously backfilled with soil from former hotel pad. Area needs to be filled to achieve elevator pit subgrade elevation.
- Four inches of frost present in morning. Contractor removed frost and placed in frost stockpile on tennis court prior to placement of fill. Placed one 12-inch thick loose lifts of 3-inch minus crushed bedrock, from parking garage #1 excavation, from 20' to 60' west of hotel footprint in area of indoor pool structure from approximate elevation ±1478' to ±1479' and compacted with a single smooth drum 10-ton roller. No density testing performed on crushed bedrock due to amount of gravel in material. Visual observation and probing of the subgrade verified the lifts were placed in accordance with design specifications.

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- Four inches of frost present in morning. Contractor removed frost and placed in frost stockpile on tennis court prior to placement of fill. Placed three 12-inch thick loose lifts of 12-inch minus crushed concrete, from 60' to 90' west of hotel footprint in area of indoor pool structure from approximate elevation ±1463' to ±1466' and compacted with a single smooth drum 10-ton roller. Area in which manhole was removed was filled with approximately four 12-inch thick loose lifts of 12-inch minus crushed concrete and heeled with bucket. No density testing performed on crushed bedrock due to size of material. Visual observation and probing of the subgrade verified the lifts were placed in accordance with design specifications.
- Six inches of frost present in natural onsite material this morning. Contractor removed frost and placed in frost stockpile on tennis court prior to placement of fill. Area in outside of Casino footprint to the north of second tier tennis court was previously cut 1 foot to a firm natural soil subgrade to elevation ± 1460' and proofrolled with a single smooth drum 10-ton roller. Placed three 12-inch thick loose lifts of natural onsite material (from stripped material and from racetrack excavation 20 to 100 feet south of OU-1B) to approximate elevation ±1460' to ±1463' and compacted with a single smooth drum 10-ton roller. Visual observation and probing of the subgrade verified the lifts were placed in accordance with design specifications.

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Inspector: Robert Fioretti



CONTRACTOR, LABOR & EQUIPMENT		
QUANTITY	DESCRIPTION	
3	Crew for Backfill-Tano	

<b>ЈОВ NO. 5485</b>			
PROJECT: Concord			
sha, NY			
CONTRACTOR: Fuller			
owner: Cappelli			
WEATHER: P. Cloudy TEMP: High 20's			
PRESENT AT SITE			
Oscar Nordstrom-Fuller			
Eric Beck-Tano			
<del>*</del>			

CHARGABLE EQUIPMENT USED	
Nuclear Density Gauge	
100	
-314	

#### GEOTECH INSPECTION REPORT

- No frost present this morning. Rat slab poured in hotel elevator pit area. 2 to 3-inches of concrete poured. 18-inches of ¾-inch clean crushed stone placed on hotel footprint from T-3.5 to T-12 except for elevator pit area. At least 22-inches of ¾-inch clean crushed stone will be placed and a 2 to 3 inch mud mat of concrete will be poured to prevent frost of the subgrade. Deo Persaud performed inspection.
- Frost stockpile on second tier tennis court was moved to temporary topsoil stockpile.
- No frost present this morning. Placed three 12-inch thick loose lifts of natural onsite material, from racetrack excavation 20 to 100 feet south of OU-1B, from 20' to 60' west of hotel footprint in area of indoor pool structure, from column lines T-7 to T-12, from approximate elevation ±1479' to ±1482' and compacted with a single smooth drum 10-ton roller. Density testing, visual observation and probing of the subgrade verified the lifts were placed in accordance with design specifications. Deo Persaud performed inspection.
- No frost present this morning. Placed three 12-inch thick loose lifts of natural onsite material, from racetrack excavation 20 to 100 feet south of OU-1B, from approximate elevation ±1463' to ±1466', outside of Casino footprint to the north of second tier tennis court and compacted with a single smooth drum 10-ton roller. Nuclear density testing, visual observation and probing of the subgrade verified the lifts were placed in accordance with design specifications. Deo Persaud performed inspection.

Inspector: Robert Fioretti Signature

	Page 2 o		nspector: Robert Fioretti Signature	
	visual observation and probing of the subgrade v specifications. Deo Persaud performed inspection	erified the lifts v n.	vere placed in accordance wi	th design
	brought it to the temporary topsoil stockpile in a C795/C981. Placed four 12-inch thick loose lifts to 100 feet south of OU-1B, from approximate elements of the control of	of natural onsite evation ±1484'	e material, from racetrack exe to ±1488'. Nuclear density to	cavation 20
•	Constitution to the contract ambantage	material to appr	oximately 10 feet below subg	grade and



CONTRACTOR, LABOR & EQUIPMENT		
QUANTITY	DESCRIPTION	
3	Crew for Backfill-Tano	
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DATE: 12/9/2008	JOB NO. 5485				
PROJECT: Concord	PROJECT: Concord				
LOCATION: Lake Kiames	sha, NY				
CONTRACTOR: Fuller	*				
owner: Cappelli					
WEATHER: P. Cloudy TEMP: High 40's					
PRESENT AT SITE					
Oscar Nordstrom-Fuller					
Eric Beck-Tano					

CHARGABLE EQUIPMENT I	USED
Nuclear Density Gauge	
12 17 12 17	

### GEOTECH INSPECTION REPORT

- Six to twelve inches of frost present in morning. Contractor removed frost and placed in frost stockpile on to the north of second tier tennis court prior to placement of fill. Placed five 12-inch thick loose lifts of natural onsite material, from racetrack excavation 20 to 100 feet south of OU-1B, from approximate elevation ±1466' to ±1471', inside of Ballroom footprint on the north side of second tier tennis court and approximately 100 feet north of the Ballroom and compacted with a single smooth drum 10-ton roller. Nuclear density testing, visual observation and probing of the subgrade verified the lifts were placed in accordance with design specifications. Deo Persaud performed inspection.
- Six to twelve inches of frost present in morning. Contractor removed frost and placed in frost stockpile on to the north of second tier tennis court prior to placement of fill. Placed three 12-inch thick loose lifts of natural onsite material in area of Casino footings C792/C203 to C384 to C984 to C795/C981, from racetrack excavation 20 to 100 feet south of OU-1B, from approximate elevation ±1487' to ±1490'. Nuclear density testing, visual observation and probing of the subgrade verified the lifts were placed in accordance with design specifications. Deo Persaud performed inspection.
- Twelve inches of frost present in morning, eight inches removed. Contractor removed frost and took it to the temporary topsoil stockpile. Placed stabilization fabric between Casino column line CD to CB and C17 to C19. Placed 8-inches of ¾-inch clean crushed stone as working surface for upper level construction. Area will be proofrolled in spring prior to slab-on-grade approval.

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Inspector: Robert Fioretti

•	Excavated 3 to 4 feet below existing subgrade to a firm previously backfilled and approved footing
	subgrade for Casino Footings C388, C397, C978, C996, C967, and C979 and compacted with double-
	drum sheepsfoot roller. Visual observation and probing of the footing subgrades verified a footing
	subgrade of 4.5 TSF bearing capacity. All footings were poured today. Deo Persaud performed
	inspection.

•	Placed one 12-inch thick loose lifts of natural onsite material from racetrack excavation 20 to 100 feet
	south of OU-1B on area of hotel footing on northeast side of footprint to avoid frost getting under the
	footing. Told this was a temporary placement and was not compacted.

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Inspector: Robert Fioretti Signature\_\_\_\_



	CONTRACTOR, LABOR & EQUIPMENT		
QUANTITY	DESCRIPTION		
1	Crew for Backfill-Tano		

date: 12/17/2008	<b>ЈОВ NO. 5485</b>
PROJECT: Concord	
LOCATION: Lake Kiames	ha, NY
contractor: Fuller	
owner: Cappelli	
weather: P. Cloudy	темр: Mid 30's
PRESEN	NT AT SITE
Oscar Nordstrom-Fulle	r
Eric Beck-Tano	•
<u> </u>	

CHARGABLE EQUIPMENT USE	ED
Nuclear Density Gauge	
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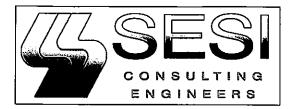
#### GEOTECH INSPECTION REPORT

- No frost present this morning. Placed one 12-inch thick loose lift of 12-minus R.C.A. material, from approximate elevation ±1463' to ±1464'. Placed three 12-inch thick loose lifts of natural onsite material, from racetrack sediment basin C excavation, from approximate elevation ±1464' to ±1467', inside of Ballroom footprint on the far north side of second tier tennis court on the east side of the haul road in the center of the tennis court and compacted with a single smooth drum 10-ton roller. Nuclear density testing, visual observation and probing of the subgrade verified the lifts were placed in accordance with design specifications. Deo Persaud performed inspection.
- No frost present this morning. Contractor removed 2 feet of extremely moist material and brought it to the temporary topsoil stockpile. Placed two 12-inch thick loose lifts of natural onsite material, from racetrack sediment basin C excavation, from column line C12 to C14 from CW to CZ, from approximate elevation ±1488' to ±1490' and compacted with a single smooth drum 10-ton roller. Visual observation and probing of the subgrade verified the lifts were placed in accordance with design specifications. Deo Persaud performed inspection.
- Footing subgrade for Casino Footing C377, C965, and C975, column line C14 form CW to CZ as shown above was approved to elevation ±1490'. Contractor placed 8 to 12-inches of ¾-inch clean crushed stone for C377 and C965, and 8 to 12-inches of R.C.A. material for C975 and compacted with double-drum sheepsfoot roller. Visual observation and probing of the footing subgrades verified a footing subgrade of 4.5 TSF bearing capacity. Footings were not poured today and footing subgrade will be checked again tomorrow. The footings were covered with blankets to protect the subgrade from frost. Deo Persaud performed inspection.

Page 1 of 2 Inspector: Robert Fioretti

- Casino Footings from column line C15 to C19 and CW to CZ were backfilled with 3 or 4 12-inch thick loose lifts of R.C.A. material depending upon depth to footing bottom. Loose moist material was removed to a firm material prior to backfill. MHE rejected footings C387, C978 and C979 due to presence of water and backfill procedures. Michael St. Pierre (MSP) inspected all 3 footings and found them to be in good condition. C978 and C979 had not had water, ice and muck removed from around footing and C387 had become undermined in a small area on 1 side of the footing. MSP discussed the conditions with Michael Lamorueax(ML) and ML agreed with MSP that footings C978 and C979 could remain provided that Mark Skellett inspect the mucking out of the footings. ML did not agree with MSP on footing C387 and required it to be removed. MSP was present during the mucking out of footing C978 and found the footing subgrade to be in a firm condition. The majority of the muck was removed and approximately 6 inches of ¾ inch clean crushed stone was placed in the bottom of the excavation. Footing backfill with RCA continued from there. Visual observation and probing of the subgrade verified the lifts were placed in accordance with design specifications.
- No frost present in morning. Placed three 12-inch thick loose lifts of R.C.A. material from 0' to 20' west of hotel footprint in area of indoor pool structure, from column lines T-3.5 to T-11, from approximate elevation ±1484' to ±1487' and compacted with a single smooth drum 10-ton roller. Placed 12-inches of ¾-inch clean crushed stone. Nuclear density testing, visual observation and probing of the subgrade verified the lifts were placed in accordance with design specifications. Deo Persaud performed inspection.
- Placed 24-inch thick loose lifts of 12-inch minus R.C.A. material, from approximate elevation ±1463' to ±1465', in future hotel footprint in area on east side of second tier tennis court area. Area was not compacted as laborers where taking out any loose metal material. Deo Persaud performed inspection.

Inspector: Robert Fioretti
Signature



CONTRACTOR, LABOR & EQUIPMENT		
QUANTITY	DESCRIPTION	
1	Crew for Backfill-Tano	

DATE: 12/11/2008	<b>ЈОВ NO. 5485</b>	
PROJECT: Concord		
LOCATION: Lake Kiame	sha, NY	
contractor: Fuller		
owner: Cappelli		
weather: Rain	темр: Mid 30's	
PRESENT AT SITE		
Oscar Nordstrom-Fulle	er	
Eric Beck-Tano		
<u> </u>		
CHARGABLE	EQUIPMENT USED	

# **GEOTECH INSPECTION REPORT**

• No frost present in morning. Contractor removed 8-inches of crushed bedrock material and stockpiled for future use. Placed stabilization fabric between Casino column line CF to CB and C15 to C16. Placed 8-inches of ¾-inch clean crushed stone as working surface for upper level construction. Area will be prooffolled in spring prior to slab-on-grade approval. Deo Persaud performed inspection.

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Inspector: Robert Fioretti