

GEOTECHNICAL ENGINEERING INVESTIGATION REPORT

FOR

THE CONCORD RESORT
Thompson, New York

PREPARED FOR:

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INTRODUCTION AND BACKGROUND

We have completed our engineering study of the subsurface conditions as they pertain to foundations and site grading for the proposed Concord Resort to be located at the site of the old Concord Hotel in the Town of Thompson, Sullivan County, New York. The main hotel site is bounded by Kiamesha Lake Road to the north, Concord Road to the east, a 9-hole golf course to the south and Kiamesha Lake to the west. The proposed track area is bounded by Concord Road to the west, the International golf course to the north and east and undeveloped land to the south.

Based on our review of the O.R. Mancini, Inc. report dated September 8, 1998, the original Concord Hotel was constructed in the early 1930's with numerous additions from 1950 to 1984. The hotel has recently been demolished and the foundations removed.

Based on the most recent site plans prepared for the project, the planned development will consist of: construction of a Casino with one-level of parking below; two (2) 4-story (above at-grade parking) parking structures; an indoor pool; an approximate 30-story hotel; a ballroom; a theater and event center; a harness track; a paddock; a clubhouse; a shed; retaining walls; associated utilities and at-grade paved parking and roadway areas.

Topographically, existing grades in the planned building areas vary from elevation 1463± in the western portion of the property, to elevation 1501± in the north corner of Parking Garage 2 and then downward to elevation 1476 ± at the eastern corner of Parking Garage 1, in the eastern side of the property. There is a high point of 1510+ in Kiamesha Lake Road in the northeastern portion of the project area.

In the area of the proposed track, the site grades vary from elevation 1552± near the center of the proposed track, sloping downwards in all directions to a low elevation of 1490± in the area of the maintenance shed.

Based on the proposed grades, cuts of up to 11± feet and fills to 31± feet will be required to achieve building area subgrade elevations in the main casino and hotel area.

In the area of the proposed track, cuts of up to 53± feet and fills to 21± feet will be required to achieve subgrade elevations.

It should be noted that the exact building locations and proposed grades may still vary by a few feet, but should not affect the recommendations presented in this report.

FIELD AND LABORATORY INVESTIGATIONS

Our study consisted of a site reconnaissance, a review of existing soils and geologic data, and multiple field investigations consisting of test pits excavated with a rubber-tire backhoe, borings drilled using a rotary drill rig and geoprobes. The locations of the test pits, borings and geoprobes are illustrated on Figures 1 and 1B.

Our subsurface field investigations were done in September 2000, July 2002, October-November 2007, and April thru October 2008. A total of 53 borings, 44 test pits and 33 geoprobes were completed.

The fieldwork was done under the full-time observation of engineers/technicians from our firm. The individual boring logs, test pit logs and geoprobe logs, which describe the materials encountered, are presented in the attached Appendices.

Representative samples of the soils encountered in the borings and test pit excavations were obtained and brought to our laboratory for visual identification and classification testing. Classification testing consisted of water content determinations, minus No. 200 mesh sieve tests, and mechanical grain size distribution analyses. The results of the water contents and minus No. 200 sieve tests are shown on the individual boring logs. The results of the grain size analyses are shown in graphical form in the Appendix.

SUBSURFACE CONDITIONS

General

The generalized subsurface conditions consist of a miscellaneous fill varying from 1.0 to 8.0± feet thick in the planned building areas overlying dense to very dense sands and silts. Underlying the sands at depths of 2 feet to 39 feet is sandstone/shale bedrock. Rock coring was done in several of the borings. The thickness of fill and depth to bedrock at each boring and test pit location is shown on Figures 1 and 1B.

Casino, Event Center, Ballroom & Theater Area

Approximately 1 to 8± feet of miscellaneous fill exists in the building areas overlying medium-dense to very dense coarse to fine sand with varying amounts of silt and gravel. Occasional layers of clayey silt/silt were encountered beneath the fill and above the sand stratum in some locations.

Sandstone and shale bedrock was encountered at depths of 2.0 ± to 21 ± feet (elevation 1457± to 1496 ±) below existing grade. Rock coring varying from 5

feet to 10 feet was done in 11 of the borings drilled in this area. The percent core recovery varied from 12.5 to 100 and the RQD values (rock quality designation) varied from 0 to 82 percent indicating highly variable rock quality from poor to good.

Hotel Tower

Beneath a thin topsoil/asphalt layer in one of the borings is 3± feet of miscellaneous mostly granular fill. The remainder of the borings did not encounter a fill layer. The soils beneath the fill/topsoil/asphalt vary from very dense coarse to fine sand to very dense silt, with varying amounts of sand and gravel.

Sandstone and shale bedrock was encountered at depths of 7.5 to 20± feet (elevation 1460± to 1475±) below existing grade. Rock coring varying from 5 to 16.5 feet was done in three of the borings drilled in this area. The percent core recovery varied from 48 to 100 and the RQD values varied from 0 to 82 percent indicating highly variable rock quality from poor to good.

Indoor Pool

Up to 3± feet of miscellaneous granular fill exists in this area. Beneath the fill, where encountered, are medium dense coarse to fine sands with varying amounts of gravel and silt to depths of 4± to 7± feet. Underlying the sand to the top of the rock is a dense to very dense silt with varying amounts of sand and gravel. Occasional thin layers of clayey silt were found in the silt stratum.

Sandstone and shale bedrock was encountered at depths of 12± to 23± feet (elevation 1440± to 1466±), below existing grade. Sixteen and one half feet of rock coring was done in SB-23 with the percent core recovery of 48 to 100 and an RQD of 0 to 58 percent, indicating highly variable rock quality from poor to good.

Parking Garage 1

Approximately 1± to 5.5± feet of miscellaneous fill exists in this area. The fill varies from granular soil to solid waste type materials. Beneath the fill is a medium-dense to very dense coarse to fine sand with varying amounts of silt and gravel that extends to the top of rock

Sandstone and shale bedrock was encountered at depths of 5.5 to 21± feet (elevation 1463± to 1494) below existing grade. Five feet of rock coring was done in Boring SB-7 with the percent core recovery of 100 and an RQD of 0 percent indicating highly fractured rock.

Parking Garage 2

Subsurface data was obtained only in the western side of Parking Garage 2, approximately 2± to 4± feet of existing miscellaneous fill was encountered in the westernmost area of Parking Garage 2, which is underlain by dense to very dense coarse to fine sand with variable amounts of silt, gravel and occasional clayey silt.

Sandstone and shale bedrock was encountered at depths of 3.5 to 14.8± feet (elevation 1473± to 1495±) below existing grade in the westernmost building area.

Five to 10 feet of rock coring was done in B-5 and SB-22 with the percent core recovery varying from 70 to 96 and the RQD values from 0 to 48 percent indicating mostly highly fractured rock.

Retaining Walls/ Concord Road Relocation

The proposed construction will consist of the relocation of approximately 1,700 ± L.F. of Concord Road with approximately 320 feet adjacent to the existing dump as shown on Figure 1. The approximate length of the retaining walls in this area is 830± feet.

Borings B-1 thru B-6, B-14 and B-15 were drilled in the planned retaining wall area. A miscellaneous fill was encountered in all of the borings with the thickness varying from 2± to 19.5± feet. The greatest fill thickness is in the borings done in the old dump area. Below the fill is a mostly medium-dense to dense coarse to fine sand with varying amounts of silt and gravel. Sandstone/shale rock was encountered in 5 of the 8 borings at depths of 25± to 29± feet below existing grade. Rock coring was done in two of the borings with the percent core recovery varying from 16.7 to 90 and the RQD values from 6.7 to 35.8 percent indicating poor rock quality.

Dump Area

A review of the "Main Parking Dump" plan provided by JM Associates indicates the thickness of the dump to be up to 16± feet with the material consisting of a mixture of sand, silt, gravel, cans, bottles, cloth, wood, asphalt, tires, metal, carpet, refuse, concrete, etc. Several of the test pits shown on the sketch indicate that no waste was found.

Our investigation of the dump area consisted of the drilling of nine borings, three at the proposed road centerline every 100 feet, three at the right of way line every 100 feet offset from the centerline borings by 50 feet, and three borings 50 feet east of the right of way line in the middle of the dump area.

The proposed road centerline borings (B-11, B-12 and B-13) encountered a dense to very dense 2±-foot thick fill layer consisting of a brown coarse to fine sand, little silt and trace gravel. Beneath the fill, to the completion depths of the borings at 15 to 22 feet below existing grade is generally a brown coarse to fine sand, trace to some silt, trace to little gravel with occasional cobbles. Auger refusal was encountered in boring B-11 at a depth of 15 feet.

The proposed road right of way borings (B-8, B-9 and B-10) encountered a 13 to 18 foot thick fill layer consisting of a brown sand, silt and gravel with wood, ash, concrete, brick and asphalt. This fill appears to be part of the dump area. Beneath the fill, to the completion depths of the borings at 22 to 27 feet below existing grade is generally a brown coarse to fine sand, little to some silt, trace gravel with occasional cobbles and boulders.

The dump area borings (B-3, B-14 and B-15) encountered a 10 to 19± foot thick fill layer consisting of a brown sand, silt and gravel with wood, ash, concrete, brick and asphalt. Beneath the fill, to the completion depths of the borings at 17 to 29 feet below existing grade is generally a brown coarse to fine sand, little to some silt, trace gravel with occasional cobbles and boulders. Auger refusal was encountered at 29 feet in boring B-3 and two 5-foot rock cores were taken. The rock cored is gray sandstone and had core recoveries of 16.7 and 16.7 percent and RQD's of 10.4 and 6.7 percent. The low rock core recovery and RQD values indicate a highly weathered/fractured rock.

Groundwater was encountered in most of the borings in the area of the dump at depths ranging from 14 to 21.5 feet below existing grade. Some water seepage should be anticipated at higher elevations, after periods of heavy rain particularly at the soil/rock interface.

Track Area

The geoprobes, borings and test pits in this area did not encounter any existing fill except for OU-1B-17, which had 2.4 feet of fill at the ground surface and OU-1B-24 which had approximately 10 to 12± feet of fill associated with the existing UST's.

Beneath the topsoil/asphalt, where encountered is a natural medium-dense to very dense medium to fine sand, with varying amounts of silt and gravel and cobbles and boulders. This stratum extends to the completion depths of up to 50± feet at the geoprobes/test pit locations. Rock was encountered in TP-T5 and TP-T6 (at depths of 8± to 9± feet), which were excavated in the proposed detention pond areas and in the deep monitoring wells at depths ranging from 30 to 50± feet bgs.

EVALUATION AND RECOMMENDATIONS

General

Based on the borings, test pit excavations and geoprobes, this site can be considered good to excellent (except a few isolated areas) with respect to providing satisfactory support of the planned structures. No organic material other than surface vegetation, roots, and topsoil was encountered and the uncontrolled fills that were found appear to be relatively shallow, except in the old dump area and the majority of these existing fills will be excavated as part of the proposed construction.

The natural granular soils are in a mostly dense to very dense condition and will provide suitable support for conventional shallow foundations with moderate allowable bearing capacities.

Higher bearing capacities can be realized if the foundations are constructed on the underlying sandstone/shale bedrock.

The existing fills in the planned building areas may be left in place to support floor slabs and the at-grade parking areas and will provide suitable floor and pavement support after proofrolling and remediating any soft areas disclosed by the proofrolling.

General Site Preparation Procedures

In general, the site preparation procedures will consist of stripping the surface vegetation, topsoil, asphalt, and any organic material, if encountered, from within building and parking areas, and then cutting and filling the site to grade. Prior to placing any fill, the existing uncontrolled fill and natural soils should be proofrolled with a heavy vibratory roller. The proofrolling should consist of making 4 complete coverages of the area. Any soft areas disclosed should be excavated to stable material and backfilled in compacted lifts to achieve 95 percent of Modified Proctor Density (ASTM D 1557).

In areas where a minimum of 3 feet of fill is required to reach finished subgrade elevation, the existing asphalt/concrete need not be removed; however, it should be broken-up in place (maximum size 18" to 24") to allow for water drainage prior to the placement of fill. This is currently being done in the existing tennis courts on the west end of the project site.

The concrete in the existing tennis courts should be broken up and then be covered with 12"± to 18"± of fill and the area proofrolled, prior to placing additional fill.

The existing retaining wall located in the northwest corner of the planned future construction area may be left in place.

Following is a guide for the reuse of onsite materials:

Onsite Soils - The inorganic soils beneath the topsoil in cut areas may be used as structural fill; however, if these materials possess a high silt/clay content, they cannot be worked or compacted when significantly over optimum water content and, once wet, will required a long period of time to dry. The ease with which soil fills can be constructed on this site will, to a high degree, depend on the time of year in which construction takes place and the construction procedures utilized by the earthwork contractor. In order to reuse these soils, they may need to be spread out to let dry or treated with lime/cement to reduce the moisture content and make them workable.

It is the intent to reuse the existing site soils in compacted fills throughout the project. Acceptable uses for the onsite inorganic soils are as follows:

- Structural fill all areas
- General Fill in landscape areas

Visual observations and density tests should be done throughout the placement of the onsite soils to determine that they are being placed in accordance with our recommendations.

Onsite Rock - Sandstone/shale may be encountered during some excavations and will likely require a large track backhoe or hoe-ram to excavate the rock below the top one to two feet. The rock may be used in structural fills as long as it is sufficiently broken down such that a dense mass without any large or significant voids is achieved. Compaction of rock fills, if any, should be accomplished by placing the rock as thinly as possible and working it into the underlying material with a large piece of equipment such as a D-8 bulldozer. Rock fills should be constructed so that all large voids between the rock pieces are eliminated in the placement and working of the material.

The existing rock is currently being crushed onsite for reuse. Acceptable uses for the crushed rock are as follows:

- Backfill under footings
- Structural fill in all areas
- Pipe bedding and/or backfill

This material does not require density testing. After placement of the crushed rock, it should be compacted under our visual observation.

Recycled Concrete Aggregate (RCA) - We have completed geotechnical laboratory testing of the RCA samples from the stockpiles onsite. The RCA is broken into coarse grained and fine-grained RCA. The results of the laboratory testing are attached. We have also approved the use of recycled concrete that has a maximum particle size of 12± inches.

The existing RCA is currently being crushed onsite for reuse. Acceptable uses for the RCA are as follows:

- Backfill under footings
- Structural fill in all areas
- Pipe bedding and/or backfill
- Roadbase (onsite roads only)
- Slab-on-grade subbase

Visual observations and density tests should be done throughout the placement of the RCA to determine that it is being placed in accordance with our recommendations.

Fill Procedures:

Where structural fill is being placed in steep slope areas, benching into the existing slope should be done.

The fill should be placed in maximum 12-inch thick lifts, with each layer compacted to the required density using a large vibratory roller (minimum 10-ton static drum weight). Building area fills should be compacted to a minimum of 92 percent and average of 95 percent of the maximum Modified Proctor Density (ASTM D 1557).

Offsite borrow material, if required, should be well graded granular material and have a maximum particle size of 6 inches and the maximum amount of fines (percentage passing a No. 200 mesh sieve) should be 15% to help facilitate construction during wet weather. The "fines" should be non-plastic. The granular fill should be compacted using a large vibratory roller (Dynapac CA-15 or equivalent) to achieve the same density requirements as above.

Backfill in confined areas such as utility trenches and foundations within load bearing or paved areas should be placed in maximum 6-inch thick layers and compacted to a minimum of 92 percent and average of 95 percent density as described above.

As previously indicated, some of the onsite soils may contain significant percentages of silt and some clay and will readily soften during wet weather and from construction activity. Wetting or drying of the fill material should be accomplished as necessary to achieve the required density. The subgrade

should be graded to drain and tight-rolled at the end of the day, if wet weather is anticipated.

Rock fill should be placed in maximum 12-inch thick lifts, with each layer compacted to the required density using a large vibratory roller (minimum 10-ton static drum weight).

Earthwork Construction During Cold Weather Conditions

We are providing recommendations for earthwork construction during the upcoming cold weather conditions that will exist at the above referenced site during the winter months. The primary concern in dealing with frozen soils is the volume change (expansion) that occurs upon freezing and the subsequent softening and consolidation of the soils upon thawing.

In order to have a significant amount of expansion of soils during freezing conditions, it requires that the soils have more than 5 to 8± percent "fines" (material passing a No. 200 mesh sieve) and a source of water such as groundwater, wet weather or a high natural water content. The largest soil expansion during freezing occurs when there are sufficient "fines" and shallow groundwater that rises as a result of capillary action, which creates ice lenses and thus large volume changes from the freezing and thawing cycle. Natural granular soils and granular compacted fills that contain less than 5 to 8± percent "fines" do not experience a significant expansion upon freezing.

The two major types of material that will be used in construction of the compacted fills at this site will be the recycled crushed concrete and the natural silty granular soils. The recycled concrete should be relatively non-frost susceptible while the natural soil will be susceptible to significant volume changes due to freeze/thaw cycles. The following are our recommendations for earthwork construction during freezing weather conditions using these materials:

Recycled Crushed Concrete - This material should not be frost susceptible and can therefore be taken from the stockpiles, spread in 12± inch thick lifts and compacted to 95 percent of Modified Proctor Density (ASTM D 1557). There may be some clumping of particles during cold periods but these will readily break down during placement and compaction.

We also recommend that any areas to receive fill after a night of below freezing temperatures be "tracked" with a dozer prior to placing the initial lift of fill for the day.

Natural Silty Granular Soils - The natural site soils are frost susceptible and will likely experience a significant amount of frost "heave" during periods of

cold weather. The following procedures are recommended to minimize the effects of the cold weather on the construction activity:

- If there is more than 2 to 3 inches of frost in the borrow area, the frost should be stripped prior to loading the fill for use in the compacted fill construction. The stripped frozen soils can either be stockpiled for later use after thawing or spread in the deeper parking area fills (greater than 10 feet below finished grade) in a maximum layer of 6 inches. The frozen soil can also be used in planned landscape areas.
- Fill areas that contain 2 inches of frost or less need not have the frost removed prior to placing additional fill; however, the area should be "tracked" with the dozer prior to placing additional fill. If more than 2 inches of frost is present, the frozen soil should be stripped, prior to placing additional fill. The stripped frozen soils can be used as indicated previously.
- In order to reduce the amount of frost that will have to be stripped, we recommend that smaller areas of fill be done at a time (i.e., several lifts of fill in a small area each day rather than one fill lift over a large area). Additionally, if extremely cold weather is anticipated (particularly over a weekend) a layer of salt hay or mulch could be spread in the compacted fill and cut areas and then stripped prior to cutting/filling. Other alternates would include the use of insulation blankets placed on the subgrade to limit the amount of frost that develops or heated blankets/heating systems that can thaw the ground.

PAVEMENT AREAS

Prior to placing additional fill, proposed paved areas should be stripped of topsoil/asphalt and proofrolled as discussed earlier in this report.

The compaction criteria for fills in parking areas may consist of 92 percent, except in the uppermost 2 feet where 95 percent should be achieved to provide for good pavement support. Visual observations and in-place field density tests should be made to determine the adequacy of the compaction.

In on-grade parking areas where a minimum of 3 feet of fill is required to reach finished subgrade elevation, the topsoil/asphalt need not be excavated. We should inspect the subgrade prior to paving, to confirm that the estimated CBR and design pavement sections are appropriate. The site soils will have a moderate CBR value (California Bearing Ratio) on the order of 10 to 20 percent.

GENERAL FOUNDATION DESIGN CRITERIA

All footings may be placed on compacted structural fill, the dense natural granular soils and/or on the sandstone/shale bedrock. Footings founded on the compacted fill or natural granular soils may be designed for a maximum net allowable average bearing pressure of 4.5 tsf (9,000 psf). The allowable net bearing pressure can be increased to 10.0 tsf (20,000 psf) if the footings are founded on the sound sandstone/shale bedrock.

If the bottom of footing excavations becomes softened as the result of construction activity or wet weather, the soft material should be excavated and replaced with clean $\frac{3}{4}$ inch crushed stone.

All excavations greater than 5 feet in depth should have the sides sloped back to a maximum slope of 1 horizontal to 1 vertical and/or be sheeted and braced in accordance with applicable codes.

Exterior footings and those footings potentially exposed to frost action should be founded a minimum of 4.0 feet below adjacent exterior grade. Interior footings within heated areas may be founded at conventional depths below the floor slab. Footings founded on sound rock are not required to extend 4.0 feet below grade for frost protection.

The floor slab should be designed using a subgrade modulus of 175 pci, assuming that a 6-inch thick layer of granular material with a maximum particle size of 1.5 inches and a maximum percent passing the No. 200 mesh sieve of 12 percent is placed beneath the floor slab.

All retaining walls should be provided with positive drainage behind the wall to preclude hydrostatic pressures from developing.

The site soils have been classified as Site Class B and C, as indicated elsewhere in this report, for seismic design purposes in accordance with the Building Code of NY State, 2007. See the attached Table 1 for a summary of soil design parameters. Site soils classified as Site Class B will be rock or where there is less than 10 feet of soil cover over rock. Site soils classified as Site Class C will be areas that have greater than 10 feet of soil cover over rock.

After satisfactory completion of the outlined building area preparation procedures, footings and floor slabs founded on the compacted structural fill/natural soils/rock should have post-construction total settlements of less than 1-inch and maximum differential settlements in 50 feet of less than $\frac{1}{2}$ inch.

AREA SPECIFIC FOUNDATION RECOMMENDATIONS

Casino, Event Center, Ballroom & Theater Areas

The proposed finished floor grade in these areas is elevation 1495.0. Existing grades vary from a low of 1463 in the northwest portion of the building area (proposed Ballroom), to elevation 1500± in the south portion of the planned Casino area. Based on the finished floor elevation of 1495, cuts of up to 6.0± feet and fills of up to 31.0± feet will be required to achieve finished subgrade elevation.

The required cuts and fills and depth to rock are variable over the building area, but it appears that the majority of the proposed footings will be founded on compacted structural fill or the natural soils and may be designed for a net allowable average soil bearing pressure of 4.5 tsf (9,000 psf). Footing excavations in the higher-elevation areas may extend close to or into the sandstone/shale bedrock. Footings that are founded on sound bedrock may be designed for a maximum net allowable bearing pressure of 10.0 tsf (20,000 psf).

The seismic design Site Class for this area varies from B in the eastern half of the planned Casino area to C in the remainder of the building area.

In areas that require fill, the existing fill should be proofrolled, any soft areas removed and a controlled compacted structural fill placed to subgrade elevation.

Hotel Tower

The proposed finished floor grade in the hotel tower is elevation 1495.0. Existing grades vary from elevation 1470 at the western end of the building area to elevation 1486 at the eastern side of the planned hotel tower. Based on the finished floor elevation of 1495, fills of 8± to 24± feet will be required to achieve finished subgrade elevation.

The significant depth of fill and depth to bedrock makes founding the footings on rock uneconomical. Therefore, either spread footings or a mat foundation constructed on a compacted structural fill consisting of recycled crushed concrete, are the recommended foundation support systems. The spread footings/mat may be designed for a maximum average net allowable soil bearing pressure of 4.5 tsf (9,000 psf).

Any existing fill in the Hotel Tower area should be excavated to dense natural soils, prior to placing any compacted structural fill,

The seismic design Site Class for the Hotel Tower is C.

Future Hotel Tower

Existing grades in the future hotel area vary from elevation 1463± to elevation 1482 and with an assumed finished grade of elevation 1495 will require 12± to 31± feet of fill to attain finished subfloor grade. The recommendations for the presently planned hotel are appropriate for the future hotel.

Because of the close proximity to adjacent planned structures, we recommend that the building area fills be placed as part of the present phase of construction.

Indoor Pool

The proposed finished grade in the indoor pool area is 1495. Existing grades vary from elevation 1463± on the north side to elevation 1478± on the south side of the planned pool area.

Based on the finished floor elevation of 1495, fills of 16± to 31± feet will be required to attain finished subgrade elevation.

We recommend that the pool area be founded on conventional spread/strip footings on a compacted structural fill. The footings may be designed for a maximum net allowable soil bearing pressure of 4.5 tsf (9,000 psf).

The seismic design Site Class for the pool area is C.

Parking Garage 1

The proposed lowest floor grade in this building area is elevation 1488.0. Existing grades vary from elevation 1476± in the eastern building corner to elevation 1500± in the northwest building corner.

Based on the finished lowest level floor elevation of 1488, cuts of up to 13± feet and fills of up to 11± feet will be required to attain finished subgrade elevation.

The area of deeper cuts in the northern portion of the Parking Garage 1 will encounter rock above finished grade; therefore, a portion of this structure will be founded on rock and a portion on natural soils/compacted structural fill.

We recommend that Parking Garage 1 be founded on spread/strip footings founded on either rock or soil and be designed for a net allowable bearing pressure of 10.0 tsf (20,000 psf) on rock and 4.5 tsf (9,000 psf) on the natural soils/compacted structural fill.

Parking Garage 2

The proposed finished lowest floor grade in this building area is elevation 1499.5. Existing grades vary from elevation 1488± at the southeast corner to elevation 1501± at the northeast corner of the building.

Based on the finished lowest level floor elevation of 1499.5, cuts of up to 2.5± feet and fills of up to 10.5± feet will be required to reach finished subgrade elevation.

We recommend that Parking Garage 2 be founded on spread/strip footings founded on the natural soils/compacted structural fill. The footings may be designed for a maximum average net allowable soil bearing pressure of 4.5 tsf (9,000 psf). Any existing fill should be excavated and/or proofrolled, prior to placing the compacted structural fill.

The seismic design Site Class for Parking Garage 2 is C.

Racetrack Support Buildings

The proposed Clubhouse will be in a deep cut area and therefore suitable soils will be encountered at footing subgrade elevation. The planned Paddock and Maintenance Shed will be located in primarily a fill area.

We recommend that the above structures be founded on spread/strip footings founded on the natural soils/compacted structural fill and designed for a maximum net allowable soil bearing of 4.5 tsf (9,000 psf).

The seismic design Site Class for this area is C.

RETAINING WALLS/ ROADWAY RELOCATION

Based on the borings and historical information we have reviewed, the subsurface conditions in the proposed roadway relocation area can be considered fair to good with respect to providing satisfactory support of the planned roadway and retaining walls. The existing fill encountered in the dump area does not contain a significant amount of organic material and has been in place for a long period of time, resulting in a mostly medium-dense condition.

Site preparation procedures in the proposed roadway and Retaining Wall area should consist of stripping any surface vegetation, topsoil and miscellaneous surficial debris from within the roadway and retaining wall areas. Prior to placing any new fill, the existing uncontrolled fill should be proofrolled with a heavy vibratory roller such as an Ingersoll-Rand SP-100 or equivalent. The

TABLE I

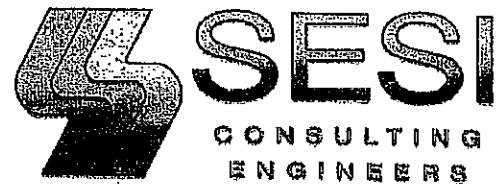
SUMMARY OF SOIL DESIGN PARAMETERS

PARAMETER	VALUE
1. Allowable Average Bearing Capacity (net)	
Natural Granular Soils/Compacted Fill	4.5 tsf
Sandstone/Shale Bedrock	10.0 tsf
2. Total Unit Weight	125 pcf
3. Angle of Internal Friction - Backfill against Structures	32 degrees
4. Earth Pressure Coefficient (See Note 1)	
Active Earth Pressure (Ka)	0.32
Earth Pressure @ Rest (Ko)	0.50
Passive Earth Pressure (Kp)	3.26
5. Coefficient of Sliding (concrete over soil)	0.40
6. Subgrade Modulus for Floor Slab Design (Granular Fill)	175 pci
7. Slopes (Above groundwater and maximum height of 15 ft)	
Maximum Cut Slope in Soil	2.0 H:IV
Maximum Fill Slope in Soil	2.0 H:IV
8. Seismic Design Criteria- Site Class (see report for specific areas)	B or C

Notes:

- 1.) A drainage medium should be installed along all retaining walls to avoid hydrostatic pressures from developing.
- 2.) Compaction equipment used within 5± feet of permanent walls should not weight more than 5,000 pounds.

APPENDIX



**BORING LOGS AND TEST PIT LOGS FROM SUBSURFACE
INVESTIGATION
10/13/00**

PROJECT NO. <u>N- 5485</u>		INSPECTED BY: <u>WSP</u>		BORING NO. <u>B-1</u>	
LOCATION <u>SEE FIGURE 1</u>		APPROX. ELEV. <u>98.8'</u>		DATE <u>9/15/00</u>	

DEPTH FT.	SAMPLES	RESISTANCE * SAMPLING	DEPTH FT.	DESCRIPTION
0		20		Dark Brown coarse to fine SAND, little Silt, trace Gravel
5		26		Brown coarse to fine SAND, some Silt, little Gravel with Rock Fragments
10		55/2"		Gray and brown decomposed SANDSTONE W.C. = 6.3% (-200) = 2.0%
				Gray Weathered SANDSTONE
15		50/1" 5 min. 4 min. 1 min. 1 min. 4 min.		Rock Core: Run 1 (15'-20') Recovery = 40"/60" = 66.7% RQD = 5"/60" = 12%
20				BORING COMPLETE AT 20 FEET
25				
30				
35				

SAMPLER: 2-INCH O.D. SPLIT BARREL 140 LB. HAMMER 30 INCH DROP * Blows/Ft.	DEPTH TO WATER: <u>10.5±</u> DATE: <u>9/15/00</u> REMARKS: <u>AT COMPLETION OF BORING</u>
--	--

Fig. 2

SESI CONSULTING ENGINEERS, PC

PROJECT NO.	N- 5485	INSPECTED BY:	WSP	BORING NO.	B-2
LOCATION	SEE FIGURE 1	APPROX. ELEV.	98.1'	DATE	9/15/00

DEPTH FT.	S A M P L E S	R E S I S T A N C E * S A M P L I N G	D E P T H F T	DESCRIPTION
0				Asphalt 2.5" Brown coarse to fine SAND, little to some Gravel, little Silt
5				Augered 6" into Gray Sandstone BORING COMPLETE AT 2 FEET
6				
7				
8				
9				
10				
11				
12				
13				
14				
15				
16				
17				
18				
19				
20				
21				
22				
23				
24				
25				
26				
27				
28				
29				
30				
31				
32				
33				
34				
35				

SAMPLER: 2-INCH O.D. SPLIT BARREL
140 LB. HAMMER 30 INCH DROP *Blows/Ft.

DEPTH TO WATER: Dry DATE: 9/15/00
REMARKS: AT COMPLETION OF BORING

PROJECT NO. <u>N- 5485</u>		INSPECTED BY: <u>WSP</u>		BORING NO. <u>B-3</u>	
LOCATION <u>SEE FIGURE 1</u>		APPROX. ELEV. <u>98.3'</u>		DATE <u>9/15/00</u>	

DEPTH FT.	SAMPLES	RESISTANCE * SAMPLING	DEPTH FT.	DESCRIPTION
0				Asphalt 2.5" Brown coarse to fine SAND, little to some Gravel, little clayey Silt
5		7		Brown clayey SILT and coarse to fine Sand, little Gravel <div style="text-align: right;">W.C. = 14.7% (-200) = 46.5%</div>
10				Weathered SANDSTONE
15		65/1"		Split Spoon Refusal at 10.1 Feet BORING COMPLETE AT 10.1 FEET
20				
25				
30				
35				

SAMPLER: 2-INCH O.D. SPLIT BARREL 140 LB. HAMMER 30 INCH DROP * Blows/Ft.	DEPTH TO WATER: <u>Dry</u> DATE: <u>9/15/00</u> REMARKS: AT COMPLETION OF BORING
--	---

Fig.

4

SESI CONSULTING ENGINEERS, PC

PROJECT NO. N-5485 INSPECTED BY: WSP BORING NO. B-4
 LOCATION SEE FIGURE 1 APPROX. ELEV. 99.3' DATE 9/15/00

DEPTH FT.	SAMPLES	RESISTANCE * SAMPLING	DEPTH FT.	DESCRIPTION
0		12		Asphalt 2.5" Probable Fill: Brown coarse to fine SAND, some Gravel, little Silt
5		65/2"		Weathered and Fractured SANDSTONE
10				Split Spoon Refusal at 5.2 Feet BORING COMPLETE AT 5.2 FEET
15				
20				
25				
30				
35				
SAMPLER: 2-INCH O.D. SPLIT BARREL 140 LB. HAMMER 30 INCH DROP * Blows/Ft.				DEPTH TO WATER: <u>Dry</u> DATE: <u>9/15/00</u> REMARKS: AT COMPLETION OF BORING

PROJECT NO. <u>N-5485</u>		INSPECTED BY: <u>WSP</u>		BORING NO. <u>B-5</u>
LOCATION <u>SEE FIGURE 1</u>		APPROX. ELEV. <u>99.0'</u>	DATE <u>9/15/00</u>	

DEPTH FT.	SAMPLES	RESISTANCE * SAMPLING	DEPTH FT.	DESCRIPTION
0	19			Asphalt 2.5"
				FILL: Brown Sand, Silt, Gravel, trace Clay
				Brown clayey SILT and coarse to fine Sand, little Gravel
5				Weathered SANDSTONE
				Auger Refusal at 4.6 Feet BORING COMPLETE AT 4.6 FEET
10				
15				
20				
25				
30				
35				

SAMPLER: 2-INCH O.D. SPLIT BARREL 140 LB. HAMMER 30 INCH DROP * Blows/Ft.	DEPTH TO WATER: <u>Dry</u> DATE: <u>9/15/00</u> REMARKS: AT COMPLETION OF BORING
--	---

PROJECT NO. <u>N- 5485</u>		INSPECTED BY: <u>WSP</u>		BORING NO. <u>B-6</u>	
LOCATION <u>SEE FIGURE 1</u>		APPROX. ELEV. <u>79.0'</u>		DATE <u>9/15/00</u>	

DEPTH FT.	SAMPLES	* RESISTANCE SAMPLES INCH	DEPTH FT.	DESCRIPTION
0		50		FILL: Brown Sand, Silt, Gravel, Cobble and Occasional Construction Debris
5		37		Gray and Brown coarse to fine SAND, some Gravel, little clayey Silt
10		88/7"		Weathered SANDSTONE
15				Split Spoon Refusal at 10.7 Feet BORING COMPLETE AT 10.7 FEET
20				
25				
30				
35				

SAMPLER: 2-INCH O.D. SPLIT BARREL 140 LB. HAMMER 30 INCH DROP * Blows/Ft.	DEPTH TO WATER: <u>Dry</u> DATE: <u>9/15/00</u> REMARKS: <u>AT COMPLETION OF BORING</u>
--	--

Fig. 7

SESI CONSULTING ENGINEERS, PC

PROJECT NO. N- 5485INSPECTED BY WSP

TEST PIT NO.

TP-1

LOCATION See Figure 1APPROX. ELEV. 99.0±WATER OBSERVATION NOT ENCOUNTEREDDATE EXCAVATED 9/15/00

DEPTH FT.	DESCRIPTION / SOIL CLASSIFICATION	RELATIVE DENSITY OR CONSISTENCY
0— —	Asphalt 2.5"- 3.0"	
1— —	Brown coarse to fine SAND, some Gravel, little Silt	
2— —	Rock at 2.0 to 3.0 Feet	
3— —	TEST PIT COMPLETE AT 2.0 TO 3.0 FEET	
4— —		
5— —		
6— —		
7— —		
8— —		
9— —		
10— —		
11— —		
12— —		
13— —		
14— —		

NOTE:

SESI CONSULTING ENGINEERS, PC

Fig. 8

PROJECT NO. <u>N- 5485</u>	INSPECTED BY <u>WSP</u>	TEST PIT NO. <u>TP-2</u>
LOCATION <u>See Figure 1</u>	APPROX. ELEV. <u>89.0</u>	
WATER OBSERVATION <u>NOT ENCOUNTERED</u>		DATE EXCAVATED <u>9/15/00</u>

DEPTH FT.	DESCRIPTION / SOIL CLASSIFICATION	RELATIVE DENSITY OR CONSISTENCY
0 — —	Asphalt 2.5"	
1 — —	FILL: Brown coarse to fine SAND, some Gravel, little Silt with occasional 1" layers of asphalt	
2 — —		
3 — —	Gray- brown coarse to fine SAND, little to some Gravel, little Silt	Medium Dense
4 — —		to Dense
5 — —		
6 — —		
7 — —		
8 — —		
9 — —		
10 — —	TEST PIT COMPLETE AT 9.83 FEET	
11 — —		
12 — —		
13 — —		
14 — —		

NOTE:

SESI CONSULTING ENGINEERS, PC

PROJECT NO. <u>N-5485</u>		INSPECTED BY <u>WSP</u>	TEST PIT NO. <u>TP-3</u>
LOCATION <u>See Figure 1</u>		APPROX. ELEV. <u>93.0</u>	
WATER OBSERVATION <u>NOT ENCOUNTERED</u>		DATE EXCAVATED <u>9/15/00</u>	

DEPTH FT.	DESCRIPTION / SOIL CLASSIFICATION	RELATIVE DENSITY OR CONSISTENCY
0— —	TOPSOIL 8"	
1— —	Probable FILL: Gray- brown coarse to fine SAND, little Silt, trace Gravel	
2— —		
3— —	Gray- brown coarse to fine SAND, some Gravel, little Silt	Medium Dense to Dense
4— —		
5— —		
6— —		
7— —		
8— —		
9— —	----Gray SANDSTONE	
10— —	TEST PIT COMPLETE AT 9.33 FEET	
11— —		
12— —		
13— —		
14— —		

NOTE:

SESI CONSULTING ENGINEERS, PC

PROJECT NO. <u>N- 5485</u>	INSPECTED BY <u>WSP</u>	TEST PIT NO. <u>TP-4</u>
LOCATION <u>See Figure 1</u>	APPROX. ELEV. <u>78.0</u>	
WATER OBSERVATION <u>NOT ENCOUNTERED</u>		DATE EXCAVATED <u>9/15/00</u>

DEPTH FT.	DESCRIPTION / SOIL CLASSIFICATION	RELATIVE DENSITY OR CONSISTENCY
0— —	FILL: Gray- brown coarse to fine SAND, some Gravel, little Silt with occasional Cobbles- boulders, concrete, metal, brick	
1— —		
2— —		
3— —		
4— —		
5— —	Gray- brown coarse to fine SAND, some Gravel, little Silt ----Gray SANDSTONE	Medium Dense to Dense
6— —		
7— —		
8— —	TEST PIT COMPLETE AT 7.5 FEET	
9— —		
10— —		
11— —		
12— —		
13— —		
14— —		

NOTE:

SESI CONSULTING ENGINEERS, PC

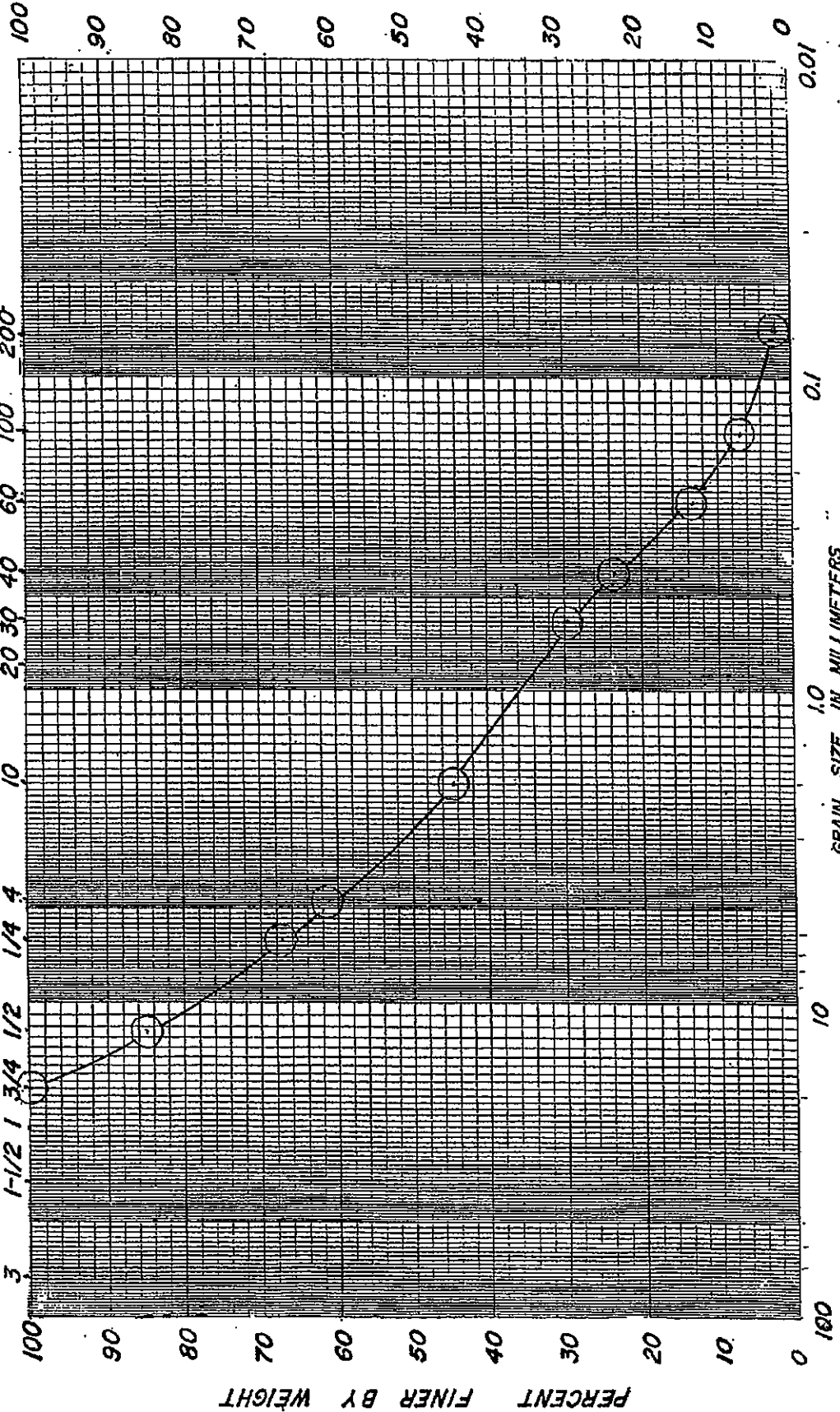
GRAIN SIZE DISTRIBUTION

GRAVEL		SAND		SILT OR CLAY	
Coarse	Fine	Coarse	Fine	Silt	Clay

U.S. STANDARD SIEVE SIZE

3 1-1/2 1 3/4 1/2 1/4 4 10 20 30 40 60 100 200

Trace little some and and some little trace

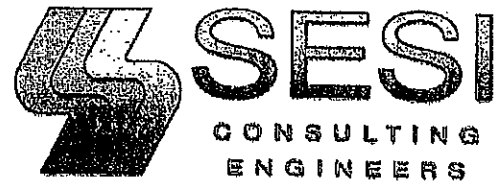


PERCENT FINER BY WEIGHT

GRAIN SIZE IN MILLIMETERS

FIG. 12

Project No. N-5485 Date 10/12/00 Tested by MSF Checked by WSP Sample No. B-1/s-3
Visual ID. (Mod: Burmister) Gray + brown medium to fine GRAVEL and coarse to fine Sand, trace Silt



**BORING LOGS FROM SUBSURFACE INVESTIGATION
2/10/2003**

PROJECT NO. <u>N-5485</u>		INSPECTED BY: <u>MM</u>		BORING NO. <u>B-1</u>
LOCATION <u>SEE FIGURE 1</u>		APPROX. ELEV. <u>1476±</u>	DATE <u>7/16/2002</u>	

DEPTH FT.	SAMPLES	RESISTANCE * SAMPLING IN G E	DEPTH FT.	DESCRIPTION
0		26		FILL: Brown coarse to fine SAND, little Silt, trace Gravel W.C.=3.6%
5		13		Brown coarse to fine SAND, little Silt, trace Gravel with Cobbles
10		26		
15		20		Brown coarse to fine SAND, some Silt, trace Gravel
20		50/0"		with Cobbles and Boulders
25		48		
30		82		
35				BORING COMPLETE AT 32 FEET

SAMPLER: 2-INCH O.D. SPLIT BARREL 140 LB. HAMMER 30 INCH DROP * Blows/Ft.	DEPTH TO WATER: <u>20 ft. ±</u> DATE: <u>7/16/2002</u> REMARKS: <u>AT COMPLETION OF BORING</u>
--	---

PROJECT NO. <u>N-5485</u>		INSPECTED BY: <u>MM</u>		BORING NO. <u>B-2</u>
LOCATION <u>SEE FIGURE 1</u>		APPROX. ELEV. <u>1474±</u>	DATE <u>7/16/2002</u>	

DEPTH FT.	SAMPLES	RESISTANCE * SAMPLING INCHES	DEPTH FT.	DESCRIPTION
0		12		FILL: Brown coarse to fine SAND, little Silt, trace Gravel with occasional Cobbles
5		50/0"		
10		9		Brown coarse to fine SAND, little to some Silt, trace Gravel with occasional Cobbles and Boulders
15		30		
20		45		Brown coarse to fine SAND, some Silt, little Gravel
25		50/0"		with Cobbles and Boulders
30				AUGER REFUSAL AT 26.5 FEET BORING COMPLETE AT 26.5 FEET
35				

SAMPLER: 2-INCH O.D. SPLIT BARREL 140 LB. HAMMER 30 INCH DROP * Blows/Ft.	DEPTH TO WATER: <u>20 ft. ±</u> DATE: <u>7/16/2002</u> REMARKS: AT COMPLETION OF BORING
--	--

PROJECT NO. <u>N-5485</u>		INSPECTED BY: <u>MM</u>		BORING NO. <u>B-3</u>
LOCATION <u>SEE FIGURE 1</u>		APPROX. ELEV. <u>1467+</u>	DATE <u>7/15/2002</u>	

DEPTH FT.	SAMPLES	RESISTANCE * SAMP L I N G C E	DEPTH FT.	DESCRIPTION
0	■	58		FILL: Brown medium to fine GRAVEL, little coarse to fine Sand, trace Silt
5		6		FILL: Brown coarse to fine SAND, little to some Silt, trace Gravel with Cobbles
10	■	15		
15		14		FILL: Brown coarse to fine SAND, some Silt, trace Gravel with Cinders and Ash
20	■	8		Dark Brown coarse to fine SAND, some Silt with organic Brown coarse to fine SAND, some Silt, trace Gravel with organic
25		61		Brown coarse to fine SAND, some Silt, trace Gravel with Cobble
30	■	3 min. 4 min. 4 min. 3 min. 4 min.		Rock Core #1 Recovery = 10"/60" = 16.7% RQD = 6.25"/60" = 10.4%
35				

SAMPLER: 2-INCH O.D. SPLIT BARREL 140 LB. HAMMER 30 INCH DROP * Blows/Ft.	DEPTH TO WATER: <u>19.5 ft. ±</u> DATE: <u>7/15/2002</u> REMARKS: <u>AT COMPLETION OF BORING</u>
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Fig. 4 SESI CONSULTING ENGINEERS PC

PROJECT NO. <u>N-5485</u>		INSPECTED BY: <u>MM</u>		BORING NO.	B-3(cont.)
LOCATION <u>SEE FIGURE 1</u>		APPROX. ELEV. <u>1467±</u>		DATE <u>7/15/2002</u>	

DEPTH F T	S A M P L E S	* R E S I S T A N C E	D E P T H F T	DESCRIPTION
30 35 40 45 50 55 60 65		4 min. 6 min. 6 min. 5 min. 5 min.		Rock Core #2 Recovery = $10"/60" = 16.7\%$ RQD = $4"/60" = 6.7\%$
				BORING COMPLETE AT 39 FEET

SAMPLER: 2-INCH O.D. SPLIT BARREL 140 LB. HAMMER 30 INCH DROP * Blows/Ft.	DEPTH TO WATER: <u>19.5 ft. ±</u> DATE: <u>7/15/2002</u> REMARKS: <u>AT COMPLETION OF BORING</u>
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PROJECT NO. <u>N-5485</u>		INSPECTED BY: <u>MM</u>		BORING NO. <u>B-4</u>
LOCATION <u>SEE FIGURE 1</u>		APPROX. ELEV. <u>1468+</u>	DATE <u>7/16/2002</u>	

DEPTH FT.	SAMPLES	RESISTANCE * SAMPLING	DEPTH FT.	DESCRIPTION
0		15		FILL: Brown coarse to fine SAND, little Silt, trace Gravel W.C.=1.5%
5		14		Brown coarse to fine SAND, little Silt, trace Gravel, trace wood
10		8		
15		13		Brown coarse to fine SAND, little to some Silt, trace Gravel
20		28		Brown coarse to fine SAND, some Silt, little Gravel
25		50/1"		AUGER REFUSAL AT 25.5 FEET BORING COMPLETE AT 25.5 FEET
30				
35				

SAMPLER: 2-INCH O.D. SPLIT BARREL 140 LB. HAMMER 30 INCH DROP * Blows/Ft.	DEPTH TO WATER: <u>18.5 ft. ±</u> DATE: <u>7/16/2002</u> REMARKS: AT COMPLETION OF BORING
--	--

PROJECT NO. <u>N-5485</u>		INSPECTED BY: <u>MM</u>		BORING NO. <u>B-5</u>
LOCATION <u>SEE FIGURE 1</u>		APPROX. ELEV. <u>1471±</u>		DATE <u>7/10/2002</u>

DEPTH FT.	S A M P L E S	RESISTANCE * S A M P L I N G C E	DEPTH FT.	DESCRIPTION
0		70		FILL: Brown coarse to fine SAND, little Silt, trace Gravel with occasional Cobbles
5		7		Brown coarse to fine SAND, little to some Silt, trace Gravel
10		52		with occasional Cobbles
15		50/4"		with Boulders
20		52		Brown coarse to fine SAND, little Silt, trace Gravel with Cobble
25		50/2" 5 min 5 min 6 min 5 min 6 min		with Shale Rock Core Recovery = 54"/60" = 90% RQD = 21.5"/60" = 35.8%
30				AUGER REFUSAL AT 25.5 FEET BORING COMPLETE AT 30.5 FEET
35				

SAMPLER: 2-INCH O.D. SPLIT BARREL 140 LB. HAMMER 30 INCH DROP * Blows/Ft.	DEPTH TO WATER: <u>18 ft. ±</u> DATE: <u>7/10/2002</u> REMARKS: <u>AT COMPLETION OF BORING</u>
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Fig. 6 SESI CONSULTING ENGINEERS PC

PROJECT NO. <u>N-5485</u>		INSPECTED BY: <u>MM</u>		BORING NO. <u>B-6</u>
LOCATION <u>SEE FIGURE 1</u>		APPROX. ELEV. <u>1475±</u>	DATE <u>7/10/2002</u>	

DEPTH FT.	SAMPLES	RESISTANCE * SAMP LING N G C E	DEPTH FT.	DESCRIPTION
0	■	19		FILL: Brown coarse to fine SAND, some Silt, little Gravel
5	■	7		Brown coarse to fine SAND, some Silt, little Gravel
10	■	22		
15	■	56		Red/Brown coarse to fine SAND, some Silt, trace Gravel with Weatherd Rock
20	■	50/3"		Brown coarse to fine SAND, some Silt, little Gravel with Weathered Rock with occasional Cobbles
25	■	50/4"		AUGER REFUSAL AT 26 FEET BORING COMPLETE AT 26 FEET
30				
35				

SAMPLER: 2-INCH O.D. SPLIT BARREL 140 LB. HAMMER 30 INCH DROP * Blows/Ft.	DEPTH TO WATER: <u>18 ft. ±</u> DATE: <u>7/10/2002</u> REMARKS: <u>AT COMPLETION OF BORING</u>
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PROJECT NO. <u>N-5485</u>		INSPECTED BY: <u>MM</u>		BORING NO. <u>B-7</u>	
LOCATION <u>SEE FIGURE 1</u>		APPROX. ELEV. <u>1475±</u>		DATE <u>7/10/2002</u>	

DEPTH FT.	SAMPLES	RESISTANCE * SAMPLING	DEPTH FT.	DESCRIPTION
0		35		FILL: Brown coarse to fine SAND, little Silt, trace Gravel
5		119		Red/Brown coarse to fine SAND, little Silt, trace Gravel with Weathered/Fractured Rock
10		83		
15		23		with occasional Cobbles
20		73		Red/Brown coarse to fine SAND, some Silt, little Gravel with Weathered Rock
25		4 min. 6 min. 6 min. 7 min. 7min.		Rock Core Recovery = $40"/60" = 66.7\%$ RQD = $35.75"/60" = 59.6\%$
30				BORING COMPLETE AT 30 FEET
35				

SAMPLER: 2-INCH O.D. SPLIT BARREL 140 LB. HAMMER 30 INCH DROP * Blows/Ft.	DEPTH TO WATER: <u>19 ft. ±</u> DATE: <u>7/10/2002</u> REMARKS: AT COMPLETION OF BORING
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Fig. 8 SESI CONSULTING ENGINEERS PC

PROJECT NO.		N-5485		INSPECTED BY:		RH		BORING NO.		B-8	
LOCATION		SEE FIGURE 1		APPROX. ELEV.		1471±		DATE		7/12/2002	

D E P T H F T	S A M P L E S	R E S I S T A N C E	D E P T H F T	D E S C R I P T I O N
0		53		FILL: Brown coarse to fine SAND, trace Silt, trace Gravel
5		20		
10		18		
15		16		with Brick and Asphalt
20		12		Brown coarse to fine SAND and Silt, trace Gravel
25				BORING COMPLETE AT 22 FEET
30				
35				

SAMPLER: 2-INCH O.D. SPLIT BARREL 140 LB. HAMMER 30 INCH DROP * Blows/Ft.	DEPTH TO WATER: 17 ft ± DATE: 7/12/2002 REMARKS: AT COMPLETION OF BORING
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Fig. 9 SESI CONSULTING ENGINEERS PC

PROJECT NO. <u>N-5485</u>		INSPECTED BY: <u>RH</u>		BORING NO. <u>B-9</u>	
LOCATION <u>SEE FIGURE 1</u>		APPROX. ELEV. <u>1471±</u>		DATE <u>7/12/2002</u>	

DEPTH FT.	SAMPLES	RESISTANCE * SAMPLING INCH	DEPTH FT.	DESCRIPTION
0	█	22		FILL: Brown coarse to fine SAND, little Silt, trace Gravel
5	█	12		
10	█	11		with Decomposed Wood
15	█	41		Gray/Brown coarse to fine SAND, little Silt, trace Gravel with Weathered Shale
20	█	15		Brown coarse to fine SAND and clayey Silt
25				BORING COMPLETE AT 22 FEET
30				
35				

SAMPLER: 2-INCH O.D. SPLIT BARREL 140 LB. HAMMER 30 INCH DROP * Blows/Ft.	DEPTH TO WATER: <u>16 ±</u> DATE: <u>7/12/2002</u> REMARKS: <u>AT COMPLETION OF BORING</u>
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PROJECT NO. <u>N-5485</u>		INSPECTED BY: <u>MM</u>		BORING NO. <u>15-10</u>	
LOCATION <u>SEE FIGURE 1</u>		APPROX. ELEV. <u>1469±</u>		DATE <u>7/11/2002</u>	

DEPTH FT.	SAMPLES	RESISTANCE	DEPTH FT.	DESCRIPTION
0		57		FILL: Brown coarse to fine SAND, little Silt, trace Gravel
5		7		
10		5		with Wood, Ash, and Concrete
15		40		Brown/Black coarse to fine SAND, some Silt, trace Gravel, trace wood
20		50/2"		with Boulders
25		79		Brown coarse to fine SAND, little Silt, trace Gravel
30				BORING COMPLETE AT 27 FEET
35				

SAMPLER: 2-INCH O.D. SPLIT BARREL 140 LB. HAMMER 30 INCH DROP * Blows/Ft.	DEPTH TO WATER: <u>14 ft. ±</u> DATE: <u>7/11/2002</u> REMARKS: AT COMPLETION OF BORING
--	--

PROJECT NO. <u>N-5485</u>		INSPECTED BY: <u>RH</u>		BORING NO. <u>8-1</u>	
LOCATION <u>SEE FIGURE 1</u>		APPROX. ELEV. <u>1471±</u>		DATE <u>7/12/2002</u>	

D E P T H F T .	S A M P L E S	R E S I S T A N C E	D E P T H F T	DESCRIPTION
0	█	30		FILL: Brown coarse to fine SAND, little Silt, trace Gravel
5	█	15		Brown medium to fine SAND, some Silt, trace organic clayey Silt with occasional Cobble
10	█	10		with Grey Shale
15				AUGER REFUSAL AT 15 FEET BORING COMPLETE AT 15 FEET
20				
25				
30				
35				

SAMPLER: 2-INCH O.D. SPLIT BARREL 140 LB. HAMMER 30 INCH DROP * Blows/Ft.	DEPTH TO WATER: <u> </u> DATE: <u>7/12/2002</u> REMARKS: <u>AT COMPLETION OF BORING</u>
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PROJECT NO. <u>N-5485</u>		INSPECTED BY: <u>RH</u>		BORING NO. <u>B-12</u>	
LOCATION <u>SEE FIGURE 1</u>		APPROX. ELEV. <u>1471±</u>		DATE <u>7/12/2002</u>	

DEPTH F T	S A M P L E S	R E S I S T A N C E	D E P T H F T	DESCRIPTION
0		40		FILL: Brown coarse to fine SAND, little Gravel, little Silt W.C.=9.2%
5		21		Red/Brown coarse to fine SAND, little Silt, trace Gravel with occational Cobbles
10		7		
15		3		
20		22		Brown coarse to fine SAND, some clayey Silt, trace Gravel
25				BORING COMPLETE AT 22 FEET
30				
35				

SAMPLER: 2-INCH O.D. SPLIT BARREL 140 LB. HAMMER 30 INCH DROP * Blows/Ft.	DEPTH TO WATER: <u>15.5 ft. ±</u> DATE: <u>7/12/2002</u> REMARKS: AT COMPLETION OF BORING
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PROJECT NO.		N-5485		INSPECTED BY:		RH		BORING NO.		B-73	
LOCATION		SEE FIGURE 1		APPROX. ELEV.		1473+		DATE		7/12/2002	

DEPTH FT.	SAMPLES	RESISTANCE SAMPLE INCH	DEPTH FT.	DESCRIPTION
0		52		FILL: Brown coarse to fine SAND, trace Silt, trace Gravel
5		19		Brown coarse to fine SAND, trace Silt, trace Gravel
10		11		with occasional Cobbles
15		22		
20		32		Brown medium to fine SAND, some clayey Silt
25				BORING COMPLETE AT 22 FEET
30				
35				

SAMPLER: 2-INCH O.D. SPLIT BARREL 140 LB. HAMMER 30 INCH DROP * Blows/Ft.	DEPTH TO WATER: 21.5 ft. ± DATE: 7/12/2002 REMARKS: AT COMPLETION OF BORING
--	---

PROJECT NO. <u>N-5485</u>		INSPECTED BY: <u>MM</u>		BORING NO. <u>B-14</u>
LOCATION <u>SEE FIGURE 1</u>		APPROX. ELEV. <u>1466±</u>	DATE <u>7/16/2002</u>	

DEPTH FT.	SAMPLES	RESISTANCE	DEPTH FT.	DESCRIPTION
0	█	28		FILL: Brown coarse to fine SAND, little Silt, little Gravel
5	█	33		FILL: Brown coarse to fine SAND, little Silt, trace Gravel, trace Ash with Cobbles, Boulders, and Asphalt
10	█	29		
15	█	11		
20	█	16		Brown coarse to fine SAND, little Silt, little Gravel
25				BORING COMPLETE AT 22 FEET
30				
35				

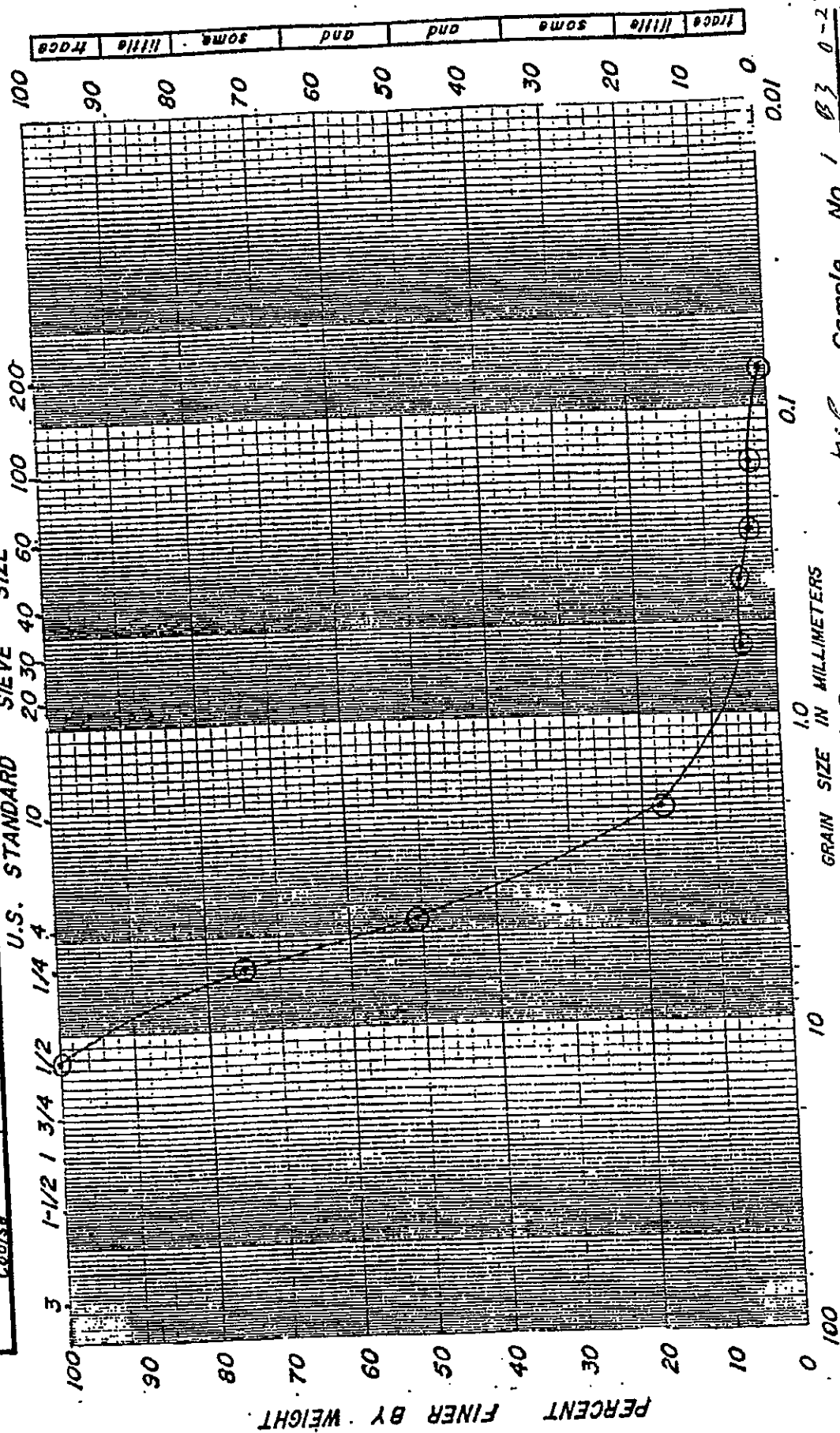
SAMPLER: 2-INCH O.D. SPLIT BARREL 140 LB. HAMMER 30 INCH DROP * Blows/Ft.	DEPTH TO WATER: <u> </u> DATE: <u>7/16/2002</u> REMARKS: AT COMPLETION OF BORING
--	--

PROJECT NO. <u>N-5485</u>		INSPECTED BY: <u>MM</u>		BORING NO. <u>B-15</u>	
LOCATION <u>SEE FIGURE 1</u>		APPROX. ELEV. <u>1466+</u>		DATE <u>7/16/2002</u>	

DEPTH FT	SAMPLES	RESISTANCE * SAMPLING INCHES	DEPTH FT	DESCRIPTION
0		77/8"		FILL: Brown coarse to fine SAND, little Silt, trace Gravel
5		21		with occasional Cobbles
10		23		Brown coarse to fine SAND, some Silt, trace Gravel with occasional Cobble
15		16		Brown coarse to fine SAND, little Silt, trace Gravel with occasional Cobble
20				BORING COMPLETE AT 17 FEET
25				
30				
35				

SAMPLER: 2-INCH O.D. SPLIT BARREL 140 LB. HAMMER 30 INCH DROP * Blows/Ft.	DEPTH TO WATER: _____ DATE: <u>7/16/2002</u> REMARKS: <u>AT COMPLETION OF BORING</u>
--	---

Fig. 16 SESI CONSULTING ENGINEERS PC

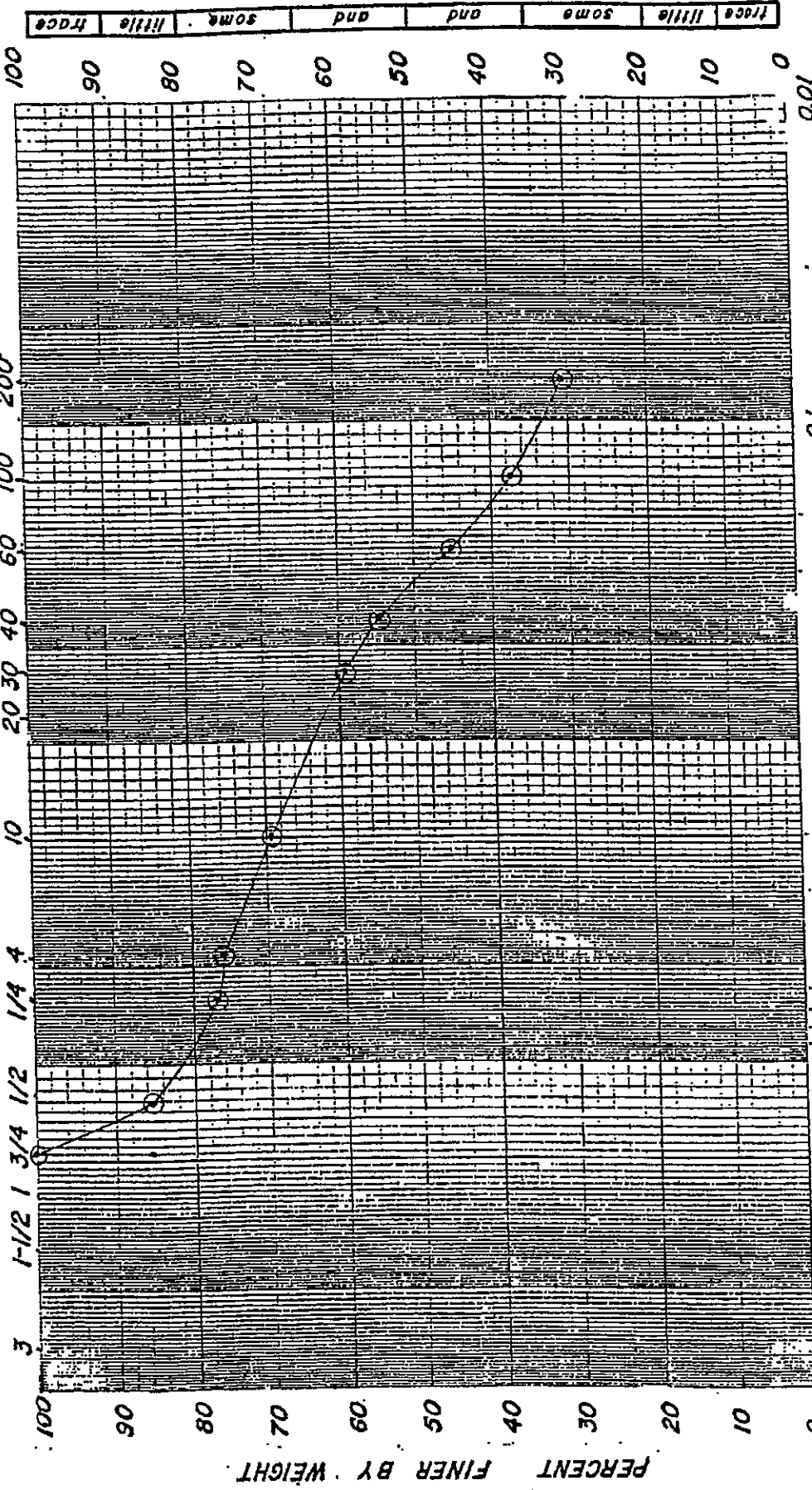
[illegible]

	0	100	10	1.0
			GRAIN SIZE IN MILLIMETERS	
Project No.	6485	Date	7/19/02	
Visual ID.	(Mod. Burmister)			
		Tested by	LSF	Sample No.
		Brown medium to fine	RAVELL	little coarse to fine sand, too F1

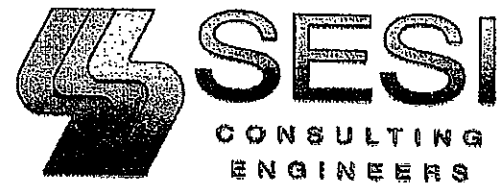
GRAIN SIZE DISTRIBUTION

GRAVEL		SAND		SILT OR CLAY	
Coarse	Fine	Coarse	Fine	Silt	Clay

U.S. STANDARD SIEVE SIZE



Project No. 5485 Date 7/19/62 Tested by kp Checked by HSP Sample No. 1 B6 0-2
 Visual ID. (Mod. Burmister) Brown, medium to fine Sand, some medium to fine gravel, some silt FIG



**TEST PIT LOGS FOR PROPOSED TRACK AREA
(FOR STORMWATER PURPOSES)
4/14/08**

PROJECT NO.	N-5500	PROJECT	Monticello, NY	TEST PIT NO.	TP-T1
LOCATION	SEE FIGURE 1	APPROX. ELEV.	1480' ±	INSPECTED BY	JZ
WATER OBSERVATION	Seepage @ 4.0 Feet			DATE EXCAVATED	4/14/2008

DEPTH FT.	DESCRIPTION / SOIL CLASSIFICATION	RELATIVE DENSITY OR CONSISTENCY
0 —	6" Topsoil/ Grass/ Roots	
1 —		
2 —	Red-brown coarse to fine SAND, some Silt, little coarse to fine Gravel	Medium Dense
3 —Large Boulders	
4 —		
5 —		to
6 —		
7 —		
8 —		
9 —with Cobble and Boulders	Dense
10 —	TEST PIT COMPLETE AT 9.5 FEET	
11 —		
12 —		
13 —		
14 —		

NOTE:

SESI CONSULTING ENGINEERS, PC

PROJECT NO. <u>N-5500</u>		PROJECT <u>Monticello, NY</u>	TEST PIT NO.	TP-T2
LOCATION <u>SEE FIGURE 1</u>		APPROX. ELEV. <u>1502' ±</u>	INSPECTED BY	JZ
WATER OBSERVATION <u>Seepage @ 3.0 Feet</u>		DATE EXCAVATED <u>4/14/2008</u>		

DEPTH FT.	DESCRIPTION / SOIL CLASSIFICATION	RELATIVE DENSITY OR CONSISTENCY
0	Topsoil/ Grass/ Roots	
1		
2	FILL: Brown medium to fine SAND, little medium to fine Gravel, little Silt	Loose
3	FILL: Red-brown coarse to fine SAND and Silt, little medium to fine Gravel	Loose
4		
5	Red-brown coarse to fine SAND, some Silt, little medium to fine Gravel ...with Cobbles	Medium Dense
6		
7		to
8		
9with Cobbles and Boulders	Dense
10		
11	TEST PIT COMPLETE AT 9.5 FEET	
12		
13		
14		

NOTE:

SESI CONSULTING ENGINEERS, PC

PROJECT NO. <u>N-5500</u>	PROJECT <u>Monticello, NY</u>	TEST PIT NO. <u>TP-T3</u>
LOCATION <u>SEE FIGURE 1</u>	APPROX. ELEV. <u>1507' ±</u>	INSPECTED BY <u>JZ</u>
WATER OBSERVATION <u>Seepage @ 3.0 Feet</u>		DATE EXCAVATED <u>4/14/2008</u>

DEPTH FT.	DESCRIPTION / SOIL CLASSIFICATION	RELATIVE DENSITY OR CONSISTENCY
0 —	6" Topsoil/ Grass/ Roots	
1 —	Red-brown medium to fine SAND, some Silt, little medium to fine Gravel	Medium Dense
2 —		
3 —		
4 —		
5 —		
6 —with Cobbles	to Dense
7 —with Cobbles and Boulders	
8 —		
9 —		
10 —		
11 —	TEST PIT COMPLETE AT 9.0 FEET	
12 —		
13 —		
14 —		

NOTE:

SESI CONSULTING ENGINEERS, PC

Fig. 4

PROJECT NO. <u>N-5500</u>	PROJECT <u>Monticello, NY</u>	TEST PIT NO. <u>TP-T4</u>
LOCATION <u>SEE FIGURE 1</u>	APPROX. ELEV. <u>1513' ±</u>	INSPECTED BY <u>JZ</u>
WATER OBSERVATION <u>Seepage @ 1.5 Feet</u>		DATE EXCAVATED <u>4/14/2008</u>

DEPTH FT.	DESCRIPTION / SOIL CLASSIFICATION	RELATIVE DENSITY OR CONSISTENCY
0 —	6" Topsoil/ Grass/ Roots	
1 —		
2 —	Red-brown medium to fine SAND, some Silt, little medium to fine Gravel	Dense
3 —with Cobbles	
4 —		to
5 —with Cobbles and Boulders	Very Dense
6 —		
7 —		
8 —		
9 —		
10 —	-----	
11 —	TEST PIT COMPLETE AT 9.5 FEET	
12 —		
13 —		
14 —		

NOTE:

SESI CONSULTING ENGINEERS, PC

Fig. 5

PROJECT NO. <u>N-5500</u>	PROJECT <u>Monticello, NY</u>	TEST PIT NO. <u>TP-T5</u>
LOCATION <u>SEE FIGURE 1</u>	APPROX. ELEV. <u>1510' ±</u>	INSPECTED BY <u>JZ</u>
WATER OBSERVATION <u>Seepage @ 1.5 Feet</u>		DATE EXCAVATED <u>4/14/2008</u>

DEPTH FT.	DESCRIPTION / SOIL CLASSIFICATION	RELATIVE DENSITY OR CONSISTENCY
0 —	Topsoil/ Grass/ Roots	Soft
1 —		
2 —		
3 —	Red-brown medium to fine SAND, some Silt, little medium to fine Gravel	Dense
4 —		
5 —		
6 —		
7 —with Cobbles and Boulders	
8 —		
9 —	Refusal on Rock TEST PIT COMPLETE AT 8.0 FEET	
10 —		
11 —		
12 —		
13 —		
14 —		

NOTE:

SESI CONSULTING ENGINEERS, PC

PROJECT NO. <u>N-5500</u>		PROJECT <u>Monticello, NY</u>	TEST PIT NO. <u>TP-T6</u>
LOCATION <u>SEE FIGURE 1</u>		APPROX. ELEV. <u>1489' ±</u>	INSPECTED BY <u>JZ</u>
WATER OBSERVATION <u>Seepage @ 2.0 Feet</u>		DATE EXCAVATED <u>4/14/2008</u>	

DEPTH FT.	DESCRIPTION / SOIL CLASSIFICATION	RELATIVE DENSITY OR CONSISTENCY
0 — — —	Topsoil/ Grass/ Roots	Soft
1 — — —	Red-brown coarse to fine SAND, some Silt, little medium to fine Gravel with Cobbles and Boulders	Dense to Very Dense
2 — — —		
3 — — —		
4 — — —		
5 — — —		
6 — — —		
7 — — —		
8 — — —		
9 — — —	Refusal on Rock TEST PIT COMPLETE AT 9.0 FEET	
10 — — —		
11 — — —		
12 — — —		
13 — — —		
14 — — —		

NOTE:

SESI CONSULTING ENGINEERS, PC

PROJECT NO.	N-5500	PROJECT	Monticello, NY	TEST PIT NO.	TP-T7
LOCATION	SEE FIGURE 1	APPROX. ELEV.	1492' ±	INSPECTED BY	JZ
WATER OBSERVATION	Seepage @ 3.0 Feet			DATE EXCAVATED	4/14/2008

DEPTH FT.	DESCRIPTION / SOIL CLASSIFICATION	RELATIVE DENSITY OR CONSISTENCY
0 —	Topsoil/ Grass/ Roots	Soft
1 —	Red-brown medium to fine SAND, some Silt, some medium to fine Gravel with Cobbles and Boulders	Medium Dense to Dense
2 —		
3 —		
4 —		
5 —		
6 —		
7 —		
8 —		
9 —	Refusal on Rock TEST PIT COMPLETE AT 9.0 FEET	
10 —		
11 —		
12 —		
13 —		
14 —		

NOTE:

SESI CONSULTING ENGINEERS, PC



FIELD TEST PIT LOG

TEST PIT NO.: RT-TP-1
 SHEET 1 OF 1
 DATE: START: 10/3/08
 END: 10/3/08
 O.G. ELEV.: Approx. 1545

PROJECT NAME: Concord LOCATION: Race Track

JOB NO.: 5485 CLIENT: Cappelli-Concord

INSPECTOR: CDM CONTRACTOR: Nacirema

EQUIPMENT: Backhoe-365C

WATER LEVEL DEPTH: N/A NOT ENCOUNTERED: X

DEPTH (FT)	SAMPLE NO./ TYPE	SOIL CLASSIFICATION	DESCRIPTION	REMARKS
0			Red-Brown medium to fine Sand, some Silt, little coarse to fine Gravel	
			With Gray coarse grained sandstone cobbles from 5-ft. to 20-ft.	
			(Mottled with Orange medium to fine Sand from 1-ft. to 4-ft.)	
10				
20				
30				
40			End of Test pit at 24-ft. (no bedrock encountered)	



FIELD TEST PIT LOG

TEST PIT NO.:	RT-TP-2
SHEET	1 OF 1
DATE: START:	10/3/08
END:	10/3/08
O.G. ELEV.:	Approx. 1548

PROJECT NAME: Concord LOCATION: Race Track

JOB NO.: 5485 CLIENT: Cappelli-Concord

INSPECTOR: CDM CONTRACTOR: Nacirema

EQUIPMENT: Backhoe-365C

WATER LEVEL DEPTH: N/A NOT ENCOUNTERED: X

DEPTH (FT)	SAMPLE NO./TYPE	SOIL CLASSIFICATION	DESCRIPTION	REMARKS
0			Red-Brown medium to fine Sand, some Silt, little coarse to fine Gravel	
			With Gray coarse grained sandstone cobbles from 4-ft. to 24-ft.	
10				
20				
30			End of Test pit at 24-ft. (no bedrock encountered)	
40				

DEPTH (FT)	SAMPLE NO./ TYPE	SOIL CLASSIFICATION	DESCRIPTION	REMARKS
0			Red-Brown medium to fine Sand, some Silt, trace Gravel	
			Mottled veins of Orange medium to fine sand from 0-ft. to 2-ft.	
			Red-Brown medium to fine Sand, some Silt, little coarse to fine Gravel	
			With Gray coarse sandstone cobbles from 4-ft. to 23-ft.	
10				
20				
30				
40				



FIELD TEST PIT LOG

TEST PIT NO.:	RT-TP-4
SHEET	1 OF 1
DATE: START:	10/3/08
END:	10/3/08
O.G. ELEV.:	Approx. 1548

PROJECT NAME: Concord LOCATION: Race Track

JOB NO.: 5485 CLIENT: Cappelli-Concord

INSPECTOR: CDM CONTRACTOR: Nacirema

EQUIPMENT: Backhoe-365C

WATER LEVEL DEPTH: N/A NOT ENCOUNTERED: X

DEPTH (FT)	SAMPLE NO./ TYPE	SOIL CLASSIFICATION	DESCRIPTION	REMARKS
0			Red-Brown medium to fine Sand, some Silt, little medium to fine Gravel	
			Red-Brown medium to fine Sand, some Silt, little coarse to fine Gravel	
			With Gray coarse sandstone cobbles from 4-ft. to 24-ft.	
10				
20				
			End of Test pit at 24-ft. (no bedrock encountered)	
30				
40				



FIELD TEST PIT LOG

TEST PIT NO.:	RT-TP-5	
SHEET	1	OF 1
DATE: START:	10/3/08	
	END: 10/3/08	
O.G. ELEV.:	Approx. 1538	

PROJECT NAME: Concord LOCATION: Race Track

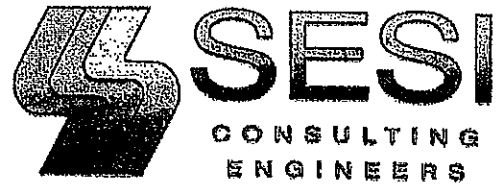
JOB NO.: 5485 CLIENT: Cappelli-Concord

INSPECTOR: CDM CONTRACTOR: Nacirema

EQUIPMENT: Backhoe-365C

WATER LEVEL DEPTH: N/A NOT ENCOUNTERED: X

DEPTH (FT)	SAMPLE NO./ TYPE	SOIL CLASSIFICATION	DESCRIPTION	REMARKS
0			Topsoil	
			Red-Brown medium to fine Sand, some Silt, little medium to fine Gravel	
			Red-Brown medium to fine Sand, some Silt, little coarse to fine Gravel	
			With Gray coarse sandstone cobbles from 4-ft. to 24-ft.	
10				
20				
			End of Test pit at 24-ft. (no bedrock encountered)	
30				
40				



**BORING LOGS, TEST PIT LOGS, AND MONITORING WELL
LOGS FROM REMEDIAL INVESTIGATION
10/29/07 - 11/2/07 AND 5/5/08 - 5/14/08**



LOCATION NAME: Concord Resort

BORING NO.

MW-1 / OU-1c

Monticello, NY

JOB NO.

7180

BORING BY: GBI / Jim C. & Dave C.

DATE STARTED

10/28/2007

GROUND ELEVATION:

INSPECTOR: JZ

DATE COMPLETED

10/30/2007

GROUNDWATER TABLE DEPTH

0 Hr. N/A Date N/A 24 Hr. N/A Date N/A

DEPTH (ft)	PID (ppm)	SAMPLE No.	DEPTH		Blows on Spoon				REC (in)	SOIL DESCRIPTION AND STRATIFICATION	SYMBOL
			FROM (ft)	TO (ft)	0/6	6/12	12/18	18/24			
0	45	1	0	2	8	17	15	20	18	(Top 2") Asphalt/ 3" of Gravel Base coarse below Asphalt (Bottom 13") Red/Brown c-f SAND, little m-f Gravel little Silt	
	0	2	2	4	31	27	30	33	24	Same	
5											
	0	3	5	7	17	35	37	45	24	Same	
	0	4	7	9	37	38	41	40	24	Same	
10	0	5	9	11	51	30	33	45	20	Same	
	0	6	11	13	50/3				3	Same (rock in tip)	
	0	7	13	15	20	31	18	40	18	Same	
15	0	8	15	17	30	44	47	44	15	Same	
	0	9	17	19	47	60	50	64	24	Same	
20	0	10	19	21	22	48	45	64	24	Same	
	0	11	21	23	50/3				3	Same (rock in tip)	
	0	12	23	25	48	50/3			2	Same (rock in tip)	
25	0	13	25	27	22	32	50/3		12	Same	
30											
35											
	0	14	35	37	28	50/4			8	Same (rock in tip)	
40											

Nominal I.D. of Hole	1 1/2 in
Nominal I.D. of Split Barrel Sampler	1 1/2 in
Weight/Type of Hammer on Drive Pipe	300 lb
Weight/Type of Hammer on Split Barrel	140 lb
Drop of Hammer on Drive Pipe	in
Core Size	

The subsurface information shown hereon was obtained for the design and estimating purposes for our client. It is made available to authorized users only that they may have access to the same information available to our client. It is presented in good faith, but it is not intended as a substitute for investigations, interpretations or judgment of such authorized users. Information on the logs should not be relied upon without the geotechnical engineers recommendations contained in the report from which these logs were extracted.

Pp: Pocket Penetrometer; WOH: Weight of Hammer; WOR: Weight of Rod

Approximate Change in Strata:

Inferred Change in Strata:

Soil descriptions represent a field identification after D. M. Burmister unless otherwise noted.

FIGURE #



LOCATION NAME: Concord Resort
Monticello, NY

BORING NO. MW-1 / OU-1c
JOB NO. 7180
GROUND ELEVATION:

BORING BY: GBI / Jim C. & Dave C.

DATE STARTED 10/29/2007

GROUNDWATER TABLE DEPTH

INSPECTOR: JZ

DATE COMPLETED 10/30/2007

0 Hr. N/A Date N/A 24 Hr. N/A Date N/A

DEPTH (ft)	PID (ppm)	SAMPLE No.	DEPTH		Blows on Spoon				REC (in)	SOIL DESCRIPTION AND STRATIFICATION	SYMBOL
			FROM (ft)	TO (ft)	0/6	6/12	12/18	18/24			
40											
	0	15	40	42	50/5				4	Same (rock in tip)	
45											
	0	16	45	47	45	50	44	60	12	Same (rock in tip)	
50	0	17	50	52	50/1				1	Same (rock in tip)	
										END OF BORING AT 50 FEET	
										Refusal on Rock	
										NO Ground Water	
										NO Monitoring Well	
55											
60											
65											
70											
75											
80											

Nominal I.D. of Hole	in
Nominal I.D. of Split Barrel Sampler	1 1/4 in
Weight/type of Hammer on Drive Pipe	300 lb
Weight/type of Hammer on Split Barrel	140 lb
Drop of Hammer on Drive Pipe	In
Core Size	

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Pp: Pocket Penetrometer; WOH: Weight of Hammer; WOR: Weight of Rod

Approximate Change in Strata: _____ Inferred Change in Strata: _____

Soil descriptions represent a field identification after D. M. Burnister unless otherwise noted.

FIGURE #



LOCATION NAME: Concord Resort

BORING NO.

MW-32 / OU-1A

Monticello, NY

JOB NO.

7180

GROUND ELEVATION:

BORING BY: GBI / Jim C. & Dave C.

DATE STARTED

11/1/2007

GROUNDWATER TABLE DEPTH

INSPECTOR: JZ

DATE COMPLETED

11/2/2007

0 Hr.

N/A

Date

N/A

24 Hr.

N/A

Date

N/A

DEPTH (ft)	PID (ppm)	SAMPLE No.	DEPTH		Blows on Spoon				REC (In)	SOIL DESCRIPTION AND STRATIFICATION	SYMBOL
			FROM (ft)	TO (ft)	0/6	6/12	12/18	18/24			
0											
5	0	1	0	2	2	2	4	2	10	Brown m-f SAND, little m-f Gravel, little Silt	
	0	2	2	4	8	16	24	28	24	(Top 6") Same	
10	0	3	4	6	50/3				3	(Bottom 18") Red/Brown m-f SAND, some c-f Gravel, little Silt (Very Dense, Hard to drill, and very dry)	
	0	4	6	8	14	28	36	46	18	Same	
15	0	5	8	10	50/3				3	Same	
	0	6	10	12	50/3				3	Same	
20	0	7	12	14	50/4				4	Same	
	0	8	15	17	50/4				3	Same	
25	0	9	17	19	30	44	60/4		12	Same	
	0	10	19	21	50/1				1	Fractured Cobbles	
30	0	11	25	27	100/5				0	No Recovery (Rock in tip)	
										END OF BORING AT 25 FEET	
35										Refusal on Rock	
										Monitoring Well Installed	
40											

Nominal I.D. of Hole	in
Nominal I.D. of Split Barrel Sampler	1 1/4 in
Weight/type of Hammer on Drive Pipe	300 lb
Weight/type of Hammer on Split Barrel	140 lb
Drop of Hammer on Drive Pipe	in
Core Size	

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Pp: Pocket Penetrometer; WOH: Weight of Hammer; WOR: Weight of Rod

Approximate Change in Strata: _____ Inferred Change in Strata: _____

Soil descriptions represent a field identification after D. M. Burmister unless otherwise noted.

FIGURE #

Page 1 of #



LOCATION NAME: Concord Resort
Monticello, NY

BORING NO. MW-36 / OU-1A
JOB NO. 7180
GROUND ELEVATION:

BORING BY: GBI / Jim C. & Dave C.

DATE STARTED 11/2/2007

INSPECTOR: JZ

DATE COMPLETED 11/2/2007

GROUNDWATER TABLE DEPTH

0 Hr. N/A Date N/A 24 Hr. N/A Date N/A

DEPTH (ft)	PID (ppm)	SAMPLE No.	DEPTH		Blows on Spoon				REC (in)	SOIL DESCRIPTION AND STRATIFICATION	SYMBOL
			FROM (ft)	TO (ft)	0/6	6/12	12/18	18/24			
0										Asphalt	
										Concrete	
										Gravel	
										Concrete	
5	0	1	4	6	13	11	7	16	6	Brown c-f SAND, some c-f Gravel, little Silt (rock in tip)	
	0	2	6	8	17	31	14	25	10	(Top 6") Gray c-f Gravel, little c-f Sand, trace Silt	
	0	3	8	10	28	50/2			6	(Bottom 4") Red/Brown c-f SAND, some Silt, little c-f Gravel	
10										Rocks in Tip/Rocks in spoon	
	0	4	10	12	31	27	24	28	12	Red/Brown c-f SAND, little Gravel, little Silt	
	0	5	12	14	61	50/3			9	Red/Brown Shale	
15											
20										END OF BORING AT 18 FEET	
										Auger Refusal on Rock	
										Monitoring Well Installed	
25											
30											
35											
40											

Nominal I.D. of Hole	in
Nominal I.D. of Split Barrel Sampler	1 1/4 in
Weight/type of Hammer on Drive Pipe	300 lb
Weight/type of Hammer on Split Barrel	140 lb
Drop of Hammer on Drive Pipe	in
Core Size	

The subsurface information shown hereon was obtained for the design and estimating purposes for our client. It is made available to authorized users only that they may have access to the same information available to our client. It is presented in good faith, but it is not intended as a substitute for investigations, interpretations or judgment of such authorized users. Information on the logs should not be relied upon without the geotechnical engineers recommendations contained in the report from which these logs were extracted.

Pp: Pocket Penetrometer; WOH: Weight of Hammer; WOR: Weight of Rod

Approximate Change in Strata: _____ Inferred Change in Strata: _____

Soil descriptions represent a field identification after D. M. Burnmaster unless otherwise noted.

FIGURE #



FIELD TEST PIT LOG

TEST PIT NO.:	TP-1
SHEET	1 OF 1
DATE: START:	5/5/08
END:	5/5/08
O.G. ELEV.:	+1460

PROJECT NAME: Concord LOCATION: Klamesha Lake, NY

JOB NO.: 7375 CLIENT: Cappelli

INSPECTOR: Chris Mazur; Charlie Paternostro CONTRACTOR: _____

EQUIPMENT: _____

WATER LEVEL DEPTH: Seepage @ 6' NOT ENCOUNTERED: _____

C	PID	RELATIVE DENSITY	DESCRIPTION	REMARKS
	0	Loose	FILL: Brown - medium to fine SAND, little silty Clay, Boulders & Cobbles Shale, with brick, concrete, lumber, asphalt	
	0	Medium	Brown m-f SAND, some silty Clay, Boulders Shale	
	0		END OF TEST PIT 9.5', SHALE BEDROCK ENCOUNTERED	
5				
10				
15				
20				



PROJECT NAME: Concord LOCATION: Klamesha Lake, NY

JOB NO.: 7375 CLIENT: Cappelli

INSPECTOR: Chris Mazur; Charlie Paternostro CONTRACTOR: _____

EQUIPMENT:

WATER LEVEL DEPTH: _____ NOT ENCOUNTERED: ☒

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FIELD TEST PIT LOG

TEST PIT NO.: TP-3
SHEET 1 OF 1
DATE: START: 5/5/08
END: 5/5/08
O.G. ELEV.: +1488

PROJECT NAME: Concord LOCATION: Kiamesha Lake, NY

JOB NO.: 7375 CLIENT: Cappelli

INSPECTOR: Chris Mazur; Charlie Paternostro CONTRACTOR:

EQUIPMENT:

WATER LEVEL DEPTH: NOT ENCOUNTERED: X

C	PID	RELATIVE DENSITY	DESCRIPTION	REMARKS
	0	Loose	FILL: Brown - coarse to fine SAND, little silty Clay, with brick, concrete, asphalt, black tar, glass, iron piping, ply wood, asbestos pipe	
5				
	0		END OF TEST PIT @ 8' SHALE BEDROCK ENCOUNTERED	
10				
15				
20				



TEST PIT NO.: TP-4
SHEET 1 OF 1
DATE: START: 5/5/08
END: 5/5/08
O.G. ELEV.: +1499.5

PROJECT NAME: Concord LOCATION: Klamesha Lake, NY

JOB NO.: 7375 CLIENT: Cappelli

INSPECTOR: Chris Mazur; Charlie Palernostro CONTRACTOR: _____

EQUIPMENT: _____

WATER LEVEL DEPTH: _____ NOT ENCOUNTERED: X

C		PID	RELATIVE DENSITY	DESCRIPTION	REMARKS
		0	Loose	FILL: Light Brown coarse to fine Sand, with brick, glass, wood, pipe, Cobbles Shale/Sandstone	
	0	Loose	FILL: Larger diameter pipe, pieces of boiler, asbestos boiler pack, wire mesh, steele, clay tile, light bulbs, fiber glass insulation cinder blocks, coarse to fine gravel		
5					
	0		END OF TEST PIT 5.5', SANDSTONE BEDROCK ENCOUNTERED		
10					
15					
20					



FIELD TEST PIT LOG

TEST PIT NO.:	TP-6
SHEET	1 OF 1
DATE: START:	5/8/08
END:	5/8/08
O.G. ELEV.:	+1472

PROJECT NAME: Concord LOCATION: Klamesha Lake, NY

JOB NO.: 7375 CLIENT: Cappelli

INSPECTOR: Chris Mazur, Charlie Paternostro CONTRACTOR: _____

EQUIPMENT: _____

WATER LEVEL DEPTH: _____ NOT ENCOUNTERED: X

C	PID	RELATIVE DENSITY	DESCRIPTION	REMARKS
	0	Loose	FILL: Brown-light brown coarse to medium SAND, with lumber, pipe, blacktop, trace Silt	
5			Concrete Slab present in test pit	
	0	Medium	Dark brown to Gray, coarse to fine SAND, Shale Cobbles, trace Silt	
10	0		END OF TEST PIT 9.0', SHALE BEDROCK ENCOUNTERED	
15				
20				



FIELD TEST PIT LOG

TEST PIT NO.:	TP-7
SHEET	1 OF 1
DATE: START:	5/8/08
END:	5/8/08
O.G. ELEV.:	+1474

PROJECT NAME: Concord LOCATION: Kiamesha Lake, NY

JOB NO.: 7375 CLIENT: Cappelli

INSPECTOR: Chris Mazur; Charlie Paternostro CONTRACTOR: _____

EQUIPMENT: _____

WATER LEVEL DEPTH: _____ NOT ENCOUNTERED: X

C	PID	RELATIVE DENSITY	DESCRIPTION	REMARKS
	0	Loose	FILL: Brown/Red Shale Rock Brown - Black-coarse to medium Sand, little silty Clay, with cinders, sheet rock, burned lumber, metal pipe, wire, copper pipe	
5	0	Medium	FILL: Gray- Black-Brown-Orange "rust", Brown coarse to medium SAND, weathered Shale rock	
	0	Medium	Brown-medium to fine SAND, some silty Clay	
10			END OF TEST PIT 8', SHALE BEDROCK ENCOUNTERED	
15				
20				



TEST PIT NO.: TP-8
SHEET 1 OF 1
DATE: START: 5/8/08
END: 5/8/08
O.G. ELEV.: + 1470

PROJECT NAME: Concord LOCATION: Klamesha Lake, NY

JOB NO.: 7375 CLIENT: Cappelli

INSPECTOR: Chris Mazur, Charlie Palernostro CONTRACTOR: _____

EQUIPMENT: _____

WATER LEVEL DEPTH: 3.5' NOT ENCOUNTERED:

NOT ENCOUNTERED: _____

C	PID	RELATIVE DENSITY	DESCRIPTION	REMARKS
	0	Medium	FILL: Brown/Red Shale rock on surface, coarse Gravel with burned wood, metal, cinder block, rusted pipe, wire, Soil was multicolored, black, brown, orange white, coarse to medium sand	
5				
			END OF TEST 6.0', SHALE BEDROCK ENCOUNTERED	
10				
15				
20				



FIELD TEST PIT LOG

TEST PIT NO.:	TP-9
SHEET	1 OF 1
DATE: START:	5/6/08
END:	5/6/08
O.G. ELEV.:	+1486

PROJECT NAME: Concord LOCATION: Klamesha Lake, NY

JOB NO.: 7375 CLIENT: Cappelli

INSPECTOR: Chris Mazur, Charlie Paternostro CONTRACTOR: _____

EQUIPMENT: _____

WATER LEVEL DEPTH: _____ NOT ENCOUNTERED: X

C	PID	RELATIVE DENSITY	DESCRIPTION	REMARKS
	0	Medium	Brown coarse to medium SAND, shale rock cobbles, pea gravel, trace silt	
	0	Medium	Concrete slab, with Asbestos Transite pipe, steel pipe, concrete	
	0	Dense	Brown coarse to fine sand, shale rock-cobbles, trace clay	
5			END OF TEST PIT 4.0' bg, SAND STONE BEDROCK ENCOUNTERED	
10				
15				
20				



TEST PIT NO.: TP-10
SHEET 1 OF 1
DATE: START: 5/12/08
END: 5/12/08
O.G. ELEV.: +1460

PROJECT NAME: Concord LOCATION: Klamasha Lake, NY

JOB NO.: 7375 CLIENT: Cappelli

INSPECTOR: Chris Mazur; Charlie Paternostro CONTRACTOR: _____

EQUIPMENT: _____

WATER LEVEL DEPTH: _____ NOT ENCOUNTERED: X

C	PID	RELATIVE DENSITY	DESCRIPTION	REMARKS
	0	Dense	Brown- coarse to fine sand, trace silt, Shale cobbles & boulders	
5				
10			END OF TEST PIT 7.0' bg, SHALE BEDROCK ENCOUNTERED	
15				
20				



FIELD TEST PIT LOG

TEST PIT NO.:	TP-11
SHEET	1 OF 1
DATE: START:	5/13/08
END:	5/13/08
O.G. ELEV.:	+1492

PROJECT NAME: Concord LOCATION: Klamesha Lake, NY

JOB NO.: 7375 CLIENT: Cappelli

INSPECTOR: Chris Mazur, Charlie Paternostro CONTRACTOR: _____

EQUIPMENT: _____

WATER LEVEL DEPTH: _____ NOT ENCOUNTERED: X

C	PID	RELATIVE DENSITY	DESCRIPTION	REMARKS
	0	Medium	Brown, coarse to fine SAND, Shale boulders & cobbles, with concrete, blacktop, rebar, brick, carpet, metal pipe, lumber	
5				
			END OF TEST PIT 5.5' bg, BEDROCK ENCOUNTERED	
10				
15				
20				



FIELD TEST PIT LOG

TEST PIT NO.: AE-1
SHEET 1 OF 1
DATE: START: 5/12/08
END: 5/12/08
O.G. ELEV.: 1480

PROJECT NAME: Concord LOCATION: Klamesha Lake, NY

JOB NO.: 7375 CLIENT: Cappelli

INSPECTOR: Chris Mazur; Charlie Patemostro CONTRACTOR: _____

EQUIPMENT: _____

WATER LEVEL DEPTH: _____ NOT ENCOUNTERED: X

C	PID	RELATIVE DENSITY	DESCRIPTION	REMARKS
	0	Loose	Fill: Red - Brown coarse to fine SAND, angular/sub-angular Shale, coarse to fine Gravel, trace Silt	
	0	Medium	Orange - Brown CLAY, some Sand, little Silt	
	0	Dense	Brown coarse to fine SAND, little Silt	
5				
10			END OF TEST PIT 7.0' bg, SHALE BEDROCK ENCOUNTERED	
15				
20				



FIELD TEST PIT LOG

TEST PIT NO.: AE-2
 SHEET 1 OF 1
 DATE: START: 5/12/08
 END: 5/12/08
 O.G. ELEV.: 1496

PROJECT NAME: Concord LOCATION: Klamesha Lake, NY

JOB NO.: 7375 CLIENT: Cappelli

INSPECTOR: Chris Mazur; Charlie Paternostro CONTRACTOR: _____

EQUIPMENT: _____

WATER LEVEL DEPTH: _____ NOT ENCOUNTERED: X

C	PID	RELATIVE DENSITY	DESCRIPTION	REMARKS
	0	Medium	Filt: Brown coarse to medium SAND and Silt, trace Clay, Fragments of Wood and Metal present.	
	0	Medium	Light Brown - Gray medium to fine SAND, some Clay, trace Silt	
5	0	Medium	Gray - Light Brown medium to fine SAND, trace Silt, fragments of angular/sub-angular Shale	
	0	Medium	Light - Dark Brown medium to fine SAND, some Clay, trace Silt, large pieces of Shale encountered	
10			END OF EXCAVATION 10.0' bg SHALE BEDROCK ENCOUNTERED	
15				
20				



FIELD TEST PIT LOG

TEST PIT NO.:	AE-3
SHEET	1 OF 1
DATE: START:	5/14/08
END:	5/14/08
O.G. ELEV.:	1494

PROJECT NAME: Concord LOCATION: Klamesha Lake, NY

JOB NO.: 7375 CLIENT: Cappelli

INSPECTOR: Chris Mazur, Charlie Paternostro

CONTRACTOR: _____

EQUIPMENT: _____

WATER LEVEL DEPTH: _____

NOT ENCOUNTERED: X

C	PID	RELATIVE DENSITY	DESCRIPTION	REMARKS
	0	Medium	Brown coarse to fine SAND, trace Clay, trace Silt, fragments of angular/sub-angular Shale	No vapors/staining encountered No env. soil sample taken
		Medium	Brown - Red/Brown coarse to fine SAND, and Silt, angular/sub-angular Shale	
5		Dense	Red - Brown SHALE, trace Silt	
			END OF TESTPIT 6.0' bg, SHALE BEDROCK ENCOUNTERED	
10				
15				
20				



FIELD TEST PIT LOG

TEST PIT NO.:	AE-4
SHEET	1 OF 1
DATE: START:	5/14/08
END:	5/14/08
O.G. ELEV.:	1486

PROJECT NAME: Concord LOCATION: Kiamesha Lake, NY

JOB NO.: 7375 CLIENT: Cappelli

INSPECTOR: Chris Mazur; Charlie Paternostro CONTRACTOR: _____

EQUIPMENT: _____

WATER LEVEL DEPTH: _____ NOT ENCOUNTERED: X

C	PID	RELATIVE DENSITY	DESCRIPTION	REMARKS
	0	Loose	Fill: Light Brown - Brown, medium to fine SAND, little Silt, trace Clay	No vapors/staining encountered No env. soil sample taken
	0	Medium	Fill: Brown medium to fine SAND, little Silt, trace Clay, Misc. debris present (Transite and Metal pipe fragments, cinder blocks, glass and Construction and Demolition debris)	
5	0	Medium	Fill: Light Brown - Gray coarse to fine SAND, little Silt, trace Clay, fragments of asphalt and brick	
	0	Dense	Gray - Reddish/Brown coarse to medium SAND, little Clay, trace Silt, angular to sub-angular Shale	
			END OF TEST PIT AT 9.0 ft-bgs, SHALE BEDROCK ENCOUNTERED	
10				
15				
20				



O.G. ELEV.: 1490

NOT ENCOUNTERED: X

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FIELD TEST PIT LOG

TEST PIT NO.:	AE-6
SHEET	1 OF 1
DATE: START:	5/14/08
END:	5/14/08
O.G. ELEV.:	1490

PROJECT NAME: Concord LOCATION: Klamesha Lake, NY

JOB NO.: 7375 CLIENT: Cappelli

INSPECTOR: Chris Mazur; Charlie Paternostro CONTRACTOR: _____

EQUIPMENT: _____

WATER LEVEL DEPTH: _____ NOT ENCOUNTERED: X

C	PID	RELATIVE DENSITY	DESCRIPTION	REMARKS
	0	Loose	Fill: Brown coarse - medium SAND, little Clay, angular/sub-angular Rock	
	0	Loose	Fill: Same, with fragments of black Rock	
	0	Medium	Fill: Same, with fragments of wood and pipe	
5				
	0	Dense	Same, with shale angular/sub-angular Rock	
	0	Dense	Light Brown - Gray, little Clay, angular/sub-angular Rock	
10			END OF TEST PIT 10.0' bg, SHALE BEDROCK ENCOUNTERED	
15				
20				



FIELD TEST PIT LOG

TEST PIT NO.:	UST-1
SHEET	1 OF 1
DATE: START:	5/5/08
END:	5/5/08
O.G. ELEV.:	+1482

PROJECT NAME: Concord LOCATION: Klamesha Lake, NY

JOB NO.: 7375 CLIENT: Cappelli

INSPECTOR: Chris Mazur, Charlie Paternostro CONTRACTOR: _____

EQUIPMENT: _____

WATER LEVEL DEPTH: 8.5' NOT ENCOUNTERED: _____

C	PID	RELATIVE DENSITY	DESCRIPTION	REMARKS
	0	Medium	Brown medium to fine Sand, some silty Clay, Trace Organics (tree roots)	
5				
	0	Medium	Brown coarse to fine SAND, some silty Clay, Shale cobbles & boulders	
10				
			END OF TEST PIT @ 10' bg, SHALE BEDROCK ENCOUNTERED	
15				
20				

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FIELD TEST PIT LOG

TEST PIT NO.:	UST-3
SHEET	1 OF 1
DATE: START:	5/5/08
END:	5/5/08
O.G. ELEV.:	+1472

PROJECT NAME: Concord LOCATION: Klamesha Lake, NY

JOB NO.: 7375 CLIENT: Cappelli

INSPECTOR: Chris Mazur, Charlie Paternostro CONTRACTOR: _____

EQUIPMENT: _____

WATER LEVEL DEPTH: _____ NOT ENCOUNTERED: X

C	PID	RELATIVE DENSITY	DESCRIPTION	REMARKS
	0	Medium	Brown coarse to fine SAND, little silty Clay, Shale cobbles	
5				
10	0		END OF TEST PIT 10' bg, SHALE BEDROCK ENCOUNTERED	
15				
20				



PROJECT NAME: Concord LOCATION: Kiamesha Lake, NY

JOB NO.: 7375 CLIENT: Cappelli

INSPECTOR: Chris Mazur; Charlie Paternostro CONTRACTOR: _____

EQUIPMENT:

WATER LEVEL DEPTH: 2.5' NOT ENCOUNTERED:

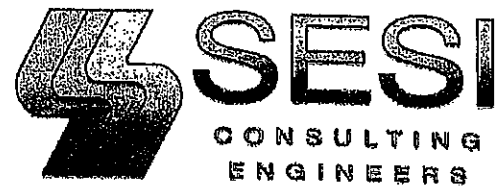
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O.G. ELEV.: + 1499

NOT ENCOUNTERED:

C	PID	RELATIVE DENSITY	DESCRIPTION	REMARKS
	0	Medium	Brown-coarse to fine SAND, trace silty clay, some Shale Cobbles & Boulders	
5				
			END OF TESTPIT 8.0', SHALE BEDROCK ENCOUNTERED	
10				
15				
20				



**BORING LOGS FROM SUBSURFACE INVESTIGATION
6/13/08 – 7/15/08**

			PROJECT NAME:		CONCORD		BORING NO.		SB-1		
			LOCATION:		Monticello, NY		JOB NO.		5485		
BORING BY: GBI			DATE STARTED:		8/13/2008		GROUNDWATER TABLE DEPTH: NE				
INSPECTOR: JN			DATE COMPLETED:		6/13/2008		0 Hr. N/A Date N/A 24 Hr. N/A Date N/A				
DEPTH (ft)	METHOD	SAMPLE No.	DEPTH		Blows on Spoon				REC (in)	SOIL DESCRIPTION AND STRATIFICATION	SYMBOL
			FROM (ft)	TO (ft)	0/6	6/12	12/18	18/24			
0											
	SS	1	0	2	4	8	7	7	12	3" Topsoil	
										Brown coarse to fine SAND, little Silt, trace Gravel	
5											
	SS	2	5	7	44	54	75	69	22	Red-Brown SILT, some coarse Gravel, little coarse to fine Sand	
10											
	SS	3	10	12	14	75/2"			8	Brown medium to fine SAND, trace Silt (Weathered Rock)	
15									1.0m	ROCK CORE: RUN 1 (11'-16')	
									2.0m	RECOVERY = 55" / 60" = 91.7 %	
									1.5m	RQD = 20 / 60" = 33.3%	
									1.0m	Gray coarse grained, slightly weathered, strong SANDSTONE	
									1.0m	w/ closely spaced fractures	
20									1.0m	ROCK CORE: RUN 2 (16'-21')	
									1.25m	RECOVERY = 55" / 60" = 91.7%	
									1.25m	RQD = 30" / 60" = 50.0%	
									1.25m	Red-brown fine grained, slightly to highly weathered, very	
									1.25m	weak to medium strong SHALE w/ closely spaced fractures	
25										BORING COMPLETE AT 21.0 FEET	
30											
35											
40											

Nominal I.D. of Hole	in
Nominal I.D. of Split Barrel Sampler	1 1/4 in
Weight/type of Hammer on Drive Pipe	300 lb
Weight/type of Hammer on Split Barrel	140 lb
Drop of Hammer on Drive Pipe	in
Core Size	in


The subsurface information shown hereon was obtained for the design and estimating purposes for our client. It is made available to authorized users only that they may have access to the same information available to our client. It is presented in good faith, but it is not intended as a substitute for investigations, interpretations or judgment of such authorized users. Information on the logs should not be relied upon without the geotechnical engineers recommendations contained in the report from which these logs were extracted.

Pp: Pocket Penetrometer; WOH: Weight of Hammer; WOR: Weight of Rod

Approximate Change in Strata: _____ Inferred Change in Strata: _____

Soil descriptions represent a field identification after D. M. Burnister unless otherwise noted.

FIGURE 2

					PROJECT NAME:		CONCORD		BORING NO.		SB-2		
					LOCATION:		Monticello, NY		JOB NO.		5485		
BORING BY: GBI					DATE STARTED:		6/13/2008		GROUNDWATER TABLE DEPTH: NE				
INSPECTOR: JN					DATE COMPLETED:		6/13/2008		0 Hr. N/A		Date N/A		
									24 Hr. N/A		Date N/A		
DEPTH (ft)	METHOD	SAMPLE No.	DEPTH FROM TO		Blows on Spoon				REC	SOIL DESCRIPTION AND STRATIFICATION			SYMBOL
0			(ft)	(ft)	0/6	6/12	12/18	18/24	(in)				
	SS	1	0	2	6	8	6	8	10	3" Topsoil & Roots			
										Brown coarse to fine SAND, little Silt, little coarse to medium Gravel			
5													
	SS	2	5	7	33	75/5"			10	Red-Brown SILT, some coarse Gravel, little coarse to fine Sand			
										Refusal on Rock			
10										BORING COMPLETE AT 6.5 FBET			
15													
20													
25													
30													
35													
40													

Nominal I.D. of Hole	in
Nominal I.D. of Split Barrel Sampler	1 1/4 in
Weight/type of Hammer on Drive Pipe	300 lb
Weight/type of Hammer on Split Barrel	140 lb
Drop of Hammer on Drive Pipe	in
Core Size	in

The subsurface information shown hereon was obtained for the design and estimating purposes for our client. It is made available to authorized users only that they may have access to the same information available to our client. It is presented in good faith, but it is not intended as a substitute for investigations, interpretations or judgment of such authorized users. Information on the logs should not be relied upon without the geotechnical engineers recommendations contained in the report from which these logs were extracted.

Pp: Pocket Penetrometer; WOH: Weight of Hammer; WOR: Weight of Rod

Approximate Change in Strata: _____ Inferred Change in Strata: _____


Soil descriptions represent a field identification after D. M. Burmeister unless otherwise noted.

FIGURE 3

BORING BY: GBI		DATE STARTED: 6/13/2008		GROUNDWATER TABLE DEPTH: NE			
INSPECTOR: JN/JZ		DATE COMPLETED: 6/13/2008		0 Hr.	N/A	Date N/A	24 Hr. N/A
DEPTH		DEPTH					

Nominal I.D. of Hole	In	<p>The subsurface information shown hereon was obtained for the design and estimating purposes for our client. It is made available to authorized users only that they may have access to the same information available to our client. It is presented in good faith, but it is not intended as a substitute for investigations, interpretations or judgment of such authorized users. Information on the logs should not be relied upon without the geotechnical engineers recommendations contained in the report from which these logs were extracted.</p> <p>Pp: Pocket Penetrometer; WOH: Weight of Hammer; WOR: Weight of Rod</p>
Nominal I.D. of Split Barrel Sampler	1 1/2 In	
Weight/type of Hammer on Drive Pipe	300 lb	
Weight/type of Hammer on Split Barrel	140 lb	
Drop of Hammer on Drive Pipe	In	
Core Size	In	

Approximate Change in Strata: _____ Inferred Change in Strata: _____

				PROJECT NAME:		CONCORD		BORING NO.		SB-4			
				LOCATION:		Monticello, NY		JOB NO.		5485			
BORING BY: GBI				DATE STARTED:		6/13/2008		GROUNDWATER TABLE DEPTH: NE					
INSPECTOR: JZ				DATE COMPLETED:		6/13/2008		0 Hr. N/A		Date N/A			
DEPTH (ft)		METHOD	SAMPLE No.	DEPTH (ft)		Blows on Spoon				REC (in)	SOIL DESCRIPTION AND STRATIFICATION		SYMBOL
0		ss	1	0 2		3 5 6 11				24	Red-brown c-f SAND, some c-f Gravel, little Silt		
5													
		ss	2	5 7		12 50/3" - -				9	Same (rock in tip) ... Weathered Rock (auger grinding)		
10													
15											Refusal on Rock BORING COMPLETE AT 8.0 FBET		
20													
25													
30													
35													
40													

Nominal I.D. of Hole	In
Nominal I.D. of Split Barrel Sampler	1 3/4 In
Weight/type of Hammer on Drive Pipe	300 lb
Weight/type of Hammer on Split Barrel	140 lb
Drop of Hammer on Drive Pipe	In
Core Size	In


The subsurface information shown hereon was obtained for the design and estimating purposes for our client. It is made available to authorized users only that they may have access to the same information available to our client. It is presented in good faith, but it is not intended as a substitute for investigations, interpretations or judgment of such authorized users. Information on the logs should not be relied upon without the geotechnical engineers recommendations contained in the report from which these logs were extracted.

Pp: Pocket Penetrometer; WOH: Weight of Hammer; WOR: Weight of Rod

Approximate Change in Strata: _____ Inferred Change in Strata: _____

Soil descriptions represent a field identification after D. M. Burnmaster unless otherwise noted.

FIGURE 5

		PROJECT NAME:		CONCORD		BORING NO.		SB-5			
		LOCATION:		Monticello, NY		JOB NO.		5485			
BORING BY: GBI		DATE STARTED:		6/13/2008		GROUNDWATER TABLE DEPTH: NE					
INSPECTOR: JZ		DATE COMPLETED:		6/16/2008		0 Hr. N/A		Date N/A			
						24 Hr. N/A		Date N/A			
DEPTH (ft)	METHOD	SAMPLE No.	DEPTH FROM TO (ft) (ft)		Blows on Spoon				REC (In)	SOIL DESCRIPTION AND STRATIFICATION	SYMBOL
0					0/6	6/12	12/18	18/24		6" Concrete & Rebar / 6" Gravel Weathered Rock	
5										ROCK CORE: RUN 1 (2.5'-7.5') RECOVERY = 55" / 60" = 91.6% RQD = 0" / 60" = 0% Gray medium to fine grained, slightly weathered, medium strong SANDSTONE w/ very closely spaced fractures	
10										ROCK CORE: RUN 2 (7.5'-12.5') RECOVERY = 51" / 60" = 85% RQD = 0" / 60" = 0% Gray medium to fine grained, slightly weathered, medium strong SANDSTONE w/ very closely spaced fractures	
15										BORING COMPLETE AT 12.5 FEET	
20											
25											
30											
35											
40											

Nominal I.D. of Hole	In
Nominal I.D. of Split Barrel Sampler	1 1/4 In
Weight/type of Hammer on Drive Pipe	300 lb
Weight/type of Hammer on Split Barrel	140 lb
Drop of Hammer on Drive Pipe	In
Core Size	In


The subsurface information shown hereon was obtained for the design and estimating purposes for our client. It is made available to authorized users only that they may have access to the same information available to our client. It is presented in good faith, but it is not intended as a substitute for investigations, interpretations or judgment of such authorized users. Information on the logs should not be relied upon without the geotechnical engineers recommendations contained in the report from which these logs were extracted.

Pp: Pocket Penetrometer; WOH: Weight of Hammer; WOR: Weight of Rod

Approximate Change in Strata: _____ Inferred Change in Strata: _____

Soil descriptions represent a field identification after D. M. Burmister unless otherwise noted.

FIGURE 6

		PROJECT NAME:		CONCORD		BORING NO.		SB-6			
		LOCATION:		Monticello, NY		JOB NO.		5485			
BORING BY: GBI		DATE STARTED:				GROUNDWATER TABLE DEPTH: NE					
INSPECTOR: JZ		DATE COMPLETED:				0 Hr. N/A		Date N/A			
						24 Hr. N/A		Date N/A			
DEPTH (ft)	METHOD	SAMPLE No.	DEPTH FROM TO (ft) (ft)		Blows on Spoon				REC	SOIL DESCRIPTION AND STRATIFICATION	SYMBOL
0					0/6	6/12	12/18	18/24	(in)		
5										NOT DRILLED	
10											
15											
20											
25											
30											
35											
40											

Nominal I.D. of Hole	in
Nominal I.D. of Split Barrel Sampler	1 1/4 in
Weight/type of Hammer on Drive Pipe	300 lb
Weight/type of Hammer on Split Barrel	140 lb
Drop of Hammer on Drive Pipe	in
Core Size	in


The subsurface information shown hereon was obtained for the design and estimating purposes for our client. It is made available to authorized users only that they may have access to the same information available to our client. It is presented in good faith, but it is not intended as a substitute for investigations, interpretations or judgment of such authorized users. Information on the logs should not be relied upon without the geotechnical engineers recommendations contained in the report from which these logs were extracted.

Pp: Pocket Penetrometer; WOH: Weight of Hammer; WOR: Weight of Rod

Approximate Change in Strata: _____ Inferred Change in Strata: _____

Soil descriptions represent a field identification after D. M. Burmister unless otherwise noted.

FIGURE 7


			PROJECT NAME:		CONCORD		BORING NO.		SB-7				
			LOCATION:		Monticello, NY		JOB NO.		5485				
BORING BY: GBI			DATE STARTED:		6/16/2008		GROUNDWATER TABLE DEPTH: NE						
INSPECTOR: JZ			DATE COMPLETED:		6/16/2008		0 Hr. N/A		Date N/A				
DEPTH (ft)		METHOD	SAMPLE No.	DEPTH		Blows on Spoon				REC (in)	SOIL DESCRIPTION AND STRATIFICATION	SYMBOL	
0				FROM (ft)	TO (ft)	0/6	6/12	12/18	18/24				
5		ss	1	0	2	4	7	8	13	18	Brown/Red-brown c-f SAND, little c-f Gravel, little Silt, trace Roots		
10		ss	2	5	7	10	11	19	13	18		Same	
15		ss	3	10	12	100/3"	-	-	-	2"			Rock in tip
20											ROCK CORE: RUN1 (12' - 17')		
25												RECOVERY = 60" / 60" = 100%	
30													RQD = 0" / 60" = 0%
35											Gray medium to fine grained, slightly weathered, medium strong SANDSTONE w/ very closely spaced fractures		
40												BORING COMPLETE AT 17.0 FEET	

Nominal I.D. of Hole	in	The subsurface information shown hereon was obtained for the design and estimating purposes for our client. It is made available to authorized users only that they may have access to the same information available to our client. It is presented in good faith, but it is not intended as a substitute for investigations, interpretations or judgment of such authorized users. Information on the logs should not be relied upon without the geotechnical engineers recommendations contained in the report from which these logs were extracted.
Nominal I.D. of Split Barrel Sampler	1 1/4 in	
Weight/type of Hammer on Drive Pipe	300 lb	
Weight/type of Hammer on Split Barrel	140 lb	
Drop of Hammer on Drive Pipe	in	
Core Size	in	

Pp: Pocket Penetrometer; WOH: Weight of Hammer; WOR: Weight of Rod

Soil descriptions represent a field identification after D. M. Burmister unless otherwise noted.

FIGURE 8


		PROJECT NAME:		CONCORD		BORING NO.		SB-8			
		LOCATION:		Monticello, NY		JOB NO.		5485			
BORING BY: GBI		DATE STARTED:		6/16/2008		GROUNDWATER TABLE DEPTH: Seepage @ 11'					
INSPECTOR: JZ		DATE COMPLETED:		8/16/2008		0 Hr. 14.5' Date 6/16/08 24 Hr. N/A Date N/A					
DEPTH (ft)	METHOD	SAMPLE No.	DEPTH		Blows on Spoon				REC (in)	SOIL DESCRIPTION AND STRATIFICATION	SYMBOL
			FROM (ft)	TO (ft)	0/6	6/12	12/18	18/24			
0	SS	1	0	2	14	14	20	22	24	Fill: Red-brown c-f SAND, some c-f Gravel, little Silt	
5											
10	SS	2	5	7	29	32	49	58	24	Red-brown c-f SAND, some c-f Gravel, little Silt, w/ Cobbles	
15	SS	3	10	12	27	18	24	15	24	Orange-brown/Red-brown/Gray c-f SAND, some c-f Gravel little Silt w/ Cobbles	
20	SS	4	15	17	12	22	20	41	24	Red-brown c-f SAND, some c-f Gravel, little Silt (Rock in tip)	
25	SS	5	20	22	19	50/2"	-	-	8	Red-brown c-m SAND and c-f Gravel, little Silt w/ Cobbles	
30										Refusal on Rock	
35										BORING COMPLETE AT 21.0 FBET	
40											

Nominal I.D. of Hole	in	The subsurface information shown hereon was obtained for the design and estimating purposes for our client. It is made available to authorized users only that they may have access to the same information available to our client. It is presented in good faith, but it is not intended as a substitute for investigations, interpretations or judgment of such authorized users. Information on the logs should not be relied upon without the geotechnical engineers recommendations contained in the report from which these logs were extracted.
Nominal I.D. of Split Barrel Sampler	1 1/4 in	
Weight/type of Hammer on Drive Pipe	300 lb	
Weight/type of Hammer on Split Barrel	140 lb	
Drop of Hammer on Drive Pipe	In	
Core Size	In	Pp: Pocket Penetrometer; WOH: Weight of Hammer; WOR: Weight of Rod

Approximate Change in Strata: _____ Inferred Change in Strata: _____


Soil descriptions represent a field identification after D. M. Burmister unless otherwise noted.

FIGURE 9

				PROJECT NAME:		CONCORD		BORING NO.		SB-9	
				LOCATION:		Monticello, NY		JOB NO.		5485	
BORING BY: GBI				DATE STARTED:		6/16/2008		GROUNDWATER TABLE DEPTH: NE			
INSPECTOR: JZ				DATE COMPLETED:		6/16/2008		0 Hr. N/A		Date N/A	
DEPTH (ft)	METHOD	SAMPLE No.	DEPTH		Blows on Spoon				REC (in)	SOIL DESCRIPTION AND STRATIFICATION	SYMBOL
			FROM (ft)	TO (ft)	0/6	6/12	12/18	18/24			
0											
	SS	1	0	2	18	21	27	30	24	Fill: Red-brown c-f SAND, some c-f Gravel, little Silt	
5											
	SS	2	5	7	43	50/3"	-	-	8	Red-brown c-f SAND (rock in tip)	
10										... Boulder (hard drilling from 8' to 9')	
	SS	3	10	12	58	37	33	38	24	Red-brown/Gray c-f SAND and c-f Gravel, little Silt w/ Cobbles	
15											
	SS	4	15	17	31	45	50/2"	-	12	Same	
20										Refusal on Rock	
										BORING COMPLETE AT 18.0 FEET	
25											
30											
35											
40											


Nominal I.D. of Hole	in	<p>The subsurface information shown hereon was obtained for the design and estimating purposes for our client. It is made available to authorized users only that they may have access to the same information available to our client. It is presented in good faith, but it is not intended as a substitute for investigations, interpretations or judgment of such authorized users. Information on the logs should not be relied upon without the geotechnical engineers recommendations contained in the report from which these logs were extracted.</p> <p>Pp: Pocket Penetrometer; WOH: Weight of Hammer; WOR: Weight of Rod</p> <p>Approximate Change in Strata: _____ Inferred Change in Strata: _____</p> <p>Soil descriptions represent a field identification after D. M. Burmister unless otherwise noted.</p>
Nominal I.D. of Split Barrel Sampler	1 1/2 in	
Weight/type of Hammer on Drive Pipe	300 lb	
Weight/type of Hammer on Split Barrel	140 lb	
Drop of Hammer on Drive Pipe	in	
Core Size	in	

FIGURE 10

			PROJECT NAME:		CONCORD		BORING NO.		SB-10		
			LOCATION:		Monticello, NY		JOB NO.		5485		
BORING BY: GBI			DATE STARTED:		6/16/2008		GROUNDWATER TABLE DEPTH: 10'				
INSPECTOR: JZ			DATE COMPLETED:		6/16/2008		0 Hr. 10'		Date 6/16/08 24 Hr. N/A Date N/A		
DEPTH (ft)	METHOD	SAMPLE No.	DEPTH FROM TO (ft) (ft)		Blows on Spoon 0/6 6/12 12/18 18/24 (in)				REC	SOIL DESCRIPTION AND STRATIFICATION	SYMBOL
0	SS	1	0	2	16	27	30	26	24	Fill: Red-brown c-f SAND, some c-f Gravel, little Silt	
5											
	SS	2	5	7	21	36	76	45	24	Red-brown c-f SAND, some c-f Gravel, little Silt, w/ Cobbles	
10											
	SS	3	10	12	40	24	23	21	24	Same	
15										... Hard drilling (weathered rock/cobbles)	
	SS	4	15	17	39	36	45	53	15	Gray c-m SAND and c-f Gravel, little Silt w/ fractured Rock	
20											
	SS	5	20	22	21	50/1"	-	-	6	Same (rock in tip)	
25										Refusal on Rock	
										BORING COMPLETE AT 21.0 FEET	
30											
35											
40											

Nominal I.D. of Hole	in	The subsurface information shown hereon was obtained for the design and estimating purposes for our client. It is made available to authorized users only that they may have access to the same information available to our client. It is presented in good faith, but it is not intended as a substitute for investigations, interpretations or judgment of such authorized users. Information on the logs should not be relied upon without the geotechnical engineers recommendations contained in the report from which these logs were extracted.
Nominal I.D. of Split Barrel Sampler	1 1/4 in	
Weight/type of Hammer on Drive Pipe	300 lb	
Weight/type of Hammer on Split Barrel	140 lb	
Drop of Hammer on Drive Pipe	in	
Core Size	in	
Pp: Pocket Penetrometer; WOH: Weight of Hammer; WOR: Weight of Rod		
Approximate Change in Strata: _____ Inferred Change in Strata: _____		
Soil descriptions represent a field identification after D. M. Burnister unless otherwise noted.		

FIGURE 11

		PROJECT NAME:		CONCORD		BORING NO.		SB-11			
		LOCATION:		Monticello, NY		JOB NO.		5485			
BORING BY: GBI		DATE STARTED:		7/15/2008		GROUNDWATER TABLE DEPTH: 7.0' b.g.					
INSPECTOR: CDM		DATE COMPLETED:		7/16/2008		0 Hr. N/A		Date N/A			
						24 Hr. N/A		Date N/A			
DEPTH (ft)	METHOD	SAMPLE No.	DEPTH FROM TO (ft) (ft)		Blows on Spoon				REC (In)	SOIL DESCRIPTION AND STRATIFICATION	SYMBOL
0					0/6	6/12	12/18	18/24			
	ss	1	1	3	10	10	11	14	17	Fill: Brown SILT, little coarse to fine Sand, little coarse to fine Gravel	
5										Brown coarse to fine SAND, little coarse to fine Gravel, trace Silt	
	ss	2	5	7	38	36	28	32	0	NO RECOVERY	
10											
	ss	3	10	12	24	58	77	72	20	Gray SILT and coarse to fine Sand (weathered Sandstone)	
15											
										Refusal on rock	
20										BORING COMPLETE AT 14.5 FEET	
25											
30											
35											
40											

Nominal I.D. of Hole	In
Nominal I.D. of Split Barrel Sampler	1 1/2 In
Weight/type of Hammer on Drive Pipe	300 lb
Weight/type of Hammer on Split Barrel	140 lb
Drop of Hammer on Drive Pipe	In
Core Size	In


The subsurface information shown hereon was obtained for the design and estimating purposes for our client. It is made available to authorized users only that they may have access to the same information available to our client. It is presented in good faith, but it is not intended as a substitute for investigations, interpretations or judgment of such authorized users. Information on the logs should not be relied upon without the geotechnical engineers recommendations contained in the report from which these logs were extracted.

Pp: Pocket Penetrometer; WOH: Weight of Hammer; WOR: Weight of Rod

Approximate Change in Strata: _____ Inferred Change in Strata: _____

Soil descriptions represent a field identification after D. M. Burmister unless otherwise noted.

FIGURE 12

				PROJECT NAME:		CONCORD		BORING NO.		SB-12	
				LOCATION:		Monticello, NY		JOB NO.		5485	
BORING BY: GBI				DATE STARTED:		7/14/2008		GROUNDWATER TABLE DEPTH: NE			
INSPECTOR: CDM				DATE COMPLETED:		7/14/2008		0 Hr. N/A Date N/A 24 Hr. N/A Date N/A			
DEPTH (ft)	METHOD	SAMPLE No.	DEPTH		Blows on Spoon				REC (in)	SOIL DESCRIPTION AND STRATIFICATION	SYMBOL
			FROM (ft)	TO (ft)	0/6	6/12	12/18	18/24			
0	SS	1	0	2	4	5	7	9	6	Red-Brown coarse to fine SAND, little coarse to fine Gravel, trace Silt	
5											
	SS	2	5	7	5	5	75/3"	-	10	Red-Brown SILT, little coarse to fine Sand, trace Gravel	
10											
	SS	3	10	12	75/2"	-	-	-	2	Red-Brown SILT, little coarse to fine Sand, little coarse to fine Gravel	
15										Gray coarse to fine grained weathered SANDSTONE	
	SS	4	15	17	75/4"	-	-	-	4		
20									.75m	ROCK CORE: RUN 1 (16'-18')	
									.75m	RECOVERY = 12"/24" = 20%	
									1m	RQD = 0"/24" = 0%	
									1m	Gray coarse to fine grained slightly weathered SANDSTONE	
25									1.25m	ROCK CORE: RUN 2 (18'-23')	
									1.25m	RECOVERY = 60"/60" = 100%	
									1.5m	RQD = 22"/60" = 37%	
										Gray coarse to fine grained slightly weathered SANDSTONE	
30										BORING COMPLETE AT 23.0 FEET	
35											
40											

Nominal I.D. of Hole	in	The subsurface information shown hereon was obtained for the design and estimating purposes for our client. It is made available to authorized users only that they may have access to the same information available to our client. It is presented in good faith, but it is not intended as a substitute for investigations, interpretations or judgment of such authorized users. Information on the logs should not be relied upon without the geotechnical engineers recommendations contained in the report from which these logs were extracted. Pp: Pocket Penetrometer; WOH: Weight of Hammer; WOR: Weight of Rod
Nominal I.D. of Split Barrel Sampler	1 1/2 in	
Weight/type of Hammer on Drive Pipe	300 lb	
Weight/type of Hammer on Split Barrel	140 lb	
Drop of Hammer on Drive Pipe	in	
Core Size	in	

Approximate Change in Strata: _____ Inferred Change in Strata: _____

Soil descriptions represent a field identification after D. M. Burmister unless otherwise noted.

FIGURE 13

Nominal I.D. of Hole	in
Nominal I.D. of Split Barrel Sampler	1% in
Weight/type of Hammer on Drive Pipe	300 lb
Weight/type of Hammer on Split Barrel	140 lb
Drop of Hammer on Drive Pipe	in
Core Size	In

Pp: Pocket Penetrometer; WOH: Weight of Hammer; WOR: Weight of Rod

Soil descriptions represent a field identification after D. M. Burmister unless otherwise noted.

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[illegible]


Nominal I.D. of Hole	in
Nominal I.D. of Split Barrel Sampler	1 1/2 in
Weight/type of Hammer on Drive Pipe	300 lb
Weight/type of Hammer on Split Barrel	140 lb
Drop of Hammer on Drive Pipe	in
Core Size	in

The subsurface information shown hereon was obtained for the design and estimating purposes for our client. It is made available to authorized users only that they may have access to the same information available to our client. It is presented in good faith, but it is not intended as a substitute for investigations, interpretations or judgment of such authorized users. Information on the logs should not be relied upon without the geotechnical engineers recommendations contained in the report from which these logs were extracted.

Approximate Change in Strata: _____ Inferred Change in Strata: _____

Soil descriptions represent a field identification after D. M. Burmister unless otherwise noted.

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				PROJECT NAME:		CONCORD		BORING NO.		SB-15	
				LOCATION:		Monticello, NY		JOB NO.		5485	
BORING BY: GBI				DATE STARTED:		6/17/2008		GROUNDWATER TABLE DEPTH: 13'			
INSPECTOR: JZ				DATE COMPLETED:		6/17/2008		0 Hr. 13' Date 6/17/08 24 Hr. N/A Date N/A			
DEPTH (ft)	METHOD	SAMPLE No.	DEPTH		Blows on Spoon				REC (in)	SOIL DESCRIPTION AND STRATIFICATION	SYMBOL
			FROM (ft)	TO (ft)	0/6	6/12	12/18	18/24			
0											
	ss	1	0	2	19	28	24	31	18	6" Concrete	
										Fill: Brown/Red-brown/Gray c-f SAND, some c-f Gravel, little Silt	
5											
	ss	2	5	7	24	27	31	32	24	Red-Brown c-f SAND, some m-f Gravel, little Silt w/ pieces of Weathered Rock (some moisture/probable perched water)	
10											
	ss	3	10	12	43	44	50/2"	-	15	Same	
15											
	ss	4	15	17	51	64	50/1"	-	6	Same w/ Weathered Rock in tip	
20										Refusal on Rock	
										BORING COMPLETE AT 18.0 FEET	
25											
30											
35											
40											

Nominal I.D. of Hole	1 1/2 in
Nominal I.D. of Split Barrel Sampler	1 1/2 in
Weight/type of Hammer on Drive Pipe	300 lb
Weight/type of Hammer on Split Barrel	140 lb
Drop of Hammer on Drive Pipe	1 in
Core Size	1 in


The subsurface information shown hereon was obtained for the design and estimating purposes for our client. It is made available to authorized users only that they may have access to the same information available to our client. It is presented in good faith, but it is not intended as a substitute for investigations, interpretations or judgment of such authorized users. Information on the logs should not be relied upon without the geotechnical engineers recommendations contained in the report from which these logs were extracted.

Pp: Pocket Penetrometer; WOH: Weight of Hammer; WOR: Weight of Rod

Approximate Change in Strata: _____ Inferred Change in Strata: _____

Soil descriptions represent a field identification after D. M. Burmister unless otherwise noted.

FIGURE 16

				PROJECT NAME:		CONCORD		BORING NO.		SB-15	
				LOCATION:		Monticello, NY		JOB NO.		5485	
BORING BY: GBI				DATE STARTED:		6/17/2008		GROUNDWATER TABLE DEPTH: 13'			
INSPECTOR: JZ				DATE COMPLETED:		6/17/2008		0 Hr. 13'		Date 6/17/08 24 Hr. N/A Date N/A	
DEPTH (ft)	METHOD	SAMPLE No.	DEPTH		Blows on Spoon				REC (in)	SOIL DESCRIPTION AND STRATIFICATION	SYMBOL
			FROM (ft)	TO (ft)	0/6	6/12	12/18	18/24			
0	SS	1	0	2	19	28	24	31	18	6" Concrete	
5										Fill: Brown/Red-brown/Gray c-f SAND, some c-f Gravel, little Silt	
10	SS	2	5	7	24	27	31	32	24	Red-Brown c-f SAND, some m-f Gravel, little Silt w/ pieces of Weathered Rock (some moisture/probable perched water)	
15	SS	3	10	12	43	44	50/2"	-	15	Same	
20	SS	4	15	17	51	64	50/1"	-	6	Same w/ Weathered Rock in tip	
25										Refusal on Rock	
30										BORING COMPLETE AT 18.0 FBET	
35											
40											


Nominal I.D. of Hole	in	The subsurface information shown hereon was obtained for the design and estimating purposes for our client. It is made available to authorized users only that they may have access to the same information available to our client. It is presented in good faith, but it is not intended as a substitute for investigations, interpretations or judgment of such authorized users. Information on the logs should not be relied upon without the geotechnical engineers recommendations contained in the report from which these logs were extracted.
Nominal I.D. of Split Barrel Sampler	1 1/2 in	
Weight/type of Hammer on Drive Pipe	300 lb	
Weight/type of Hammer on Split Barrel	140 lb	
Drop of Hammer on Drive Pipe	in	
Core Size	in	

Pp: Pocket Penetrometer; WOH: Weight of Hammer; WOR: Weight of Rod

Approximate Change in Strata: _____ Inferred Change in Strata: _____

Soil descriptions represent a field identification after D. M. Burmister unless otherwise noted.

FIGURE 16

		PROJECT NAME:		CONCORD		BORING NO.		SB-16			
		LOCATION:		Monticello, NY		JOB NO.		5485			
BORING BY: GBI		DATE STARTED:		6/17/2008		GROUNDWATER TABLE DEPTH: NE					
INSPECTOR: JZ		DATE COMPLETED:		6/17/2008		0 Hr. N/A		Date N/A			
						24 Hr. N/A		Date N/A			
DEPTH (ft)	METHOD	SAMPLE No.	DEPTH		Blows on Spoon				REC (in)	SOIL DESCRIPTION AND STRATIFICATION	SYMBOL
			FROM (ft)	TO (ft)	0/6	6/12	12/18	18/24			
0											
	ss	1	0	2	4	3	1	2	6	Topsoil (wood in tip)	
5											
	ss	2	5	7	30	13	8	14	5	Fill: Pieces of Concrete and Wood	
10											
	ss	3	10	12	14	40	45	41	15	Red-brown c-f SAND, some c-f Gravel, little Silt, w/ pieces of Weathered Rock	
15											
	ss	4	15	17	9	17	27	21	18	Red-brown c-f SAND, some Silt, little c-f Gravel w/ Weathered Rock	
20											
	ss	5	20	22	23	34	27	40	24	Same	
25											
	ss	6	25	27	29	36	32	44	24	Same	
30										BORING COMPLETE AT 27.0 FEET	
35											
40											


Nominal I.D. of Hole	In	The subsurface information shown hereon was obtained for the design and estimating purposes for our client. It is made available to authorized users only that they may have access to the same information available to our client. It is presented in good faith, but it is not intended as a substitute for investigations, interpretations or judgment of such authorized users. Information on the logs should not be relied upon without the geotechnical engineers recommendations contained in the report from which these logs were extracted.
Nominal I.D. of Split Barrel Sampler	1% In	
Weight/type of Hammer on Drive Pipe	300 lb	
Weight/type of Hammer on Split Barrel	140 lb	
Drop of Hammer on Drive Pipe	In	
Core Size	In	

Pp: Pocket Penetrometer; WOH: Weight of Hammer; WOR: Weight of Rod

Approximate Change in Strata: _____ Inferred Change in Strata: _____

Soil descriptions represent a field identification after D. M. Burmister unless otherwise noted.

FIGURE 17

		PROJECT NAME:		CONCORD		MONITORING WELL NO.		SB-17	
		PROJECT LOCATION:		Monticello, NY		JOB NO.		5485	
		BORING BY: GBI		DATE STARTED		7/15/08		DEVELOPMENT PERIOD	
INSPECTOR: CDM		DATE COMPLETED		7/15/08		DEVELOPMENT METHOD		---	
NJ DEP PERMIT NO.: N/A		DATE DEVELOPED		---		DEVELOPMENT RATE		---	
						INSIDE CASING DIAMETER (in)		4	
						BOREHOLE DIAMETER (in)		6 5/8	
						INITIAL WATER LEVEL (ft):		31	

WELL CONSTRUCTION	DEPTH (ft)	Sample	Blows on Spoon				REC (in)	SOIL DESCRIPTION AND STRATIFICATION	P.I.D.
			0/8	6/12	12/18	18/24			
Depth (feet below grade)	0								
Top of Casing	0							0" Asphalt	
Ground Surface	0							Fill: Red-Brown SILT, little medium to fine Sand, little medium to fine Gravel	
Top of Riser	0								
Top of Seal	14.0							Fill: Gray coarse to fine SAND, little Silt, little coarse to fine Gravel, trace Asphalt	
Top of Sand Pack	16.0							...auger grinding (weathered Sandstone)	
Top of Screen	18.0							... Organic debris (wood chips)	
Bottom of Screen	38.0							Brown coarse to fine SAND, little Silt, little coarse to fine Gravel (weathered Sandstone)	
Bottom of Boring	39.0								


Remarks: Strong Petroleum odor in hollow-stem auger bit, starting around 30 ft below grade. MiniRAE registered around 70.0 PPM inside the casing while it was still in the ground.

Approximate Change in Strata: _____ Inferred Change in Strata: _____

BORING COMPLETE AT 39.0 FEET

The subsurface information shown hereon was obtained for the design and estimating purposes for our client. It is made available to authorized users only that they may have access to the same information available to our client. It is presented in good faith, but it is not intended as a substitute for investigations, interpretations or judgment of such authorized users. Information on the logs should not be relied upon without the geotechnical engineers recommendations contained in the report from which these logs were extracted. Soil descriptions represent a field identification after D. M. Burmister unless otherwise noted.

FIGURE 18

		PROJECT NAME:		CONCORD		BORING NO.		SB-18			
		LOCATION:		Monticello, NY		JOB NO.		5485			
BORING BY: GBI		DATE STARTED:				GROUNDWATER TABLE DEPTH: NE					
INSPECTOR: JZ		DATE COMPLETED:				0 Hr. N/A		Date N/A			
24 Hr. N/A		Date N/A									
DEPTH (ft)	METHOD	SAMPLE No.	DEPTH FROM TO (ft) (ft)		Blows on Spoon 0/6 6/12 12/18 18/24				REC (in)	SOIL DESCRIPTION AND STRATIFICATION	SYMBOL
0											
5											
10											
15											
20											
25											
30											
35											
40											

NOT DRILLED

Nominal I.D. of Hole	1 1/2 in
Nominal I.D. of Split Barrel Sampler	1 1/2 in
Weight/type of Hammer on Drive Pipe	300 lb
Weight/type of Hammer on Split Barrel	140 lb
Drop of Hammer on Drive Pipe	1 in
Core Size	1 in


The subsurface information shown hereon was obtained for the design and estimating purposes for our client. It is made available to authorized users only that they may have access to the same information available to our client. It is presented in good faith, but it is not intended as a substitute for investigations, interpretations or judgment of such authorized users. Information on the logs should not be relied upon without the geotechnical engineers recommendations contained in the report from which these logs were extracted.

Pp: Pocket Penetrometer; WOH: Weight of Hammer; WOR: Weight of Rod

Approximate Change in Strata: _____ Inferred Change in Strata: _____

Soil descriptions represent a field identification after D. M. Burmister unless otherwise noted.

FIGURE 19

			PROJECT NAME:		CONCORD		BORING NO.		SB-19		
			LOCATION:		Monticello, NY		JOB NO.		5485		
BORING BY: GBI			DATE STARTED:		6/17/2008		GROUNDWATER TABLE DEPTH: NE				
INSPECTOR: JZ			DATE COMPLETED:		6/17/2008		0 Hr. N/A Date N/A 24 Hr. N/A Date N/A				
DEPTH (ft)	METHOD	SAMPLE No.	DEPTH		Blows on Spoon				REC (in)	SOIL DESCRIPTION AND STRATIFICATION	SYMBOL
			FROM (ft)	TO (ft)	0/6	6/12	12/18	18/24			
0	SS	1	0.5	2.5	4	14	19	19	10	6" Asphalt	
										Top 5" : Red-brown c-f SAND, some c-f Gravel, little Silt	
										Bottom 5" : Gray c-f GRAVEL, some c-f Sand, trace Silt	
5											
	SS	2	5	7	15	23	29	38	15	Red-brown m-f SAND, some Silt, little m-f Gravel w/ Weathered Rock	
10											
	SS	3	10	12	8	15	20	16	18	Red-brown clayey SILT, little fine Sand, trace Gravel w/ layers of Red-brown c-f SAND, some c-f Gravel, little Silt	
15											
	SS	4	15	17	13	11	39	38	24	Red-brown c-f SAND, some c-f Gravel, little Silt w/ layers of Red-brown clayey SILT, little f Sand, trace Gravel	
20											
	SS	5	20	22	50/2"	-	-	-	0	Rock in tip (Weathered Rock)	
25										Refusal on Rock	
										BORING COMPLETE AT 23.0 FEET	
30											
35											
40											

Nominal I.D. of Hole	In	The subsurface information shown hereon was obtained for the design and estimating purposes for our client. It is made available to authorized users only that they may have access to the same information available to our client. It is presented in good faith, but it is not intended as a substitute for investigations, interpretations or judgment of such authorized users. Information on the logs should not be relied upon without the geotechnical engineers recommendations contained in the report from which these logs were extracted.
Nominal I.D. of Split Barrel Sampler	1 1/4 in	
Weight/type of Hammer on Drive Pipe	300 lb	
Weight/type of Hammer on Split Barrel	140 lb	
Drop of Hammer on Drive Pipe	In	
Core Size	In	

Pp: Pocket Penetrometer; WOH: Weight of Hammer; WOR: Weight of Rod

Approximate Change in Strata: _____ Inferred Change in Strata: _____


Soil descriptions represent a field identification after D. M. Burmister unless otherwise noted.

FIGURE 20

Nominal I.D. of Hole	1 1/2 in	<p>The subsurface information shown hereon was obtained for the design and estimating purposes for our client. It is made available to authorized users only that they may have access to the same information available to our client. It is presented in good faith, but it is not intended as a substitute for investigations, interpretations or judgment of such authorized users. Information on the logs should not be relied upon without the geotechnical engineers recommendations contained in the report from which these logs were extracted.</p> <p>Pp: Pocket Penetrometer; WOH: Weight of Hammer; WOR: Weight of Rod</p>
Nominal I.D. of Split Barrel Sampler	1 1/2 in	
Weight/type of Hammer on Drive Pipe	300 lb	
Weight/type of Hammer on Split Barrel	140 lb	
Drop of Hammer on Drive Pipe	10 in	
Core Size	1 1/2 in	

Soil descriptions represent a field identification after D. M. Burmister unless otherwise noted.

Page 1 of 1

				PROJECT NAME:		CONCORD		BORING NO.		SB-21		
				LOCATION:		Monticello, NY		JOB NO.		5485		
BORING BY: GBI				DATE STARTED:		7/15/2008		GROUNDWATER TABLE DEPTH: 15.0' b.g.				
INSPECTOR: CDM				DATE COMPLETED:		7/15/2008		0 Hr. N/A		Date N/A		
DEPTH (ft)	METHOD	SAMPLE No.	DEPTH FROM (ft)	TO (ft)	Blows on Spoon				REC (in)	SOIL DESCRIPTION AND STRATIFICATION		SYMBOL
0					0/6	6/12	12/18	18/24				
	SS	1	1	3	10	10	11	14	17	6" Asphalt (Road)		
										Fill: Brown coarse to fine SAND, little Silt, little coarse to fine Gravel, trace Asphalt		
5												
	SS	2	5	7	9	11	19	73	21	Brown coarse to fine SAND, little Silt, little coarse to fine Gravel		
10												
	SS	3	10	12	7	8	8	7	22	SAME		
15												
	SS	4	15	17	21	35	100/4"	-	12	Brown SILT and coarse to fine Sand (weathered Sandstone)		
20												
25										Refusal on Rock		
										BORING COMPLETE AT 19.5 FEET		
30												
35												
40												

Nominal I.D. of Hole	in
Nominal I.D. of Split Barrel Sampler	1 1/4 in
Weight/type of Hammer on Drive Pipe	300 lb
Weight/type of Hammer on Split Barrel	140 lb
Drop of Hammer on Drive Pipe	in
Core Size	in


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Pp: Pocket Penetrometer; WOH: Weight of Hammer; WOR: Weight of Rod

Approximate Change in Strata: _____ Inferred Change in Strata: _____

Soil descriptions represent a field identification after D. M. Burmister unless otherwise noted.

FIGURE 21

		PROJECT NAME:		CONCORD		BORING NO.		SB-22			
		LOCATION:		Monticello, NY		JOB NO.		5485			
BORING BY: GBI		DATE STARTED:		6/17/2008		GROUNDWATER TABLE DEPTH: 6'					
INSPECTOR: JZ		DATE COMPLETED:		6/17/2008		0 Hr. 6'		Date 6/17/08 24 Hr. N/A Date N/A			
DEPTH (ft)	METHOD	SAMPLE No.	DEPTH		Blows on Spoon				REC (in)	SOIL DESCRIPTION AND STRATIFICATION	SYMBOL
			FROM (ft)	TO (ft)	0/6	6/12	12/18	18/24			
0	SS	1	0	2	6	7	11	14	24	Fill: Topsoil / Roots / Grass (top 12") Brown c-f SAND, little Silt, trace Gravel	
5											
	SS	2	5	7	11	23	43	55	24	Brown c-f SAND, some c-f Gravel, little Silt	
10											
	SS	3	10	12	50/1"	-	-	-	0	No Recovery	
15										ROCK CORE: RUN 1 (12'-17') RECOVERY = 42"/ 60" = 70% RQD = 0"/ 60" = 0% Gray medium to fine grained, slightly weathered, medium strong SANDSTONE w/ very closely spaced fractures	
20										ROCK CORE: RUN 2 (17'-22') RECOVERY = 58"/ 60" = 96% RQD = 29"/ 60" = 48% Gray medium to fine grained, slightly weathered, medium strong SANDSTONE w/ very closely spaced fractures	
25										BORING COMPLETE AT 22.0 FEET	
30											
35											
40											

Nominal I.D. of Hole	In
Nominal I.D. of Split Barrel Sampler	1 3/4 in
Weight/type of Hammer on Drive Pipe	300 lb
Weight/type of Hammer on Split Barrel	140 lb
Drop of Hammer on Drive Pipe	In
Core Size	In


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Pp: Pocket Penetrometer; WOH: Weight of Hammer; WOR: Weight of Rod

Approximate Change in Strata: _____ Inferred Change in Strata: _____

Soil descriptions represent a field identification after D. M. Burmister unless otherwise noted.

FIGURE 23

				PROJECT NAME:		CONCORD		BORING NO.		SB-23	
				LOCATION:		Monticello, NY		JOB NO.		5485	
BORING BY: GBI				DATE STARTED:		7/10/2008		GROUNDWATER TABLE DEPTH: Approx. 16ft.			
INSPECTOR: CDM				DATE COMPLETED:		7/10/2008		0 Hr. N/A Date N/A 24 Hr. N/A Date N/A			
DEPTH (ft)	METHOD	SAMPLE No.	DEPTH		Blows on Spoon				REC (in)	SOIL DESCRIPTION AND STRATIFICATION	SYMBOL
			FROM (ft)	TO (ft)	0/6	6/12	12/18	18/24			
0											
	SS	1	0	2	4	8	6	15	20	6" Topsoil	
										Brown coarse to fine SAND, little Silt, trace Gravel	
5											
	SS	2	5	7	22	55	43	46	18	Red-Brown SILT, some fine Sand, trace Gravel	
10											
	SS	3	10	12	18	75/2"	-	-	6	SAMB	
15											
	SS	4	15	17	27	60	75/2"	-	10	SAME (Weathered Rock)	
									1m	ROCK CORE: RUN 1 (16.5'-21.5')	
									1m	RECOVERY = 32" / 60" = 53 %	
									1.25m	RQD = 0" / 60" = 0%	
									2m	Red-Brown coarse to fine grained, highly weathered SANDSTONE	
									1.5m		
									2.5m	ROCK CORE: RUN 2 (21.5'-23.5')	
									2m	RECOVERY = 19" / 24" = 79 %	
									2m	RQD = 6" / 24" = 25%	
25									1.75m	Red-Brown medium to fine grained, highly weathered	
									2.25m	SANDSTONE	
									2.5m	ROCK CORE: RUN 3 (23.5'-28.0')	
									.75m	RECOVERY = 26" / 54" = 48 %	
									1.75m	RQD = 6" / 54" = 11%	
30									2m	Red-Brown medium to fine grained, highly weathered	
									2m	SANDSTONE	
									2m	ROCK CORE: RUN 4 (28.0'-33.0')	
									2.5m	RECOVERY = 60" / 60" = 100 %	
										RQD = 35" / 60" = 58%	
										Red-Brown medium to fine grained, slightly weathered	
										SANDSTONE	
40										BORING COMPLETE AT 33.0'	

Nominal I.D. of Hole	In
Nominal I.D. of Split Barrel Sampler	1 1/2 In
Weight/type of Hammer on Drive Pipe	300 lb
Weight/type of Hammer on Split Barrel	140 lb
Drop of Hammer on Drive Pipe	In
Core Size	In


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Pp: Pocket Penetrometer; WOH: Weight of Hammer; WOR: Weight of Rod

Approximate Change in Strata: _____ Inferred Change in Strata: _____

Soil descriptions represent a field identification after D. M. Burmister unless otherwise noted.

FIGURE 24

				PROJECT NAME:		CONCORD		BORING NO.		SB-24	
				LOCATION:		Monticello, NY		JOB NO.		5485	
BORING BY: GBI				DATE STARTED:		7/10/2008		GROUNDWATER TABLE DEPTH: Approx. 16ft.			
INSPECTOR: CDM				DATE COMPLETED:		7/10/2008		0 Hr. N/A		Date N/A	
DEPTH (ft)		METHOD		SAMPLE No.		DEPTH FROM TO (ft) (ft)		Blows on Spoon 0/6 6/12 12/18 18/24 (in)		REC	
0										SOIL DESCRIPTION AND STRATIFICATION	
		SS		1		0 2		5 8 10 10		12	
										6" Topsoil	
										Brown coarse to fine SAND, little Silt, trace Gravel	
5											
		SS		2		5 7		44 52 75/5" -		10	
										Red-Brown SILT, little medium to fine Sand, little coarse to fine Gravel (weathered Sandstone)	
										... Weathered Rock (auger grinding)	
10											
		SS		3		10 12		69 72 75/4" -		12	
										SAME	
										Refusal on Rock	
15										BORING COMPLETE AT 12.0 FBET	
20											
25											
30											
35											
40											

Nominal I.D. of Hole	in
Nominal I.D. of Split Barrel Sampler	1 1/4 in
Weight/type of Hammer on Drive Pipe	300 lb
Weight/type of Hammer on Split Barrel	140 lb
Drop of Hammer on Drive Pipe	in
Core Size	in


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Pp: Pocket Penetrometer; WOH: Weight of Hammer; WOR: Weight of Rod

Approximate Change in Strata: _____ Inferred Change in Strata: _____

Soil descriptions represent a field identification after D. M. Burmister unless otherwise noted.

FIGURE 25

				PROJECT NAME:		CONCORD		BORING NO.		SB-25	
				LOCATION:		Monticello, NY		JOB NO.		5485	
BORING BY: GBI				DATE STARTED:		7/10/2008		GROUNDWATER TABLE DEPTH: NE			
INSPECTOR: CDM				DATE COMPLETED:		7/10/2008		0 Hr. N/A Date N/A 24 Hr. N/A Date N/A			
DEPTH (ft)	METHOD	SAMPLE No.	DEPTH		Blows on Spoon				REC (in)	SOIL DESCRIPTION AND STRATIFICATION	SYMBOL
			FROM (ft)	TO (ft)	0/6	6/12	12/18	18/24			
0	SS	1	0	2	4	8	8	30	8	4" Topsoil	
5										Brown medium to fine SAND, little Silt, trace Gravel (weathered Sandstone)	
10	SS	2	5	7	31	43	44	50	18	SAME	
15	SS	3	10	12	28	33	37	41	20	Red-Brown SILT, little medium to fine Sand, little coarse to fine Gravel (Gray weathered Sandstone)	
20	SS	4	15	17	22	31	43	75/3"	19	Red-Brown SILT, little medium to fine Sand, trace Gravel (Gray weathered Sandstone)	
25									1.5m	ROCK CORE: RUN 1 (18.5'-23.5')	
									1m	RECOVERY = 60"/60" = 100%	
									2m	RQD = 49"/60" = 82%	
									2.25m	Gray coarse grained, slightly weathered SANDSTONE	
									2m		
30										BORING COMPLETE AT 23.5 FEET	
35											
40											

Nominal I.D. of Hole	in
Nominal I.D. of Split Barrel Sampler	1 1/2 in
Weight/type of Hammer on Drive Pipe	300 lb
Weight/type of Hammer on Split Barrel	140 lb
Drop of Hammer on Drive Pipe	in
Core Size	in

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Pp: Pocket Penetrometer; WOH: Weight of Hammer; WOR: Weight of Rod

Approximate Change in Strata: _____ Inferred Change in Strata: _____

Soil descriptions represent a field identification after D. M. Burmister unless otherwise noted.

FIGURE 26



PROJECT NAME:

CONCORD

BORING NO.

SB-26

LOCATION:

Monticello, NY

JOB NO.

5485

BORING BY: GBI

DATE STARTED:

7/10/2008

GROUND ELEVATION:

1500±

INSPECTOR: CDM

DATE COMPLETED:

7/10/2008

GROUNDWATER TABLE DEPTH: NE

0 Hr. N/A

Date N/A

24 Hr. N/A

Date N/A

DEPTH

(ft)

METHOD

SAMPLE No.

DEPTH

FROM

TO

(ft)

(ft)

Blows on Spoon

REC

SOIL DESCRIPTION AND STRATIFICATION

SYMBOL

0

5

10

15

20

25

30

35

40

SS

1

5

7

10

9

8

13

6

SS

2

10

12

38

75/1"

-

-

10

1.25m

1.25m

1m

1m

1.5m

1.5m

1.5m

1.25m

1m

1m

Fill: Misc. C+D debris (concrete, cinder blocks, rebar etc.)

Brown coarse to fine SAND, little Silt, little coarse to fine Gravel

Brown SILT, little coarse to fine Sand, trace Gravel (Gray weathered Sandstone)

ROCK CORE: RUN 1 (12'-17')

RECOVERY = 33"/60" = 55%

RQD = 4"/60" = 7%

Gray coarse grained, slightly weathered SANDSTONE

ROCK CORE: RUN 2 (17'-22')

RECOVERY = 58"/60" = 97%

RQD = 29.5"/60" = 49%

Gray coarse grained, slightly weathered SANDSTONE

BORING COMPLETE AT 22.0 FEET

Nominal I.D. of Hole	in
Nominal I.D. of Split Barrel Sampler	1 1/2 in
Weight/type of Hammer on Drive Pipe	300 lb
Weight/type of Hammer on Split Barrel	140 lb
Drop of Hammer on Drive Pipe	in
Core Size	in

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Pp: Pocket Penetrometer; WOH: Weight of Hammer; WOR: Weight of Rod

Approximate Change in Strata: _____

Inferred Change in Strata: _____

Soil descriptions represent a field identification after D. M. Burmister unless otherwise noted.

FIGURE 27

			PROJECT NAME:		CONCORD		BORING NO.		SB-27		
			LOCATION:		Monticello, NY		JOB NO.		5485		
BORING BY: GBI			DATE STARTED:		7/11/2008		GROUNDWATER TABLE DEPTH: NE		1497 ±		
INSPECTOR: CDM			DATE COMPLETED:		7/11/2008		0 Hr. N/A		Date N/A		
							24 Hr. N/A		Date N/A		
DEPTH (ft)	METHOD	SAMPLE No.	DEPTH		Blows on Spoon				REC (in)	SOIL DESCRIPTION AND STRATIFICATION	SYMBOL
			FROM (ft)	TO (ft)	0/6	6/12	12/18	18/24			
0											
	ss	1	0	2	26	31	47	40	12"	Red-brown/Orange-brown mottled coarse to fine SAND, little Silt, little coarse to fine Gravel	
5											
	ss	2	5	7	46	75/2"	-	-	6"	Brown coarse to fine SAND and coarse Gravel, trace Silt	
										... Auger grinding (weathered Sandstone)	
10											
									1m	ROCK CORE: RUN 1 (9'-14')	
									1m	RECOVERY = 60"/60" = 100%	
									1.25m	RQD = 34"/60" = 57%	
15									1m	Gray coarse grained, slightly weathered SANDSTONE	
									1m		
										BORING COMPLETE AT 14.0 FBET	
20											
25											
30											
35											
40											

Nominal I.D. of Hole	in
Nominal I.D. of Split Barrel Sampler	1 1/2 in
Weight/type of Hammer on Drive Pipe	300 lb
Weight/type of Hammer on Split Barrel	140 lb
Drop of Hammer on Drive Pipe	in
Core Size	in

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Pp: Pocket Penetrometer; WOH: Weight of Hammer; WOR: Weight of Rod

Approximate Change in Strata: _____

Inferred Change in Strata: _____

Soil descriptions represent a field identification after D. M. Burmister unless otherwise noted.

FIGURE 28



PROJECT NAME: CONCORD				BORING NO.: SB-28							
LOCATION: Monticello, NY				JOB NO.: 5485							
DATE STARTED: 7/14/2008				GROUND ELEVATION: 1492 ±							
DATE COMPLETED: 7/14/2008				GROUNDWATER TABLE DEPTH: NE							
BORING BY: GBI				0 Hr. N/A							
INSPECTOR: CDM				Date N/A							
24 Hr. N/A				Date N/A							
DEPTH (ft)	METHOD	SAMPLE No.	DEPTH		Blows on Spoon				REC (in)	SOIL DESCRIPTION AND STRATIFICATION	SYMBOL
			FROM (ft)	TO (ft)	0/6	6/12	12/18	18/24			
0											
	ss	1	0	2	9	17	22	15	12	4" Topsoil	
										Brown coarse to fine SAND, little Silt, little coarse to fine Gravel	
5											
	ss	2	5	7	17	75/5"	-	-	11	Red-brown coarse to fine SAND, little Silt, little coarse to fine Gravel (weathered SANDSTONE)	
10											
	ss	3	10	12	20	41	52	32	12	Same	
										... Auger grinding (weathered SANDSTONE)	
15											
										1m ROCK CORE: RUN1 (13.5'-18.5')	
										1.25m RECOVERY = 51"/60" = 85%	
										1.25m RQD = 26"/60" = 43%	
										1.25m Gray coarse to fine grained, slightly weathered SANDSTONE	
20											
										BORING COMPLETE AT 18.5 FBET	
25											
30											
35											
40											

Nominal I.D. of Hole	In
Nominal I.D. of Split Barrel Sampler	1 1/4 In
Weight/type of Hammer on Drive Pipe	300 lb
Weight/type of Hammer on Split Barrel	140 lb
Drop of Hammer on Drive Pipe	In
Core Size	In

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Pp: Pocket Penetrometer; WOH: Weight of Hammer; WOR: Weight of Rod

Soil descriptions represent a field identification after D. M. Burmister unless otherwise noted. Approximate Change in Strata: _____ Inferred Change in Strata: _____

FIGURE 29



PROJECT NAME: CONCORD				BORING NO.: SB-29							
LOCATION: Monticello, NY				JOB NO.: 5485							
BORING BY: GBI				DATE STARTED: 7/14/2008							
INSPECTOR: CDM				DATE COMPLETED: 7/14/2008							
				GROUNDWATER TABLE DEPTH: NE							
				0 Hr. N/A Date N/A 24 Hr. N/A Date N/A							
DEPTH (ft)	METHOD	SAMPLE No.	DEPTH		Blows on Spoon				REC (in)	SOIL DESCRIPTION AND STRATIFICATION	SYMBOL
			FROM (ft)	TO (ft)	0/6	6/12	12/18	18/24			
0										4" Asphalt	
										Fill: Brown coarse to fine SAND, some coarse to fine Gravel, little Silt, trace Asphalt	
5											
	ss	1	5	7	16	23	29	50/1"	19	Red-Brown medium to fine SAND, little Silt, little medium to fine Gravel	
10											
	ss	2	10	12	29	54	75/4"	-	12	SAME	
15										...auger grinding (weathered sandstone)	
	ss	3	15	17	30	75/1"	-	-	6	Red-Brown/Orange mottled coarse to fine SAND, little Silt, little medium to fine Gravel.	
20										BORING COMPLETE AT 17.0 FEET	
25											
30											
35											
40											

Nominal I.D. of Hole	1 in
Nominal I.D. of Split Barrel Sampler	1 1/4 in
Weight/type of Hammer on Drive Pipe	300 lb
Weight/type of Hammer on Split Barrel	140 lb
Drop of Hammer on Drive Pipe	1 in
Core Size	1 in

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Pp: Pocket Penetrometer; WOH: Weight of Hammer; WOR: Weight of Rod

Approximate Change in Strata: _____ Inferred Change in Strata: _____

Soil descriptions represent a field identification after D. M. Burmister unless otherwise noted.

FIGURE 30

THE CONCORD GEOPROBE LOGS

PROJECT NO. <u>7180</u>	PROJECT <u>Concord</u>	GEOPROBE NO. <u>OU-1B-4</u>
LOCATION <u>SEE FIGURE 1</u>	APPROX. ELEV. <u>1528.52</u>	INSPECTED BY <u>JZ/RF/CDM</u>
WATER OBSERVATION <u>Not Encountered</u>		DATE EXCAVATED <u>9/2/2008</u>

DEPTH FT.	DESCRIPTION / SOIL CLASSIFICATION	RECOVERY / P.I.D.(ppm)
0 —	0-.25': TOPSOIL .25"-3.6': Red-brown medium to fine SAND, little Silt, little Gravel with little Cobbles OU-1B-4A (.9'-2.9') 12:24	R = 43" P.I.D. = 0.0(0-5')
—		
1 —		
—		
2 —		
—		
3 —		
—		
4 —		
—		
5 —	5'-10': Same	R = 44" P.I.D. = 0.0(5'-10')
—		
6 —		
—		
7 —		
—		
8 —		
—		
9 —		
—		
10 —	10'-15': Red-brown medium to fine SAND, little Silt	R = 60" P.I.D. = 0.0(10'-14')
—		
11 —		
—		
12 —		
—		
13 —		
—		
14 —		

Time: 9:00 A.M. - 12:30 P.M.

SESI CONSULTING ENGINEERS, PC

Fig. #

1 of 3

PROJECT NO. <u>7180</u>	PROJECT <u>Concord</u>	GEOPROBE NO. <u>OU-1B-4</u>
LOCATION <u>SEE FIGURE 1</u>	APPROX. ELEV. _____	INSPECTED BY <u>JZ/RF/CDM</u>
WATER OBSERVATION <u>Not Encountered</u>		DATE EXCAVATED <u>9/2/2008</u>

DEPTH FT.	DESCRIPTION / SOIL CLASSIFICATION	RECOVERY / P.I.D.(ppm)
14		
15		
16		R = 60"
17		P.I.D = 0.0(15-20')
18	15-20': Same w/ Cobbles	
19		
20		
21	20'-25': Same	R = 60"
22		P.I.D. = 0.0(20.25')
23		
24		
25		
26	24'-27': Red-Brown medium to fine SAND, some Silt	R = 60"
27		P.I.D. = 0.0(25-30')
28		

Time: 9:00 A.M - 12:30 P.M

SESI CONSULTING ENGINEERS, PC

Fig. #

2 of 3

PROJECT NO. <u>7180</u>	PROJECT <u>Concord</u>	GEOPROBE NO. <u>OU-1B-4</u>
LOCATION <u>SEE FIGURE 1</u>	APPROX. ELEV. _____	INSPECTED BY <u>JZ/RF/CDM</u>
WATER OBSERVATION <u>moisture at 36.4'</u>		DATE EXCAVATED <u>9/2/2008</u>

DEPTH FT.	DESCRIPTION / SOIL CLASSIFICATION	RECOVERY / P.I.D.(ppm)
28	27'-30': Same	R = 58"
29		P.I.D. = 0.0(27'-30')
30		
31	End of Geo-Probe at 30 Feet	
32		
33		
34		
35		
36		
37		
38		
39		
40		
41		
42		

Time: 9:00 A.M - 12:30 P.M

SESI CONSULTING ENGINEERS, PC

Fig. #

3 of 3

PROJECT NO. 7180 PROJECT Concord GEOPROBE NO. OU-1B-5
 LOCATION SEE FIGURE 1 APPROX. ELEV. 1529.58± INSPECTED BY JZ/RF/CDM
 WATER OBSERVATION Not Encountered DATE EXCAVATED 8/20/2008

DEPTH FT.	DESCRIPTION / SOIL CLASSIFICATION	RECOVERY / P.I.D.(ppm)
0 —	0-6": Asphalt	R = 35"
—	6"-1.5': Red-brown medium to fine SAND, some medium to fine Gravel	P.I.D = 1798(2")
1 —	1.5-2.9': Brown medium to fine SAND, little Silt, trace Gravel	P.I.D = 2285(10")
—		P.I.D = 1640(2.75')
2 —	OU-1B-5A (6"-1.5') 18:34	
—		
3 —		
—		
4 —		
—		
5 —		
—	5-7.4': Same	R = 46"
6 —	7.4-8.25': Weathered Sandstone w/Asphalt	P.I.D = 1100(6')
—	8.25-8.8': Red-brown medium to fine SAND, some Silt, trace Gravel	P.I.D = 800(6.8')
7 —		P.I.D = 1400(7.5')
—		P.I.D = 500(8.5')
8 —		
—		
9 —		
—		
10 —		
—	10-10.5': Red brown medium to fine SAND, some medium to fine Gravel, little Silt	R = 39"
11 —	10.5-11.3': Brown medium to fine SAND, trace Gravel, trace Silt	P.I.D = 482(10.75')
—	11.3-12.8': Red-brown medium to fine SAND and Silt, little Gravel	P.I.D = 204(11.5')
12 —	12.8-13.25': Red-brown SILT, and Siltstone	P.I.D = 125(12')
—		P.I.D = 93.1(12.6')
13 —	OU-1B-5B (11.8-12.8') 18:45	P.I.D = 77(13.1')
—		
14 —		

Time: 9:45 am - 6:45 pm

SESI CONSULTING ENGINEERS, PC

Fig. #

1 of 2

PROJECT NO. <u>7180</u>	PROJECT <u>Concord</u>	GEOPROBE NO. <u>OU-1B-5</u>
LOCATION <u>SEE FIGURE 1</u>	APPROX. ELEV. <u>1529.58'±</u>	INSPECTED BY <u>JZ/RF/CDM</u>
WATER OBSERVATION <u>Not Encountered</u>		DATE EXCAVATED <u>8/20/2008</u>

DEPTH FT.	DESCRIPTION / SOIL CLASSIFICATION	RECOVERY / P.I.D.(ppm)
14		
15		
16	End of Geo-Probe at 15 Feet REFUSAL AT 15 FEET	
17		
18		
19		
20		
21		
22		
23		
24		
25		
26		
27		
28		

Time: 9:45 am - 6:45 pm

SESI CONSULTING ENGINEERS, PC

Fig. #

2 of 2

PROJECT NO. <u>7180</u>	PROJECT <u>Concord</u>	GEOPROBE NO. <u>OU-1B-6</u>
LOCATION <u>SEE FIGURE 1</u>	APPROX. ELEV. <u>1530.35'±</u>	INSPECTED BY <u>JZ/RF/CDM</u>
WATER OBSERVATION <u>Not Encountered</u>		DATE EXCAVATED <u>8/27/2008</u>

DEPTH FT.	DESCRIPTION / SOIL CLASSIFICATION	RECOVERY / P.I.D.(ppm)
0—	0-.8': Asphalt, Concrete and Debris	R = 52"
—	.8'-4.3': Red-brown SILT, some medium to fine Sand, little Gravel	P.I.D. = 155.0(.7')
1—		P.I.D. = 550.0(1.8')
—		P.I.D. = 800.0(2.8')
2—	OU-1B-6A (.8-1.8') 19:40	P.I.D. = 330.0(3.8')
—		
3—		
—		
4—		
—		
5—		
—		
6—	5'-9.6': Same	R = 55"
—		P.I.D. = 367.0(5.7')
7—		P.I.D. = 235.0(6.7')
—		P.I.D. = 497.0(7.0')
8—		P.I.D. = 440.0(9.1')
—		
9—		
—		
10—		
—		
11—	10'-14': Same	R = 53"
—		P.I.D. = 990.0(10.3')
12—		P.I.D. = 100(11.7')
—		P.I.D. = 77.0(12.5')
13—		P.I.D. = 43.8(13.9')
—		
14—		

Time: 11:15a.m.-8:00p.m.

SESI CONSULTING ENGINEERS, PC

Fig. #

1 of 3

PROJECT NO. <u>7180</u>	PROJECT <u>Concord</u>	GEOPROBE NO. <u>OU-1B-6</u>
LOCATION <u>SEE FIGURE 1</u>	APPROX. ELEV. <u>1530.35'±</u>	INSPECTED BY <u>JZ/RF/CDM</u>
WATER OBSERVATION <u>Not Encountered</u>		DATE EXCAVATED <u>8/27/2008</u>

DEPTH FT.	DESCRIPTION / SOIL CLASSIFICATION	RECOVERY / P.I.D.(ppm)
14		R = 56"
15	14'-17': Same	P.I.D. = 127.0(14.2') P.I.D. = 1100(14.7')
16	16.2'-16.4': Weathered Sandstone	P.I.D. = 98.4(16.0') P.I.D. = 59.5(16.56')
17		R = 54"
18	17'-20': Same	P.I.D. = 75.9(17.67') P.I.D. = 1090.0(18.3') P.I.D. = 155(19.0') P.I.D. = 52.4(19.7')
20		R = 58"
21	20'-23': Same	P.I.D. = 144.0(20.1') P.I.D. = 1215.0(20.7') P.I.D. = 108.0(21.5') P.I.D. = 73.1(22.2') P.I.D. = 56.7(22.7')
23		R = 55"
24	23'-25': Same	P.I.D. = 10.9(23.2') P.I.D. = 45.7(23.7') P.I.D. = 41.1(26.4') P.I.D. = 145.0(24.1')
25		R = 54"
26	25'-28': Same	P.I.D. = 23.4(25.2') P.I.D. = 49.9(25.7') P.I.D. = 84.7(26.6') P.I.D. = 96.6(27.2') P.I.D. = 11.3(27.9')
27		
28		

Time: 11:15a.m.-8:00p.m.

SESI CONSULTING ENGINEERS, PC

Fig. #

2 of 3

PROJECT NO. <u>7180</u>	PROJECT <u>Concord</u>	GEOPROBE NO. <u>OU-1B-6</u>
LOCATION <u>SEE FIGURE 1</u>	APPROX. ELEV. <u>1530.35'±</u>	INSPECTED BY <u>JZ/RF/CDM</u>
WATER OBSERVATION <u>Not Encountered</u>		DATE EXCAVATED <u>8/27/2008</u>

DEPTH FT.	DESCRIPTION / SOIL CLASSIFICATION	RECOVERY / P.I.D.(ppm)
28 —		
—	28'-30': Same R = 54" P.I.D. = 54.5(31.2')	P.I.D. = 59.4(28.1')
29 —	29.3'-29.6': with Weathered Sandstone P.I.D. = 43.5(31.6')	P.I.D. = 41.9(29.1')
—	OU-1B-6C (28.3'-29.5') 20:00 P.I.D. = 10.1(32.4')	P.I.D. = 89.8(29.4')
30 —		
—	End of Geo-Probe at 30 Feet	
31 —		
—		
32 —		
—		
33 —		
—		
34 —		
—		
35 —		
—		
36 —		
—		
37 —		
—		
38 —		
—		
39 —		
—		
40 —		
—		
41 —		
—		
42 —		

Time: 11:15a.m.-8:00p.m.

SESI CONSULTING ENGINEERS, PC

Fig. #

3 of 3

PROJECT NO. <u>7180</u>	PROJECT <u>Concord</u>	GEOPROBE NO. <u>OU-1B-7</u>
LOCATION <u>SEE FIGURE 1</u>	APPROX. ELEV. <u>1530.62[±]</u>	INSPECTED BY <u>JZ/RF/CDM</u>
WATER OBSERVATION <u>Not Encountered</u>		DATE EXCAVATED <u>8/27/2008</u>

DEPTH FT.	DESCRIPTION / SOIL CLASSIFICATION	RECOVERY / P.I.D.(ppm)
0—	0-.5': Asphalt, Concrete and Debris	R = 53"
—	.5'-4.5': Red-brown medium to fine SAND, some Silt,	P.I.D. = 23.5(1')
1—	some medium to fine Gravel	P.I.D. = 61.6(2')
—		P.I.D. = 21.5(2.83')
2—	OU-1B-7A (.5-1.5') 19:00	P.I.D. = 22.0(3.5')
—		
3—		
—		
4—		
—		
5—		
—		
6—	5'-10': Red-Brown medium to fine SAND, some Silt	R = 53"
—	some medium to fine Gravel	P.I.D. = 7.2(6')
7—	5.8'-6.2': with Sandstone	P.I.D. = 5.6(7.5')
—		P.I.D. = 4.7(8.5')
8—		P.I.D. = 4.1(9.5')
—		
9—	8.6'-9.2': with Sandstone	
—		
10—		
—		
11—	10'-14': Same	R = 53"
—		P.I.D. = 5.5(11')
12—		P.I.D. = 5.6(12')
—		P.I.D. = 5.6(36')
13—	13.5'-14': Weathered Sandstone	P.I.D. = 5.3(14')
—		
14—		

Time: 1:30p.m.-7:15p.m.

SESI CONSULTING ENGINEERS, PC

Fig. #

1 of 2

PROJECT NO. <u>7180</u>	PROJECT <u>Concord</u>	GEOPROBE NO. <u>OU-1B-7</u>
LOCATION <u>SEE FIGURE 1</u>	APPROX. ELEV. <u>1530.62'±</u>	INSPECTED BY <u>JZ/RF/CDM</u>
WATER OBSERVATION <u>Not Encountered</u>		DATE EXCAVATED <u>8/27/2008</u>

DEPTH FT.	DESCRIPTION / SOIL CLASSIFICATION	RECOVERY / P.I.D.(ppm)
14	14'-17': Same	R = 53"
15		P.I.D. = 4.5(14.5')
16		P.I.D. = 4.3(15.3')
17		P.I.D. = 4.0(15.9')
18		P.I.D. = 3.8(16.6')
19	17'-20: Same	R = 58"
20		P.I.D. = 2.9(17.4')
21		P.I.D. = 2.7(17.9')
22		P.I.D. = 2.5(18.7')
23		P.I.D. = 2.2(19.3')
24	20'-23': Same	P.I.D. = 1.8(19.7")
25		R = 54"
26		P.I.D. = 1.1(20.4')
27		P.I.D. = 1.0(20.9')
28		P.I.D. = 0.7(21.3')
29	OU-1B-7B (23.5' - 24.5') 19:15	P.I.D. = 0.5(22.3')
30		P.I.D. = 0.3(22.8')
31		
32		
33		
34	End of Geo-Probe at 23 Feet REFUSAL AT 23 FEET	
35		
36		
37		
38		

Time: 1:30p.m.-7:15p.m.

SESI CONSULTING ENGINEERS, PC

Fig. #

2 of 2

PROJECT NO. <u>7180</u>	PROJECT <u>Concord</u>	GEOPROBE NO. <u>OU-1B-8</u>
LOCATION <u>SEE FIGURE 1</u>	APPROX. ELEV. <u>1530.62'±</u>	INSPECTED BY <u>JZ/RF/CDM</u>
WATER OBSERVATION <u>Not Encountered</u>		DATE EXCAVATED <u>8/27/2008</u>

DEPTH FT.	DESCRIPTION / SOIL CLASSIFICATION	RECOVERY / P.I.D.(ppm)
0 —	0-.5': Asphalt	R = 46"
—	.5'-5': Red-brown medium to fine SAND, little Silt,	P.I.D = 2.8(1')
1 —	little medium to fine Gravel	P.I.D. = 0.0(1'-5')
—		
2 —	OU-1B-8A (.5'-1.5') 19:25	
—		
3 —		
—		
4 —		
—		
5 —		
—		
6 —	5'-10': Same	R = 54"
—		P.I.D = 0.0(5'-10')
7 —		
—		
8 —		
—		
9 —		
—		
10 —		
—		
11 —	10'-14': Same	R = 53"
—		P.I.D = 0.0(10'-14')
12 —		
—		
13 —		
—		
14 —		

Time: 3:15p.m.-7:30p.m.

SESI CONSULTING ENGINEERS, PC

Fig. #

1 of 3

PROJECT NO. <u>7180</u>	PROJECT <u>Concord</u>	GEOPROBE NO. <u>OU-1B-8</u>
LOCATION <u>SEE FIGURE 1</u>	APPROX. ELEV. <u>1530.62'±</u>	INSPECTED BY <u>JZ/RF/CDM</u>
WATER OBSERVATION <u>Not Encountered</u>		DATE EXCAVATED <u>8/27/2008</u>

DEPTH FT.	DESCRIPTION / SOIL CLASSIFICATION	RECOVERY / P.I.D.(ppm)
14		
15	14'-18': Red-Brown medium to fine SAND, little Silt, little medium to fine Gravel	R = 53" P.I.D. = 0.0(14'-18')
16	15.1'-15.8': Sandstone	
17		
18		
19	18'-21': Same	R = 57" P.I.D. = 0.0(18'-21')
20		
21		
22	21'-23.7': Same	R = 58" P.I.D. = 0.0(21'-25')
23		
24	24.25'-25.0': Red-Brown SILT and Siltstone	
25		
26	25'-28': Same Weathered Sandstone at (26.9'-27.2') and (27.5'-27.9')	R = 57" P.I.D. = 0.0(25'-28')
27		
28		

Time: 3:15p.m.-7:30p.m.

SESI CONSULTING ENGINEERS, PC

Fig. #

2 of 3

PROJECT NO. <u>7180</u>	PROJECT <u>Concord</u>	GEOPROBE NO. <u>OU-1B-8</u>
LOCATION <u>SEE FIGURE 1</u>	APPROX. ELEV. <u>1530.623'+</u>	INSPECTED BY <u>JZ/RF/CDM</u>
WATER OBSERVATION <u>Not Encountered</u>		DATE EXCAVATED <u>8/27/2008</u>

DEPTH FT.	DESCRIPTION / SOIL CLASSIFICATION	RECOVERY / P.I.D.(ppm)
28 —	28'-30': Same OU-1B-8B (28.0'-29.9') 19:30	R = 53" P.I.D. = 0.0(28'-30')
29 —		
30 —		
31 —	End of Geo-Probe at 30 Feet	
32 —		
33 —		
34 —		
35 —		
36 —		
37 —		
38 —		
39 —		
40 —		
41 —		
42 —		

Time: 3:15p.m.-7:30p.m.

SESI CONSULTING ENGINEERS, PC

Fig. #

3 of 3

PROJECT NO. <u>7180</u>	PROJECT <u>Concord</u>	GEOPROBE NO. <u>OU-1B-10</u>
LOCATION <u>SEE FIGURE 1</u>	APPROX. ELEV. <u>1531.14'±</u>	INSPECTED BY <u>RF/CDM</u>
WATER OBSERVATION <u>Not Encountered</u>		DATE EXCAVATED <u>8/28/2008</u>

DEPTH FT.	DESCRIPTION / SOIL CLASSIFICATION	RECOVERY / P.I.D.(ppm)
0—	0-7" Brown coarse to fine SAND, trace Silt	R = 53"
—	7"-4.4': Red-brown medium to fine SAND, little Silt, little Gravel	P.I.D = 0.0(0.0'-5.0')
1—	2.6-3.1': Sandstone	
—		
2—		
—		
3—	OU-1B-10A (8"-2') 16:03	
—		
4—		
—		
5—		
—	5'-10': Same	R = 58"
6—		P.I.D = 0.0(5.0'-10.0')
—		
7—		
—		
8—		
—		
9—		
—		
10—		
—	10'-14': Same	R = 58"
11—		P.I.D = 0.0(10.0'-14.0')
—		
12—		
—		
13—		
—		
14—		

Time: 7:00am - 4:25pm

SESI CONSULTING ENGINEERS, PC

Fig.

1 of 3

PROJECT NO. <u>7180</u>	PROJECT <u>Concord</u>	GEOPROBE NO. <u>OU-1B-10</u>
LOCATION <u>SEE FIGURE 1</u>	APPROX. ELEV. <u>1531.14'±</u>	INSPECTED BY <u>RF/CDM</u>
WATER OBSERVATION <u>Not Encountered</u>		DATE EXCAVATED <u>8/28/2008</u>

DEPTH FT.	DESCRIPTION / SOIL CLASSIFICATION	RECOVERY / P.I.D.(ppm)
14	14'-17': Same	R = 54"
15	Sandstone: 15.6-16.2'	P.I.D = 0.0(14.0'-17.0')
16		
17	17'-20': Same	R = 51"
18	Sandstone: 19.5-19.6'	P.I.D = 0.0(17.0'-20.0')
19		
20	20'-24': Same	R = 57"
21	Very Dense: 23.5-23.8'	P.I.D = 0.0(10.0'-24.0')
22		
23		
24	24'-27': Same	R = 55"
25	26.08-26.4': Brown Coarse to fine SAND, little Silt, little Gravel	P.I.D 0.0(24.0'-27.0')
	26.3-26.5': Sandstone	
26		
27	27'-30': Same	R = 54"
28	OU-1B-10B(28.9-29.4'	P.I.D = 0.0(27.0'-30.0')

Time: 7:00am - 4:25pm

SESI CONSULTING ENGINEERS, PC

Fig.

2 of 3

PROJECT NO. 7180

PROJECT Concord

GEOPROBE NO.

OU-1B-10

LOCATION SEE FIGURE 1

APPROX. ELEV. 1531.14[±]

INSPECTED BY RF/CDM

WATER OBSERVATION

Not Encountered

DATE EXCAVATED 8/28/2008

DEPTH FT.	DESCRIPTION / SOIL CLASSIFICATION	RECOVERY / P.I.D.(ppm)
28 — — 29 — — 30 — —	28.6-28.8': Sandstone	
31 — — 32 — — 33 — — 34 — — 35 — — 36 — — 37 — — 38 — — 39 — — 40 — — 41 — — 42 —	End of Geo-Probe at 30 Feet	

Time: 7:00am - 4:25pm

SESI CONSULTING ENGINEERS, PC

Fig.

3 of 3

PROJECT NO. <u>7180</u>	PROJECT <u>Concord</u>	GEOPROBE NO. <u>OU-1B-11</u>
LOCATION <u>SEE FIGURE 1</u>	APPROX. ELEV. <u>1531.44'±</u>	INSPECTED BY <u>JZ/RF/CDM</u>
WATER OBSERVATION <u>Not Encountered</u>		DATE EXCAVATED <u>8/28/2008</u>

DEPTH FT.	DESCRIPTION / SOIL CLASSIFICATION	RECOVERY / P.I.D.(ppm)
0 —	0'-8': FILL: Light Brown medium to fine SAND, some medium to fine Gravel	R = 48"
—	little Silt	P.I.D = 0.0(0.0'-4.0')
1 —	.8'-4': Red-Brown medium to fine SAND, some medium to fine Gravel	
—	little Silt with weathered Sandstone at 1.4'-2.0'	
2 —	OU-1B-11A (.6'-2.9') 13:30	
—		
3 —		
—		
4 —		
—		
5 —		
—		
6 —	5'-10': Same	R = 54"
—		P.I.D = 0.0(5'-10')
7 —		
—		
8 —		
—		
9 —		
—		
10 —		
—		
11 —	10'-14': Same	R = 58"
—		P.I.D = 0.0(10'-14')
12 —		
—		
13 —		
—		
14 —		

Time: 9:00a.m.-2:00p.m.

SESI CONSULTING ENGINEERS, PC

Fig. #

1 of 3

PROJECT NO. <u>7180</u>	PROJECT <u>Concord</u>	GEOPROBE NO. <u>OU-1B-11</u>
LOCATION <u>SEE FIGURE 1</u>	APPROX. ELEV. <u>1531.44'±</u>	INSPECTED BY <u>JZ/RF/CDM</u>
WATER OBSERVATION <u>Not Encountered</u>		DATE EXCAVATED <u>8/28/2008</u>

DEPTH FT.	DESCRIPTION / SOIL CLASSIFICATION	RECOVERY / P.I.D.(ppm)
14	14'-17': Same	R = 56"
15		P.I.D. = 0.0(14'-17')
16		
17	17'-20': Same	R = 55"
18		P.I.D. = 0.0(17'-20')
19		
20	20'-24': Same	R = 56"
21		P.I.D. = 0.0(20'-24')
22		
23	24'-27': Same	R = 56"
24		P.I.D. = 0.0(24'-27')
25		
26		
27		
28		

Time: 9:00a.m.-2:00p.m.

SESI CONSULTING ENGINEERS, PC

Fig. #

2 of 3

PROJECT NO. 7180 PROJECT Concord GEOPROBE NO. OU-1B-11
 LOCATION SEE FIGURE 1 APPROX. ELEV. 1531.44'± INSPECTED BY JZ/RF/CDM
 WATER OBSERVATION Not Encountered DATE EXCAVATED 8/28/2008

DEPTH FT.	DESCRIPTION / SOIL CLASSIFICATION	RECOVERY / P.I.D.(ppm)
28— — 29— — 30— — 31— — 32— — 33— — 34— — 35— — 36— — 37— — 38— — 39— — 40— — 41— — 42—	27'-30': Same OU-1B-11B (29.6'-30.0') 14:00 End of Geo-Probe at 30 Feet	R = 53" P.I.D. = 0.0(27'-30')

Time: 9:00a.m.-2:00p.m.

SESI CONSULTING ENGINEERS, PC

Fig. #

3 of 3

PROJECT NO. <u>7180</u>	PROJECT <u>Concord</u>	GEOPROBE NO. <u>OU-1B-12</u>
LOCATION <u>SEE FIGURE 1</u>	APPROX. ELEV. <u>1531.15'±</u>	INSPECTED BY <u>RF/CDM</u>
WATER OBSERVATION <u>Not Encountered</u>		DATE EXCAVATED <u>8/28/2008</u>

DEPTH FT.	DESCRIPTION / SOIL CLASSIFICATION	RECOVERY / P.I.D.(ppm)
0 —	0-5" Asphalt	R = 38"
—	5"-3.1': Red-brown medium to fine SAND, little Silt, little Gravel,	P.I.D = 0.0(0.0'-5.0')
1 —	w/ weathered Sandstone 2.25-2.5'	
—		
2 —	OU-1B-12A (6"-1.5') 15:32	
—		
3 —		
—		
4 —		
—		
5 —		
—	5'-10': Same	R = 56"
6 —	Sandstone 5.4-5.6', 9.3-9.4'	P.I.D = 0.0(5.0'-10.0')
—		
7 —		
—		
8 —		
—		
9 —		
—		
10 —		
—	10'-14': Same	R = 56"
11 —		P.I.D = 0.0(10.0'-14.0')
—		
12 —		
—		
13 —		
—		
14 —		

Time: 11:00 am - 3:48 pm

SESI CONSULTING ENGINEERS, PC

Fig.

1 of 3

PROJECT NO. <u>7180</u>	PROJECT <u>Concord</u>	GEOPROBE NO. <u>OU-1B-12</u>
LOCATION <u>SEE FIGURE 1</u>	APPROX. ELEV. <u>1531.15'+</u>	INSPECTED BY <u>RF/CDM</u>
WATER OBSERVATION <u>Not Encountered</u>		DATE EXCAVATED <u>8/28/2008</u>

DEPTH FT.	DESCRIPTION / SOIL CLASSIFICATION	RECOVERY / P.I.D.(ppm)
14 —	14'-17': Same	R = 54" P.I.D = 0.0(14.0'-17.0')
15 —		
16 —		
17 —	17'-20': Same	R = 55" P.I.D = 0.0(17.0'-20.0')
18 —		
19 —		
20 —	20'-24': Same Sandstone: 22-22.25' Moisture: 21-24.4'	R = 53" P.I.D = 0.0(20.0'-24.0')
21 —		
22 —		
23 —	24'-27': Same	R = 57" P.I.D. = 0.0(24.0'-27.0')
24 —		
25 —		
26 —	27'-30': Same 29.9': Very Dense	R = 44" P.I.D = 0.0(27.0'-30.0')
27 —		
28 —		

Time: 11:00 am - 3:48 pm

SESI CONSULTING ENGINEERS, PC

Fig.

2 of 3

PROJECT NO. <u>7180</u>	PROJECT <u>Concord</u>	GEOPROBE NO. <u>OU-1B-12</u>
LOCATION <u>SEE FIGURE 1</u>	APPROX. ELEV. <u>1531.15'±</u>	INSPECTED BY <u>RF/CDM</u>
WATER OBSERVATION <u>Not Encountered</u>		DATE EXCAVATED <u>8/28/2008</u>

DEPTH FT.	DESCRIPTION / SOIL CLASSIFICATION	RECOVERY / P.I.D.(ppm)
28 —	OU-1B-12B(29.1-30.0')15:48	
29 —		
30 —	End of Geo-Probe at 30 Feet	
31 —		
32 —		
33 —		
34 —		
35 —		
36 —		
37 —		
38 —		
39 —		
40 —		
41 —		
42 —		

Time: 11:00 am - 3:48 pm

SESI CONSULTING ENGINEERS, PC

Fig.

3 of 3

PROJECT NO. <u>7180</u>	PROJECT <u>Concord</u>	GEOPROBE NO. <u>OU-1B-13</u>
LOCATION <u>SEE FIGURE 1</u>	APPROX. ELEV. <u>1530.98'±</u>	INSPECTED BY <u>JZ/RF/CDM</u>
WATER OBSERVATION <u>Not Encountered</u>	DATE EXCAVATED <u>8/28/2008</u>	

DEPTH FT.	DESCRIPTION / SOIL CLASSIFICATION	RECOVERY / P.I.D.(ppm)
0 — — 1 — — 2 — — 3 — — 4 — — 5 — —	0-.7': Asphalt .7'-1.1': Red-Brown coarse to fine SAND, little Silt, trace Gravel 1.1'-1.5': SANDSTONE 1.5'-2.9': Red-Brown medium to fine SAND, little Silt, little Gravel 1.9'-2.2': with Sandstone OU-1B-13A (1.2'-2.3') 16:35	R = 35" P.I.D. = 0.0(0.0'-5.0')
6 — — 7 — — 8 — — 9 — — 10 — —	5'-10': Same Sandstone at (6.0'-6.2') and (6.7'-7.0')	R = 58" P.I.D. = 0.0(5'-10')
11 — — 12 — — 13 — — 14 — —	10'-14': Same Sandstone at (10.8'-10.9') and (12.8'-12.9')	R = 58" P.I.D. = 0.0(10'-14')

Time: 1:00p.m.-5:02p.m.

SESI CONSULTING ENGINEERS, PC

Fig. #

1 of 3

PROJECT NO. <u>7180</u>	PROJECT <u>Concord</u>	GEOPROBE NO. <u>OU-1B-13</u>
LOCATION <u>SEE FIGURE 1</u>	APPROX. ELEV. <u>1530.98'+</u>	INSPECTED BY <u>JZ/RF/CDM</u>
WATER OBSERVATION <u>Not Encountered</u>		DATE EXCAVATED <u>8/28/2008</u>

DEPTH FT.	DESCRIPTION / SOIL CLASSIFICATION	RECOVERY / P.I.D.(ppm)
14		
15	14'-17': Same	R = 58"
16	14'-14.4': Light Brown medium to fine mottled SAND	P.I.D. = 0.0(14'-17')
17		
18	17'-20': Same	R = 57"
19		P.I.D. = 0.0(17'-20')
20		
21	20'-24': Same	R = 56"
22	17'-21.9': Moisture Encountered	P.I.D. = 0.0(21'-24')
23	Sandstone at (23'-23.3') and (23.8'-23.9')	
24		
25	24'-27': Same	R = 58"
26	24'-26': Moisture Encountered	P.I.D. = 0.0(24'-27')
27		
28		

Time: 1:00p.m.-5:02p.m.

SESI CONSULTING ENGINEERS, PC

Fig. #

2 of 3

PROJECT NO. <u>7180</u>		PROJECT <u>Concord</u>		GEOPROBE NO. <u>OU-1B-13</u>	
LOCATION <u>SEE FIGURE 1</u>		APPROX. ELEV. <u>1530.98'±</u>		INSPECTED BY <u>JZ/RF/CDM</u>	
WATER OBSERVATION <u>Not Encountered</u>		DATE EXCAVATED <u>8/28/2008</u>			

DEPTH FT.	DESCRIPTION / SOIL CLASSIFICATION	RECOVERY / P.I.D.(ppm)
28 —	27'-30': Same	
—	27'-29.1': Moisture Encountered	R = 46"
29 —	Sandstone at (29.1'-29.3')	P.I.D. = 0.0(27'-30')
—	OU-1B-13B (29.1'-29.4') 17:02	
30 —		
—	End of Geo-Probe at 30 Feet	
31 —		
—		
32 —		
—		
33 —		
—		
34 —		
—		
35 —		
—		
36 —		
—		
37 —		
—		
38 —		
—		
39 —		
—		
40 —		
—		
41 —		
—		
42 —		

Time: 1:00p.m.-5:02p.m.

SESI CONSULTING ENGINEERS, PC

Fig. #

3 of 3

PROJECT NO. <u>7180</u>	PROJECT <u>Concord</u>	GEOPROBE NO. <u>OU-1B-14</u>
LOCATION <u>SEE FIGURE 1</u>	APPROX. ELEV. <u>1536.62</u>	INSPECTED BY <u>JZ/GP</u>
WATER OBSERVATION <u>Not Encountered</u>		DATE EXCAVATED <u>8/29/2008</u>

DEPTH FT.	DESCRIPTION / SOIL CLASSIFICATION	RECOVERY / P.I.D.(ppm)
0 —	0-1': Topsoil 1-3.5': Tan/Red-brown medium to fine SAND, little Silt, little Gravel 3.5-4.6': Red-brown medium to fine SAND, little Silt, little Gravel	R = 56" P.I.D. = 0.0 (0-5')
—		
1 —		
—		
2 —	Sample: OU-1B-14A (1-2.5')	
—		
3 —		
—		
4 —		
—		
5 —		
—		
6 —	5-10': Same w/ Cobbles	R = 41" P.I.D. = 0.0(5-10')
—		
7 —		
—		
8 —		
—		
9 —		
—		
10 —	10-14': Same	R = 47"
—		
11 —		
—		
12 —	Sample: OU-1B-14B (10.8-11.6') 12:50	P.I.D. = 0.0(12.6') P.I.D. = 3.6(13.6')
—		
13 —		
—		
14 —		

Time: 11:00 A.M. - 12:30 P.M.

SESI CONSULTING ENGINEERS, PC

Fig.

1 of 3

PROJECT NO. <u>7180</u>	PROJECT <u>Concord</u>	GEOPROBE NO. <u>OU-1B-14</u>
LOCATION <u>SEE FIGURE 1</u>	APPROX. ELEV. <u>1536.62</u>	INSPECTED BY <u>JZ/GP</u>
WATER OBSERVATION <u>Not Encountered</u>		DATE EXCAVATED <u>8/29/2008</u>

DEPTH FT.	DESCRIPTION / SOIL CLASSIFICATION	RECOVERY / P.I.D.(ppm)
14		
—	14-15.8': Same	R = 53"
15	15.8-17': Gray WEATHERED SANDSTONE	P.I.D. = 30.4(15.3')
—	17-18.4': Red-brown medium to fine SAND, little Silt, little Gravel, w/ Cobbles	P.I.D. = 51.7(15.5')
16	moist/wet from 14.5-15.1'	P.I.D. = 483(15.6')
—		P.I.D. = 49.2(15.8')
17		P.I.D. = 0.0(16)
—	17-20': Same	P.I.D. = 0.0(17.3')
18		P.I.D. = 56(18.25')
—		R = 48"
19		P.I.D. = 17.7(17.1')
—		P.I.D. = 11.1(17.6')
20		P.I.D. = 7.7(18.4')
—	20-24': Same w/ frequent Cobbles	P.I.D. = 9.7(19.6')
21		P.I.D. = 3.5(19.9')
—		R = 56"
22		P.I.D. = 120(20.1')
—		P.I.D. = 167(21.5')
23		P.I.D. = 48.2(22.3')
—		P.I.D. = 7.4(23.1')
24		P.I.D. = 30.3(23.6')
—	24-27': Red-brown medium to fine SAND, some Silt, trace Gravel, moist	P.I.D. = 43.8(24.1')
25	from 24-26.3'	P.I.D. = 160(24.5')
—		R = 55.5"
26		P.I.D. = 102(24.25')
—		P.I.D. = 124(24.5')
27		P.I.D. = 84.2(25.25')
—	27-30': Same	P.I.D. = 118(26')
28	30-31.6': Red-brown medium to fine SAND, little Silt, little Gravel w/	P.I.D. = 105(27')

Time: 11:00 A.M. - 12:30 P.M.

SESI CONSULTING ENGINEERS, PC

Fig.

2 of 3

PROJECT NO. <u>7180</u>	PROJECT <u>Concord</u>	GEOPROBE NO. <u>OU-1B-14</u>
LOCATION <u>SEE FIGURE 1</u>	APPROX. ELEV. <u>1536.62</u>	INSPECTED BY <u>JZ/GP</u>
WATER OBSERVATION <u>Not Encountered</u>		DATE EXCAVATED <u>8/29/2008</u>

DEPTH FT.	DESCRIPTION / SOIL CLASSIFICATION	RECOVERY / P.I.D.(ppm)
28— —	Occasional Cobbles	P.I.D. = 97.6(28.1') P.I.D. = 41.5(28.5')
29— —	Sample: OU-1B-14C (27-28.5') 1:10	R = 55"
30— —	End of Geo-Probe at 30 Feet	P.I.D. = 227(27.4')
31— —		P.I.D. = 53.3(28.5')
32— —		P.I.D. = 155(29.1')
33— —		P.I.D. = 20(30.25')
34— —		P.I.D. = 242(31')
35— —		P.I.D. = 128(31.5')
36— —		
37— —		
38— —		
39— —		
40— —		
41— —		
42— —		

Time: 11:00 A.M. - 12:30 P.M.

SESI CONSULTING ENGINEERS, PC

Fig.

3 of 3

PROJECT NO. <u>7180</u>	PROJECT <u>Concord</u>	GEOPROBE NO. <u>OU-1B-15</u>
LOCATION <u>SEE FIGURE 1</u>	APPROX. ELEV. <u>1535.16 +</u>	INSPECTED BY <u>JZ/RF/CDM</u>
WATER OBSERVATION <u>Not Encountered</u>		DATE EXCAVATED <u>9/2/2008</u>

DEPTH FT.	DESCRIPTION / SOIL CLASSIFICATION	RECOVERY / P.I.D.(ppm)
0 —		R = 59"
—	0-.5': TOPSOIL	P.I.D. = 0.0(0-5')
1 —	.5'-4.9': Red-brown medium to fine SAND, little Silt with Cobbles	
—		
2 —		
—		
3 —	Sample: OU-1B-15A (.67'-1.83') 10:43	
—		
4 —		
—		
5 —		
—		
6 —	5'-10': Same	R = 58"
—		P.I.D. = 0.0(5'-10')
7 —		
—		
8 —		
—		
9 —		
—		
10 —		
—		
11 —	10'-14': Same	R = 54"
—		P.I.D. = 0.0(10'-14')
12 —		
—		
13 —		
—		
14 —		

Time:

7:45 - 11:25

SESI CONSULTING ENGINEERS, PC

Fig.

#

1 of 3

PROJECT NO. <u>7180</u>	PROJECT <u>Concord</u>	GEOPROBE NO. <u>OU-1B-15</u>
LOCATION <u>SEE FIGURE 1</u>	APPROX. ELEV. <u>1535.16 +</u>	INSPECTED BY <u>JZ/RF/CDM</u>
WATER OBSERVATION <u>Not Encountered</u>		DATE EXCAVATED <u>9/2/2008</u>

DEPTH FT.	DESCRIPTION / SOIL CLASSIFICATION	RECOVERY / P.I.D.(ppm)
14	14'-17': Same	R = 53"
15		P.I.D. = 0.0(14'-17')
16		
17		
18	17'-20': Red-brown medium to fine SAND, little Silt Sample: OU-1B-15B (18.2'-18.8') 11:15	R = 51"
19		P.I.D. = 0.0(17'-20')
20		
21		
22	20'-24': Red-brown medium to fine SAND, some Silt with Cobbles Sample: OU-1B-15C (20.0'-21.3') 11:20	R = 56"
23		P.I.D. = 0.0(20.2')
24		P.I.D. = 7.3(20.8)
25		P.I.D. = 37.7(21.2')
26		P.I.D. = 0.0(22.2'-24.0')
27	24'-27': Red-Brown medium to fine SAND, some Silt	R = 56"
28		P.I.D. = 16.3(24.8')
		P.I.D. = 0.0(26.2'-27.9')

Time: 7:45 - 11:25

SESI CONSULTING ENGINEERS, PC

Fig. #

2 of 3

PROJECT NO. <u>7180</u>	PROJECT <u>Concord</u>	GEOPROBE NO. <u>OU-1B-15</u>
LOCATION <u>SEE FIGURE 1</u>	APPROX. ELEV. <u>1535.16 ±</u>	INSPECTED BY <u>JZ/RF/CDM</u>
WATER OBSERVATION <u>Not Encountered</u>		DATE EXCAVATED <u>9/2/2008</u>

DEPTH FT.	DESCRIPTION / SOIL CLASSIFICATION	RECOVERY / P.I.D.(ppm)
28 —	27'-30': Same	R = 58"
—		P.I.D. = 0.0(27'-30')
29 —		
30 —	30'-34': Same with Cobbles	R = 58"
—		P.I.D. = 0.0(30'-34')
31 —		
32 —		
33 —	34'-37': Same moisture at 36.4'-37'	R = 58"
—		P.I.D. = 0.0(34'-37')
35 —		
36 —		
37 —	37'-40': Same with Cobbles very moist Sample: OU-1B-15E (38.3'-39.1')	R = 58"
38 —		P.I.D. = 0.0(37'-40')
39 —		
40 —		
41 —	End of Geo-Probe at 40 Feet	
42 —		

Time: 7:45 - 11:25

SESI CONSULTING ENGINEERS, PC

Fig. #

3 of 3

PROJECT NO. <u>7180</u>	PROJECT <u>Concord</u>	GEOPROBE NO. <u>OU-1B-16</u>
LOCATION <u>SEE FIGURE 1</u>	APPROX. ELEV. <u>1535.36 +</u>	INSPECTED BY <u>JZ/RF/CDM</u>
WATER OBSERVATION <u>Not Encountered</u>		DATE EXCAVATED <u>9/2/2008</u>

DEPTH FT.	DESCRIPTION / SOIL CLASSIFICATION	RECOVERY / P.I.D.(ppm)
0 —		R = 52"
—	0-.8': Light Brown medium to fine SAND, little Silt, little Gravel	P.I.D. = 0.0(0-5')
1 —	.8'-4.3': Red-brown medium to fine SAND, little Silt, little Gravel	
—		
2 —	Sample: OU-1B-16A (.8'-2') 14:20	
—		
3 —		
—		
4 —		
—		
5 —		
—		
6 —	5'-9.9': Red-brown medium to fine SAND, little Silt, little Gravel	R = 59"
—		P.I.D. = 0.0(5'-10')
7 —		
—		
8 —		
—		
9 —		
—		
10 —		
—		
11 —	10'-14': Same with weathered Sandstone at 12.6'-12.8'	R = 59"
—		P.I.D. = 0.0(10'-15')
12 —		
—		
13 —		
—		
14 —		

Time: 10:00 - 14:30

SESI CONSULTING ENGINEERS, PC

Fig. #

1 of 3

PROJECT NO. <u>7180</u>	PROJECT <u>Concord</u>	GEOPROBE NO. <u>OU-1B-16</u>
LOCATION <u>SEE FIGURE 1</u>	APPROX. ELEV. <u>1535.36 +</u>	INSPECTED BY <u>JZ/RF/CDM</u>
WATER OBSERVATION <u>Not Encountered</u>		DATE EXCAVATED <u>9/2/2008</u>

DEPTH FT.	DESCRIPTION / SOIL CLASSIFICATION	RECOVERY / P.I.D.(ppm)
14	14'-17': Same	R = 54"
15		P.I.D. = 0.0(14'-17')
16		
17	17'-20': Same	R = 51"
18		P.I.D. = 0.0(17'-20')
19		
20	20'-24': Same with weathered Sandstone at 23.3'-23.6'	R = 57"
21		P.I.D. = 0.0(20'-24')
22		
23		
24	24'-27': Same with weathered Sandstone at 26.75'-27'	R = 55"
25		P.I.D. = 0.0(24'-27')
26		
27		
28		

Time: 10:00 - 14:30

SESI CONSULTING ENGINEERS, PC

Fig. #

2 of 3

PROJECT NO.	7180	PROJECT	Concord	GEOPROBE NO.	OU-1B-16
LOCATION	SEE FIGURE 1	APPROX. ELEV.	1535.36 ±	INSPECTED BY	JZ/RF/CDM
WATER OBSERVATION	Not Encountered			DATE EXCAVATED	9/2/2008

DEPTH FT.	DESCRIPTION / SOIL CLASSIFICATION	RECOVERY / P.I.D.(ppm)
28 —	27'-30': Same with fractured Siltstone	R = 58"
—		P.I.D. = 0.0(27'-30')
29 —	Sample: OU-1B-16B (29.3'-30') 14:30	
—		
30 —		
—		
31 —	End of Geo-Probe at 30 Feet	
—		
32 —		
—		
33 —		
—		
34 —		
—		
35 —		
—		
36 —		
—		
37 —		
—		
38 —		
—		
39 —		
—		
40 —		
—		
41 —		
—		
42 —		

Time: 10:00 - 14:30

SESI CONSULTING ENGINEERS, PC

Fig. #

3 of 3

PROJECT NO. <u>7180</u>	PROJECT <u>Concord</u>	GEOPROBE NO. <u>OU-1B-17</u>
LOCATION <u>SEE FIGURE 1</u>	APPROX. ELEV. <u>1533.81 ±</u>	INSPECTED BY <u>JZ/RF/CDM</u>
WATER OBSERVATION <u>Not Encountered</u>		DATE EXCAVATED <u>9/2/2008</u>

DEPTH FT.	DESCRIPTION / SOIL CLASSIFICATION	RECOVERY / P.I.D.(ppm)
0 —		R = 56"
—	0-2.4': Fill: Orange-brown coarse to fine SAND, little Silt, little Gravel	P.I.D. = 0.0(0-5')
1 —	with traces of Asphalt and Concrete	
—	2.4'-4.67': Red-brown medium to fine SAND, little Silt, little Gravel	
2 —		
—	Sample: OU-1B-17A (2.67'-4.6') 14:45	
3 —		
—		
4 —		
—		
5 —		
—		
6 —	5'-10": Red-brown medium to fine SAND, little Silt, little Gravel	R = 58"
—		P.I.D. = 0.0(5'-10')
7 —		
—		
8 —		
—		
9 —		
—		
10 —		
—		
11 —	10'-14': Same	R = 58"
—		P.I.D. = 0.0(10'-15')
12 —		
—		
13 —		
—		
14 —		

Time: 11:30 - 14:55

SESI CONSULTING ENGINEERS, PC

Fig. #

1 of 3

PROJECT NO. <u>7180</u>	PROJECT <u>Concord</u>	GEOPROBE NO. <u>OU-1B-17</u>
LOCATION <u>SEE FIGURE 1</u>	APPROX. ELEV. <u>1533.81 +</u>	INSPECTED BY <u>JZ/RF/CDM</u>
WATER OBSERVATION <u>Not Encountered</u>		DATE EXCAVATED <u>9/2/2008</u>

DEPTH FT.	DESCRIPTION / SOIL CLASSIFICATION	RECOVERY / P.I.D.(ppm)
14	14'-17': Same with black coarse Sandstone at 16.6'-16.83'	R = 54"
15		P.I.D. = 0.0(14'-17')
16		
17		
18	17'-20': Same	R = 53"
19		P.I.D. = 0.0(17'-20')
20		
21		
22	20'-24': Same	R = 54"
23		P.I.D. = 0.0(20'-24')
24		
25		
26	24'-27': Same	R = 55"
27		P.I.D. = 0.0(24'-27')
28		

Time: 11:30 - 14:55

SESI CONSULTING ENGINEERS, PC

Fig. #

2 of 3

PROJECT NO. <u>7180</u>	PROJECT <u>Concord</u>	GEOPROBE NO. <u>OU-1B-17</u>
LOCATION <u>SEE FIGURE 1</u>	APPROX. ELEV. <u>1533.81 +</u>	INSPECTED BY <u>JZ/RF/CDM</u>
WATER OBSERVATION <u>Not Encountered</u>		DATE EXCAVATED <u>9/2/2008</u>

DEPTH FT.	DESCRIPTION / SOIL CLASSIFICATION	RECOVERY / P.I.D.(ppm)
28 —	27'-30': Same with weathered Sandstone at 29.3'-30'	R = 52"
29 —	Sample: OU-1B-17B (28.6'-29.3') 14:55	P.I.D. = 0.0(27'-30')
30 —		
31 —	End of Geo-Probe at 30 Feet	
32 —		
33 —		
34 —		
35 —		
36 —		
37 —		
38 —		
39 —		
40 —		
41 —		
42 —		

Time: 11:30 - 14:55

SESI CONSULTING ENGINEERS, PC

Fig. #

3 of 3

PROJECT NO. <u>7180</u>	PROJECT <u>Concord</u>	GEOPROBE NO. <u>OU-1B-18</u>
LOCATION <u>SEE FIGURE 1</u>	APPROX. ELEV. <u>1532.36 ±</u>	INSPECTED BY <u>JZ/RF/CDM</u>
WATER OBSERVATION <u>Not Encountered</u>		DATE EXCAVATED <u>9/2/2008</u>

DEPTH FT.	DESCRIPTION / SOIL CLASSIFICATION	RECOVERY / P.I.D.(ppm)
0 —		R = 49"
—	0-.5': TOPSOIL	P.I.D. = 0.0(0-5')
1 —	.5'-4.1': Red-brown medium to fine SAND, little Silt, little Gravel	
—	with little Cobbles	
2 —		
—	Sample: OU-1B-18A (.5'-1.5') 15:55	
3 —		
—		
4 —		
—		
5 —		
—		
6 —	5'-10": Same	R = 59"
—		P.I.D. = 0.0(5'-10')
7 —		
—		
8 —		
—		
9 —		
—		
10 —		
—		
11 —	10'-14': Same	R = 59"
—		P.I.D. = 0.0(10'-15')
12 —		
—		
13 —		
—		
14 —		

Time: 1:30 - 4:00

SESI CONSULTING ENGINEERS, PC

Fig. #

1 of 3

PROJECT NO. <u>7180</u>	PROJECT <u>Concord</u>	GEOPROBE NO. <u>OU-1B-18</u>
LOCATION <u>SEE FIGURE 1</u>	APPROX. ELEV. <u>1532.36 +</u>	INSPECTED BY <u>JZ/RF/CDM</u>
WATER OBSERVATION <u>Not Encountered</u>		DATE EXCAVATED <u>9/2/2008</u>

DEPTH FT.	DESCRIPTION / SOIL CLASSIFICATION	RECOVERY / P.I.D.(ppm)
14		
15	14'-17': Red-brown medium to fine SAND, little Silt, little Gravel	R = 59"
16		P.I.D. = 0.0(14'-17')
17		
18	17'-20': Same	R = 48"
19		P.I.D. = 0.0(17'-20')
20		
21	20'-24': Same	R = 54"
22		P.I.D. = 0.0(20'-24')
23		
24	24'-27': Same	R = 39"
25		P.I.D. = 0.0(24'-27')
26		
27		
28		

Time: 1:30 - 4:00

SESI CONSULTING ENGINEERS, PC

Fig. #

2 of 3

PROJECT NO. <u>7180</u>	PROJECT <u>Concord</u>	GEOPROBE NO. <u>OU-1B-18</u>
LOCATION <u>SEE FIGURE 1</u>	APPROX. ELEV. <u>1532.36 +</u>	INSPECTED BY <u>JZ/RF/CDM</u>
WATER OBSERVATION <u>Not Encountered</u>		DATE EXCAVATED <u>9/2/2008</u>

DEPTH FT.	DESCRIPTION / SOIL CLASSIFICATION	RECOVERY / P.I.D.(ppm)
28 —	27'-30': Same with weathered Sandstone at 29.3'-30'	R = 55"
—		P.I.D. = 0.0(27'-30')
29 —	OU-1B-18B (29.2'-30') 16:00	
—		
30 —		
—		
31 —	End of Geo-Probe at 30 Feet	
—		
32 —		
—		
33 —		
—		
34 —		
—		
35 —		
—		
36 —		
—		
37 —		
—		
38 —		
—		
39 —		
—		
40 —		
—		
41 —		
—		
42 —		

Time: 1:30 - 4:00

SESI CONSULTING ENGINEERS, PC

Fig. #

3 of 3

PROJECT NO. <u>7180</u>	PROJECT <u>Concord</u>	GEOPROBE NO. <u>OU-1B-19</u>
LOCATION <u>SEE FIGURE 1</u>	APPROX. ELEV. <u>1531.03</u>	INSPECTED BY <u>JZ/GP</u>
WATER OBSERVATION <u>Not Encountered</u>		DATE EXCAVATED <u>8/29/2008</u>

DEPTH FT.	DESCRIPTION / SOIL CLASSIFICATION	RECOVERY / P.I.D.(ppm)
0 —	0-4": Topsoil	R = 50"
—	4"-1.75': Orange-brown Silt, medium to fine Sand, little Gravel	P.I.D. = 0.0 (0-5')
1 —	1.75-4.8': Red-brown medium to fine SAND, little Gravel, little Silt, w/ cobbles	
—		
2 —	Sample: OU-1B-19A (6"-2') 11:45	
—		
3 —		
—		
4 —		
—		
5 —		
—	5-10': Same.	R = 57.5"
6 —		P.I.D. = 0.0(5-10')
—		
7 —		
—		
8 —		
—		
9 —		
—		
10 —		
—	10-14': Same	R = 58"
11 —		P.I.D. = 0.0(10-14')
—		
12 —		
—		
13 —		
—		
14 —		

Time: 10:30 A.M. - 12:00 A.M.

SESI CONSULTING ENGINEERS, PC

Fig. #

1 of 3

PROJECT NO. <u>7180</u>	PROJECT <u>Concord</u>	GEOPROBE NO. <u>OU-1B-20</u>
LOCATION <u>SEE FIGURE 1</u>	APPROX. ELEV. <u>15.9.80</u>	INSPECTED BY <u>JZ/GP</u>
WATER OBSERVATION <u>Not Encountered</u>		DATE EXCAVATED <u>8/29/2008</u>

DEPTH FT.	DESCRIPTION / SOIL CLASSIFICATION	RECOVERY / P.I.D.(ppm)
0 —	0-4": Topsoil	R = 58"
—	4"-4.8': Red-brown medium to fine SAND, little Silt, little Gravel	P.I.D. = 0.0 (0-5')
1 —		
—	Sample: OU-1B-20A (6"-2') 10:00	
2 —		
—		
3 —		
—		
4 —		
—		
5 —		
—	5-10': Same w/ more frequent cobbles from 8.25-9.8'	R = 58"
6 —		P.I.D. = 0.0(5-10')
—		
7 —		
—		
8 —		
—		
9 —		
—		
10 —		
—	10-15': Same w/ some Silt	R = 56"
11 —		P.I.D. = 0.0(10-15')
—	Sample: OU-1B-20B (12-13.5') 10:05	
12 —		
—		
13 —		
—		
14 —		

Time: 7:45 A.M. - 10:00 A.M.

SESI CONSULTING ENGINEERS, PC

Fig.

1 of 2

PROJECT NO. <u>7180</u>	PROJECT <u>Concord</u>	GEOPROBE NO. <u>OU-1B-20</u>
LOCATION <u>SEE FIGURE 1</u>	APPROX. ELEV. <u>1539.8</u>	INSPECTED BY <u>JZ/GP</u>
WATER OBSERVATION <u>Not Encountered</u>		DATE EXCAVATED <u>8/29/2008</u>

DEPTH FT.	DESCRIPTION / SOIL CLASSIFICATION	RECOVERY / P.I.D.(ppm)
14		
15		
16	15-19.5': Same	R = 55"
17	Sample: OU-1B-20C (18-19.5') 10:30	P.I.D. = 0.0(15-19.5')
18		
19		
20	19.5-20.3': Same	R = 10"
21	End of Geo-Probe at 20 Feet 3 Inches REFUSAL ON ROCK AT 20 FEET 3 INCHES	P.I.D. = 0.0(19.5-20.3')
22		
23		
24		
25		
26		
27		
28		

Time: 7:45 A.M. - 10:00 A.M.

SESI CONSULTING ENGINEERS, PC

Fig.

2 of 2

PROJECT NO. <u>7180</u>	PROJECT <u>Concord</u>	GEOPROBE NO. <u>OU-1B-21</u>
LOCATION <u>SEE FIGURE 1</u>	APPROX. ELEV. <u>1535.39</u>	INSPECTED BY <u>JZ/GP</u>
WATER OBSERVATION <u>Not Encountered</u>		DATE EXCAVATED <u>8/29/2008</u>

DEPTH FT.	DESCRIPTION / SOIL CLASSIFICATION	RECOVERY / P.I.D.(ppm)
0 —	0-3": Topsoil	R = 58"
—	3"-4.8': Red-brown medium to fine SAND, little Gravel, little Silt, w/ cobbles	P.I.D. = 0.0 (0-5')
1 —		
—	Sample: OU-1B-21A (6"-2') 10:45	
2 —		
—		
3 —		
—		
4 —		
—		
5 —		
—	5-10': Same w/ some Silt	R = 57"
6 —		P.I.D. = 0.0(5-10')
—	Sample: OU-1B-21B (8-9.5') 10:50	
7 —		
—		
8 —		
—		
9 —		
—		
10 —		
—	10-14': Same	R = 55"
11 —		P.I.D. = 0.0(10-14')
—		
12 —		
—		
13 —		
—		
14 —		

Time: 10:15 A.M. - 10:50 A.M.

SESI CONSULTING ENGINEERS, PC

Fig. #

1 of 3

PROJECT NO. <u>7180</u>	PROJECT <u>Concord</u>	GEOPROBE NO. <u>OU-1B-21</u>
LOCATION <u>SEE FIGURE 1</u>	APPROX. ELEV. <u>1535.39</u>	INSPECTED BY <u>JZ/GP</u>
WATER OBSERVATION <u>Not Encountered</u>		DATE EXCAVATED <u>8/29/2008</u>

DEPTH FT.	DESCRIPTION / SOIL CLASSIFICATION	RECOVERY / P.I.D.(ppm)
14	14-17': Same w/ more frequent Cobbles	R = 58" P.I.D. = 0.0(14-17')
15		
16		
17		
18	17-20': Same w/ less Cobbles	R = 51" P.I.D. = 0.0(17-20')
19		
20		
21		
22	20-24': Same / wet from 20.4-20.6' Sample: OU-1B-21C (20-21.3') 11:15	R = 55" P.I.D. = 0.0(20-24')
23		
24		
25		
26	24-27': Same	R = 57" P.I.D. = 0.0(24-27')
27		
28		
29		
30	27-30': Same w/ WEATHERED Rock at 29-30.4	R = 41" P.I.D. = 0.0(27-30')
31		

Time: 10:15 A.M. - 10:50 A.M.

SESI CONSULTING ENGINEERS, PC

Fig.

2 of 3

PROJECT NO. <u>7180</u>	PROJECT <u>Concord</u>	GEOPROBE NO. <u>OU-1B-21</u>
LOCATION <u>SEE FIGURE 1</u>	APPROX. ELEV. <u>1535.39</u>	INSPECTED BY <u>JZ/GP</u>
WATER OBSERVATION <u>Not Encountered</u>		DATE EXCAVATED <u>8/29/2008</u>

DEPTH FT.	DESCRIPTION / SOIL CLASSIFICATION	RECOVERY / P.I.D.(ppm)
28 —		
—		
29 —		
—		
30 —		
—	End of Geo-Probe at 30 Feet	
31 —	REFUSAL ON ROCK AT 30 FEET	
—		
32 —		
—		
33 —		
—		
34 —		
—		
35 —		
—		
36 —		
—		
37 —		
—		
38 —		
—		
39 —		
—		
40 —		
—		
41 —		
—		
42 —		

Time: 10:15 A.M. - 10:50 A.M.

SESI CONSULTING ENGINEERS, PC

Fig.

3 of 3

PROJECT NO. <u>7180</u>	PROJECT <u>Concord</u>	GEOPROBE NO. <u>OU-1B-22</u>
LOCATION <u>SEE FIGURE 1</u>	APPROX. ELEV. <u>1529.28</u>	INSPECTED BY <u>JZ/GP</u>
WATER OBSERVATION <u>Not Encountered</u>		DATE EXCAVATED <u>8/29/2008</u>

DEPTH FT.	DESCRIPTION / SOIL CLASSIFICATION	RECOVERY / P.I.D.(ppm)
0 —	0-6": Topsoil	R = 58"
—	6"-4.8': Red-brown medium to fine Sand, little Silt, little Gravel	P.I.D. = 0.0 (0-5')
1 —		
—	Sample: OU-1B-2A (6"-1.5') 9:05	
2 —		
—		
3 —		
—		
4 —		
—		
5 —		
—	5-10': Same w/Cobbles	R = 58"
6 —		P.I.D. = 0.0(5-10')
—	Sample: OU-1B-2B (6-7.5') 9:15	
7 —		
—		
8 —		
—		
9 —		
—		
10 —		
—	10-14':Same w/ more frequent Grave/Cobbles	R = 48"
11 —		P.I.D. = 0.0(10-14')
—		
12 —		
—		
13 —		
—		
14 —		

Time: 7:45 A.M. - 9:25 A.M.

SESI CONSULTING ENGINEERS, PC

Fig. #

1 of 3

PROJECT NO. <u>7180</u>	PROJECT <u>Concord</u>	GEOPROBE NO. <u>OU-1B-22</u>
LOCATION <u>SEE FIGURE 1</u>	APPROX. ELEV. <u>1529.28</u>	INSPECTED BY <u>JZ/GP</u>
WATER OBSERVATION <u>Not Encountered</u>		DATE EXCAVATED <u>8/29/2008</u>

DEPTH FT.	DESCRIPTION / SOIL CLASSIFICATION	RECOVERY / P.I.D.(ppm)
14	14-17': Same	R = 58" P.I.D. = 0.0(14-17')
15		
16		
17		
18	17-20': Same w/ less Cobbles	R = 48" P.I.D. = 0.0(17-20')
19		
20		
21		
22	20-24': Same w/some Silt	R = 59" P.I.D. = 0.0(20.24')
23		
24		
25		
26	24-27': Same	R = 57" P.I.D. = 0.0(24-27')
27		
28		
29		
30	27-30': Same	R = 54" P.I.D. = 0.0(27-30')
31		
32		
33		

Time: 7:45 A.M. - 9:25 A.M.

SESI CONSULTING ENGINEERS, PC

Fig.

2 of 3

PROJECT NO. <u>7180</u>	PROJECT <u>Concord</u>	GEOPROBE NO. <u>OU-1B-22</u>
LOCATION <u>SEE FIGURE 1</u>	APPROX. ELEV. <u>1529.28</u>	INSPECTED BY <u>JZ/GP</u>
WATER OBSERVATION <u>Not Encountered</u>		DATE EXCAVATED <u>8/29/2008</u>

DEPTH FT.	DESCRIPTION / SOIL CLASSIFICATION	RECOVERY / P.I.D.(ppm)
28—	Sample: OU-1B-2C (27.5-29') 9:25	
—		
29—		
—	End of Geo-Probe at 30 Feet	
30—		
—		
31—		
—		
32—		
—		
33—		
—		
34—		
—		
35—		
—		
36—		
—		
37—		
—		
38—		
—		
39—		
—		
40—		
—		
41—		
—		
42—		

Time: 7:45 A.M. - 9:25 A.M.

SESI CONSULTING ENGINEERS, PC

Fig.

3 of 3

PROJECT NO. <u>7180</u>	PROJECT <u>Concord</u>	GEOPROBE NO. <u>OU-1B-23</u>
LOCATION <u>SEE FIGURE 1</u>	APPROX. ELEV. <u>1530.16</u>	INSPECTED BY <u>JZ/RF/CDM</u>
WATER OBSERVATION <u>Not Encountered</u>		DATE EXCAVATED <u>8/29/2008</u>

DEPTH FT.	DESCRIPTION / SOIL CLASSIFICATION	RECOVERY / P.I.D.(ppm)
0 —	0-5': Light Brown medium to fine SAND, little Silt, little Gravel	R = 12"
—		
1 —		
—		
2 —		
—		
3 —		
—		
4 —		
—		
5 —	5-10': Gray medium to fine GRAVEL w/ red-brown medium to fine Sand	R = 43" P.I.D. = 24.7(6.3') P.I.D. = 3.7(7.1') P.I.D. = 144(8.0') P.I.D. = 70.4(8.4')
—		
6 —		
—		
7 —		
—		
8 —		
—		
9 —		
—		
10 —	10-15': Red-brown medium to fine SAND, some Silt	R = 49" P.I.D. = 302(.5') P.I.D. = 1006(11.3') P.I.D. = 364(12') P.I.D. = 263(13.1') P.I.D. = 194(13.75')
—		
11 —		
—		
12 —		
—	End of Geo-Probe at 14 Feet	
13 —		
14 —		

Time:

SESI CONSULTING ENGINEERS, PC

Fig.

1 of 1

PROJECT NO. <u>7180</u>	PROJECT <u>Concord</u>	GEOPROBE NO. <u>OU-1B-23A</u>
LOCATION <u>SEE FIGURE 1</u>	APPROX. ELEV. <u>1530.16</u>	INSPECTED BY <u>JZ/GP</u>
WATER OBSERVATION <u>Not Encountered</u>		DATE EXCAVATED <u>9/3/2008</u>

DEPTH FT.	DESCRIPTION / SOIL CLASSIFICATION	RECOVERY / P.I.D.(ppm)
0 —		R = 22"
—	0-10": Brown fine SAND, some Silt, little Gravel	P.I.D. = 0.0 (0-5')
1 —	10"-1.8': Brown coarse to fine Sand, little Gravel, little Silt	
—		
2 —	Sample: OU-1B-23A(10-1.8') 11:15	
—		
3 —		
—		
4 —		
—		
5 —		
—	5-5.4': Dark gray coarse to fine SAND, some Silt, little Gravel	R = 9"
6 —	5.4-6.1': Red-brown clayey SILT, trace Gravel, trace Sand	P.I.D. = 0.0(5-5.4')
—		P.I.D. = 2.5(5.4-6.1')
7 —		
—		
8 —		
—		
9 —		
—		
10 —		
—	10-10.8': Red-brown medium to fine Gravel, some coarse to fine Sand, little Silt	R = 14"
11 —	10.8-11.1'- Red-brown clayey SILT, trace Sand, trace Gravel	P.I.D. = 38.4(10.6')
—		P.I.D. = 23.5(11')
12 —		
—		
13 —		
—		
14 —		

Time: 11:15 A.M. - 12:15 P.M.

SESI CONSULTING ENGINEERS, PC

Fig.

1 of 3

PROJECT NO. <u>7180</u>	PROJECT <u>Concord</u>	GEOPROBE NO. <u>OU-1B-23A</u>
LOCATION <u>SEE FIGURE 1</u>	APPROX. ELEV. <u>1530.16</u>	INSPECTED BY <u>JZ/GP</u>
WATER OBSERVATION <u>Not Encountered</u>		DATE EXCAVATED <u>9/3/2008</u>

DEPTH FT.	DESCRIPTION / SOIL CLASSIFICATION	RECOVERY / P.I.D.(ppm)
14		
15		
16	15-15.75': Gray medium to fine Gravel, some coarse to fine Sand, little Silt	R = 60"
17	15.75-20': Red-brown clayey SILT, trace Sand, trace Gravel	P.I.D. = 8.9(15.4')
18		P.I.D. = 15.9(16')
19	Sample: OU-1B-23B (16.5-17.5') 12:05	P.I.D. = 19.1(16.5')
20		P.I.D. = 47(16.9')
21		P.I.D. = 20.3(17.5')
22		P.I.D. = 17.1 (18.1')
23		P.I.D. = 14.6(18.75')
24		P.I.D. = 11.5(19.25')
25	21.6-22.6': Same	P.I.D. = 10.0(20')
26		R = 32"
27		P.I.D. = 3.5(20.25')
28		P.I.D. = 5.0(20.8')
29		P.I.D. = 5.5(21.25')
30		P.I.D. = 4.8(21.6')
31		P.I.D. = 4.4(22.5')
32		
33		
34		
35	25-27.8': Same	R = 34"
36		P.I.D. = 7.1(25.25')
37	Sample: OU-1B-23C (27-27.8')	P.I.D. = 7.2(25.75')
38		P.I.D. = 6.0(26')
39		P.I.D. = 10.4(26.6')
40		P.I.D. = 5.4(27.1')

Time: 11:15 A.M. - 12:15 P.M.

SESI CONSULTING ENGINEERS, PC

Fig.

2 of 3

PROJECT NO. <u>7180</u>	PROJECT <u>Concord</u>	GEOPROBE NO. <u>OU-1B-23A</u>
LOCATION <u>SEE FIGURE 1</u>	APPROX. ELEV. <u>1530.16</u>	INSPECTED BY <u>JZ/GP</u>
WATER OBSERVATION <u>Not Encountered</u>		DATE EXCAVATED <u>9/3/2008</u>

DEPTH FT.	DESCRIPTION / SOIL CLASSIFICATION	RECOVERY / P.I.D.(ppm)
28 —		P.I.D. = 10.5(27.5')
—		P.I.D. = 6.0(27.6')
29 —		
—		
30 —		
—	End of Geo-Probe at 30 Feet	
31 —	REFUSAL ON ROCK AT 30 FEET	
—		
32 —		
—		
33 —		
—		
34 —		
—		
35 —		
—		
36 —		
—		
37 —		
—		
38 —		
—		
39 —		
—		
40 —		
—		
41 —		
—		
42 —		

Time: 11:15 A.M. - 12:15 P.M.

SESI CONSULTING ENGINEERS, PC

Fig.

3 of 3

PROJECT NO. <u>7180</u>	PROJECT <u>Concord</u>	GEOPROBE NO. <u>OU-1C-1</u>
LOCATION <u>SEE FIGURE 1</u>	APPROX. ELEV. <u>1484.37'±</u>	INSPECTED BY <u>RF/CDM</u>
WATER OBSERVATION <u>Not Encountered</u>		DATE EXCAVATED <u>9/4/2008</u>

DEPTH FT.	DESCRIPTION / SOIL CLASSIFICATION	RECOVERY / P.I.D.(ppm)
0 —	0-4": Asphalt	R = 48"
—	4"-4': Red-brown SILT, little Gravel, trace Sand	P.I.D = 0.0(0'-5')
1 —		
—		
2 —	Sample: OU-1C-1A (4"-2') 11:30	
—		
3 —		
—		
4 —		
—		
5 —		
—	5-8.9': Same	R = 47"
6 —		P.I.D = 0.0(5'-10')
—		
7 —		
—		
8 —		
—		
9 —		
—		
10 —		
—	10-11.2': Same	R = 45"
11 —	11.2-11.5': Red-brown fine SAND, little Gravel, trace Silt	P.I.D = 0.0(10'-15')
—		
12 —		
—		
13 —		
—		
14 —		

Time: 11:00 am - 3:48 pm

SESI CONSULTING ENGINEERS, PC

Fig.

1 of 3

PROJECT NO. <u>7180</u>	PROJECT <u>Concord</u>	GEOPROBE NO. <u>OU-1C-1</u>
LOCATION <u>SEE FIGURE 1</u>	APPROX. ELEV. <u>1484.37±</u>	INSPECTED BY <u>RF/CDM</u>
WATER OBSERVATION <u>Not Encountered</u>		DATE EXCAVATED <u>9/4/2008</u>

DEPTH FT.	DESCRIPTION / SOIL CLASSIFICATION	RECOVERY / P.I.D.(ppm)
14		
15		
16	15-20': Same	R = 60" P.I.D = 0.0(15'-20')
17		
18		
19		
20		
21	20'-25': Same	R = 56" P.I.D = 0.0(20'-25')
22		
23		
24		
25		
26	25.0-28.4': Red-brown fine SILT, trace Gravel, trace Sand	R= 53" P.I.D = 0.0(25'-30')
27		
28		

Time: 11:00 am - 3:48 pm

SESI CONSULTING ENGINEERS, PC

Fig.

2 of 3

PROJECT NO. 7180 PROJECT Concord GEOPROBE NO. OU-1C-1
 LOCATION SEE FIGURE 1 APPROX. ELEV. 1484.58'± INSPECTED BY RF/CDM
 WATER OBSERVATION Not Encountered DATE EXCAVATED 9/4/2008

DEPTH FT.	DESCRIPTION / SOIL CLASSIFICATION	RECOVERY / P.I.D.(ppm)
28	28.4-29.4': Weathered Shale Sample: OU-1C-1B(27.5-28.3')11:45	
29		
30		
31	End of Geo-Probe at 30 Feet	
32		
33		
34		
35		
36		
37		
38		
39		
40		
41		
42		

Time: 11:00 am - 3:48 pm

— SESI CONSULTING ENGINEERS, PC

Fig.

PROJECT NO. <u>7180</u>	PROJECT <u>Concord</u>	GEOPROBE NO. <u>OU-1C-5</u>
LOCATION <u>SEE FIGURE 1</u>	APPROX. ELEV. <u>1484.58'±</u>	INSPECTED BY <u>RF/CDM</u>
WATER OBSERVATION <u>Not Encountered</u>		DATE EXCAVATED <u>9/4/2008</u>

DEPTH FT.	DESCRIPTION / SOIL CLASSIFICATION	RECOVERY / P.I.D.(ppm)
0 —	0-2.75': Red-brown SILT, little fine Sand, little Gravel Sample: OU-1C-5A (1.6-2.75') 10:15	R = 33" P.I.D = 0.0(0'-5')
—		
1 —		
—		
2 —		
3 —	5-7.9': Same	R = 35" P.I.D = 0.0(5'-10')
—		
4 —		
—		
5 —		
6 —	10-12.5': Same	R = 60" P.I.D = 0.0(10'-12.5')
—		
7 —		
—		
8 —		
9 —	12.5-15': Red-brown fine SAND, little Silt, little Gravel	P.I.D. = 0.0 (12.5-15')
—		
10 —		
—		
11 —		
12 —		
13 —		
14 —		

Time: 11:00 am - 3:48 pm

SESI CONSULTING ENGINEERS, PC

Fig.

1 of 3

PROJECT NO. <u>7180</u>	PROJECT <u>Concord</u>	GEOPROBE NO.	OU-1C-5
LOCATION <u>SEE FIGURE 1</u>	APPROX. ELEV. <u>1484.58'±</u>	INSPECTED BY	RF/CDM
WATER OBSERVATION <u>Not Encountered</u>		DATE EXCAVATED	9/4/2008

DEPTH FT.	DESCRIPTION / SOIL CLASSIFICATION	RECOVERY / P.I.D.(ppm)
14 — —		
15 — —		
16 — —	15-16.25': Red-brown SILT, little fine Sand, little Gravel	R = 45" P.I.D. = 0.0 (15'-16.75')
17 — —	16.25-18.75': Red-brown fine SAND, little Silt, little Gravel with Boulder	P.I.D. = 0.0 (16.25-18.75')
18 — —		
19 — —		
20 — —		
21 — —	20'-21.2': Red-brown SILT, little fine Sand, little Gravel	R = 50" P.I.D. = 0.0 (20'-21.2')
22 — —	21.2-24.2': Red-brown fine SAND, some Silt, little Gravel	P.I.D. = 0.0 (21.2-24.2')
23 — —		
24 — —		
25 — —		
26 — —	25.0-26.7': Red-brown fine SAND, some Silt, little Gravel	R = 52" P.I.D. = 0.0 (25'-26.7')
27 — —	26.7-28.3': Red-brown fine SILT, little fine Sand, little Gravel	P.I.D. = 0.0 (26.7-28.3')
28 — —		

Time: 11:00 am - 3:48 pm

SESI CONSULTING ENGINEERS, PC

Fig.

PROJECT NO. <u>7180</u>	PROJECT <u>Concord</u>	GEOPROBE NO. <u>OU-1C-5</u>
LOCATION <u>SEE FIGURE 1</u>	APPROX. ELEV. <u>1484.58'±</u>	INSPECTED BY <u>RF/CDM</u>
WATER OBSERVATION <u>Not Encountered</u>		DATE EXCAVATED <u>9/4/2008</u>

DEPTH FT.	DESCRIPTION / SOIL CLASSIFICATION	RECOVERY / P.I.D.(ppm)
28	Sample: OU-1C-5B(27.7-28.5')10:30	
29		
30	End of Geo-Probe at 30 Feet	
31		
32		
33		
34		
35		
36		
37		
38		
39		
40		
41		
42		

Time: 11:00.am - 3:48.pm

SESI CONSULTING ENGINEERS, PC

Fig.

3 of 3

PROJECT NO. <u>7180</u>	PROJECT <u>Concord</u>	GEOPROBE NO. <u>OU-1C-9</u>
LOCATION <u>SEE FIGURE 1</u>	APPROX. ELEV. <u>1490.03'±</u>	INSPECTED BY <u>RF/CDM</u>
WATER OBSERVATION <u>Not Encountered</u>		DATE EXCAVATED <u>9/4/2008</u>

DEPTH FT.	DESCRIPTION / SOIL CLASSIFICATION	RECOVERY / P.I.D.(ppm)
0 —	0-6": Topsoil	R = 50"
—	4"-4.2': Red-brown m-f SAND, little Silt, little Gravel	P.I.D = 0.0(0'-5')
1 —		
—		
2 —	Sample: OU-1C-1A (6"-2.1') 17:00	
—		
3 —		
—		
4 —		
—		
5 —		
—	5-9.8': Red-brown c-f SAND, little Silt, little Gravel with small Cobbles	R = 58"
6 —		P.I.D = 0.0(5'-10')
—		
7 —		
—		
8 —		
—		
9 —		
—		
10 —		
—	10-14': Same	R = 59"
11 —		P.I.D = 0.0(10'-14')
—		
12 —		
—		
13 —		
—		
14 —		

Time:

SES CONSULTING ENGINEERS, PC

Fig.

1 of 3

PROJECT NO. <u>7180</u>	PROJECT <u>Concord</u>	GEOPROBE NO. <u>OU-1C-9</u>
LOCATION <u>SEE FIGURE 1</u>	APPROX. ELEV. <u>1490.03'±</u>	INSPECTED BY <u>RF/CDM</u>
WATER OBSERVATION <u>Not Encountered</u>		DATE EXCAVATED <u>9/4/2008</u>

DEPTH FT.	DESCRIPTION / SOIL CLASSIFICATION	RECOVERY / P.I.D.(ppm)
14	14-19': Same w/ Cobbles	R= 53" P.I.D = 0.0(14'-19')
15		
16		
17		
18		
19	19-23': Red-brown m-f SAND, little Silt, little Gravel with Cobbles	R = 59" P.I.D = 0.0(19-23')
20		
21		
22		
23		
24	23-24.1': Same	R= 44" P.I.D = 0.0(23-26')
25		
26	24.1-26': Red-brown m-f SAND, some Silt, little Gravel with Cobbles	
27		
28		
	26-30': Same w/ Cobbles	R=50" P.I.D = 0.0(26-30')

Time:

SESI CONSULTING ENGINEERS, PC

Fig.

PROJECT NO. <u>7180</u>	PROJECT <u>Concord</u>	GEOPROBE NO. <u>OU-1C-9</u>
LOCATION <u>SEE FIGURE 1</u>	APPROX. ELEV. <u>1490.03'±</u>	INSPECTED BY <u>RF/CDM</u>
WATER OBSERVATION <u>Not Encountered</u>		DATE EXCAVATED <u>9/4/2008</u>

DEPTH FT.	DESCRIPTION / SOIL CLASSIFICATION	RECOVERY / P.I.D.(ppm)
28 —	Sample: OU-1C-9B(28.9-30')17:05	
29 —		
30 —	End of Geo-Probe at 30 Feet	
31 —		
32 —		
33 —		
34 —		
35 —		
36 —		
37 —		
38 —		
39 —		
40 —		
41 —		
42 —		

Time.

SESI CONSULTING ENGINEERS, PC

Fig.

3 of 3

PROJECT NO. <u>7180</u>	PROJECT <u>Concord</u>	GEOPROBE NO. <u>OU-1C-10</u>
LOCATION <u>SEE FIGURE 1</u>	APPROX. ELEV. <u>1487.46</u>	INSPECTED BY <u>JZ/RF/CDM</u>
WATER OBSERVATION <u>Not Encountered</u>		DATE EXCAVATED <u>9/3/2008</u>

DEPTH FT.	DESCRIPTION / SOIL CLASSIFICATION	RECOVERY / P.I.D.(ppm)
0 — — 1 — — 2 — — 3 — — 4 — — 5 — —	0-.5': Topsoil .5'-4.8': Red-Brown medium to fine SAND, little Silt, little Gravel Sample: OU-1C-10A (.5'-2.5') 16:45	R = 58" P.I.D. = 0.0(0'-5')
6 — — 7 — — 8 — — 9 — — 10 — —	5'-10': Same w/ Cobbles at 8.4 - 9.1' Cobbles at 8.4 - 9.1'	R = 59" P.I.D. = 0.0(5'-10')
11 — — 12 — — 13 — — 14 — —	10'-15': Same	R = 56" P.I.D. = 0.0(10'-15')

Time:

SESI CONSULTING ENGINEERS, PC

Fig. #

1 of 3

PROJECT NO. <u>7180</u>	PROJECT <u>Concord</u>	GEOPROBE NO. <u>OU-1C-10</u>
LOCATION <u>SEE FIGURE 1</u>	APPROX. ELEV. <u>1487.46</u>	INSPECTED BY <u>JZ/RP/CDM</u>
WATER OBSERVATION <u>Not Encountered</u>		DATE EXCAVATED <u>9/3/2008</u>

DEPTH FT.	DESCRIPTION / SOIL CLASSIFICATION	RECOVERY / P.I.D.(ppm)
14		
15		
16	15'-20': Same w/ Cobbles	R = 53" P.I.D. = 0.0(15'-20')
17		
18		
19		
20		
21	20'-25': Same	R = 53"
22	22.5-23.25": Moisture Encountered	P.I.D. = 0.0(20'-25')
23		
24		
25		
26	25'-30': Same	R = 35"
27	25-28': Moisture Encountered	
28		

Time:

SESI CONSULTING ENGINEERS, PC

Fig. #

2 of 3

PROJECT NO. <u>7180</u>	PROJECT <u>Concord</u>	GEOPROBE NO. <u>OU-1C-10</u>
LOCATION <u>SEE FIGURE 1</u>	APPROX. ELEV. <u>1487.46</u>	INSPECTED BY <u>JZ/RF/CDM</u>
WATER OBSERVATION <u>Not Encountered</u>		DATE EXCAVATED <u>9/3/2008</u>

DEPTH FT.	DESCRIPTION / SOIL CLASSIFICATION	RECOVERY / P.I.D.(ppm)
28 —	Sample: OU-1C-10B (26.6'-28') 16:50	
29 —		
30 —	End of Geo-Probe at 30 Feet	
31 —		
32 —		
33 —		
34 —		
35 —		
36 —		
37 —		
38 —		
39 —		
40 —		
41 —		
42 —		

Time: _____

SESI CONSULTING ENGINEERS, PC

Fig. #

3 of 3

PROJECT NO. <u>7180</u>	PROJECT <u>Concord</u>	GEOPROBE NO. <u>OU-1C-11</u>
LOCATION <u>SEE FIGURE 1</u>	APPROX. ELEV. <u>1488.67</u>	INSPECTED BY <u>RF</u>
WATER OBSERVATION <u>Not Encountered</u>		DATE EXCAVATED <u>9/2/2008</u>

DEPTH FT.	DESCRIPTION / SOIL CLASSIFICATION	RECOVERY / P.I.D.(ppm)
0 —	0-7" Topsoil	R = 59"
—	7"-4.9': Red brown medium to fine SAND, little Gravel, little Silt	P.I.D = 0.0(0-5')
1 —		
—		
2 —	Sample: OU-1C-11A(7"-1.5') 9:00	
—		
3 —		
—		
4 —		
—		
5 —		
—	5-9.9': Same	R = 59"
6 —		P.I.D = 0.0(5-10')
—		
7 —		
—		
8 —		
—		
9 —		
—		
10 —		
—	10-14.9': Same	R = 58"
11 —		P.I.D = 0.0(10-15')
—		
12 —		
—		
13 —		
—		
14 —		

Time: 8:00 am - 9:15 am

SESI CONSULTING ENGINEERS, PC

Fig.

1 of 2

PROJECT NO. <u>7180</u>	PROJECT <u>Concord</u>	GEOPROBE NO. <u>OU-1C-11</u>
LOCATION <u>SEE FIGURE 1</u>	APPROX. ELEV. <u>1488.56</u>	INSPECTED BY <u>RF</u>
WATER OBSERVATION <u>Not Encountered</u>		DATE EXCAVATED <u>9/2/2008</u>

DEPTH FT.	DESCRIPTION / SOIL CLASSIFICATION	RECOVERY / P.I.D.(ppm)
14		
15		
16	15-18.6': Same	R= 43"
17	Sample: OU-1C-11B(15.3-18.3) 9:15	P.I.D = 0.0 (15-19')
18		
19	End of Geo-Probe at 19 Feet	
20	REFUASL AT 19 FEET	
21		
22		
23		
24		
25		
26		
27		
28		

Time: 8:00 am - 9:15 am

SES CONSULTING ENGINEERS, PC

Fig.

2 of 2

PROJECT NO. 7180

PROJECT Concord

GEOPROBE NO.

OU-1C-12

LOCATION SEE FIGURE 1

APPROX. ELEV. 1488.56

INSPECTED BY RF

WATER OBSERVATION Not Encountered

DATE EXCAVATED 9/2/2008

DEPTH FT.	DESCRIPTION / SOIL CLASSIFICATION	RECOVERY / P.I.D.(ppm)
0 — — 1 — — 2 — — 3 — — 4 — — 5 —	0-3": Topsoil 3"-2.75': Red-brown medium to fine SAND, little Silt, little Gravel Sample: OU-1C-12A(3"-2.25') 16:25	R = 33" P.I.D = 0.0(0-2.75')
— 6 — — 7 — — 8 — — 9 — — 10 —	5-10': Red-brown medium to fine SAND, little Silt, little Gravel w/ Cobbles	R = 60" P.I.D = 0.0(5-10')
— 11 — — 12 — — 13 — — 14 —	10-15': Red-brown medium to fine SAND, little Silt, little Gravel Sample: OU-1C-12B(13.75-15') 16:30	R = 60" P.I.D = 0.0(10-15')

Time: 13:30 pm - 16:30 pm

SESI CONSULTING ENGINEERS, PC

Fig.

1 of 2

PROJECT NO. <u>7180</u>	PROJECT <u>Concord</u>	GEOPROBE NO. <u>OU-1C-12</u>
LOCATION <u>SEE FIGURE 1</u>	APPROX. ELEV. <u>1488.56</u>	INSPECTED BY <u>RF</u>
WATER OBSERVATION <u>Not Encountered</u>		DATE EXCAVATED <u>9/2/2008</u>

DEPTH FT.	DESCRIPTION / SOIL CLASSIFICATION	RECOVERY / P.I.D.(ppm)
14 — —		
15 — —		
16 — —	Sample: 15-15.5': OU-1C-12B(45-60") 16:30	R= 0"
17 — —	End of Geo-Probe at 15 Feet 6 Inches	P.I.D = 0.0(15-15.5')
18 — —	REFUSAL AT 15 FEET 6 INCHES	
19 — —		
20 — —		
21 — —		
22 — —		
23 — —		
24 — —		
25 — —		
26 — —		
27 — —		
28 — —		

Time: 13:30 pm - 16:30 pm

SESI CONSULTING ENGINEERS, PC

Fig.

2 of 2

PROJECT NO. 7180 PROJECT Concord GEOPROBE NO. OU-1C-13
 LOCATION SEE FIGURE 1 APPROX. ELEV. 1484.85'± INSPECTED BY RF/CDM
 WATER OBSERVATION Not Encountered DATE EXCAVATED 9/4/2008

DEPTH FT.	DESCRIPTION / SOIL CLASSIFICATION	RECOVERY / P.I.D.(ppm)
0 — — 1 — — 2 — — 3 — — 4 — — 5 —	0-6": Topsoil 6"-2.6': Red-brown SILT, little fine Sand, trace Gravel Sample: OU-1C-13A (6"-1.5') 9:00	R = 32" P.I.D = 0.0(0'-5.0')
6 — — 7 — — 8 — — 9 — — 10 —	5-10': Red-brown SILT, little fine Sand, trace Gravel with Boulders	R = 60" P.I.D = 0.0(5'-10')
11 — — 12 — — 13 — — 14 —	10-14': Same	R = 44" P.I.D = 0.0(10'-14')

Time:

SESI CONSULTING ENGINEERS, PC

Fig.

1 of 3

PROJECT NO. <u>7180</u>	PROJECT <u>Concord</u>	GEOPROBE NO. <u>OU-1C-13</u>
LOCATION <u>SEE FIGURE 1</u>	APPROX. ELEV. <u>1484.85'±</u>	INSPECTED BY <u>RF/CDM</u>
WATER OBSERVATION <u>Not Encountered</u>		DATE EXCAVATED <u>9/4/2008</u>

DEPTH FT.	DESCRIPTION / SOIL CLASSIFICATION	RECOVERY / P.I.D.(ppm)
14	14-18': Same	R= 44" P.I.D = 0.0(14'-18')
15		
16		
17		
18	18-20.2': Same	R = 40" P.I.D = 0.0(18-22')
19		
20		
21		
22	20.2-23.3': Red-brown fine SAND, some Silt, little Gravel with Boulders	
23		
24		
25		
26	22-23.2': Red-brown SILT, some fine Sand, little Gravel	R= 36" P.I.D = 0.0(22-26')
27		
28		
29		
30	23.2-26': Red-brown fine SAND, some Silt, little Gravel with Boulders	
31		
32		
33		
34	26-27.5': Red-brown SILT, little fine Sand, little Gravel with Boulders	R=50" P.I.D = 0.0(26-30')
35		
36		
37		

Time:

SESI CONSULTING ENGINEERS, PC

Fig.

2 of 3

PROJECT NO. <u>7180</u>		PROJECT <u>Concord</u>		GEOPROBE NO. <u>OU-1C-13</u>	
LOCATION <u>SEE FIGURE 1</u>		APPROX. ELEV. <u>1484.85'±</u>		INSPECTED BY <u>RF/CDM</u>	
WATER OBSERVATION <u>Not Encountered</u>		DATE EXCAVATED <u>9/4/2008</u>			

DEPTH FT.	DESCRIPTION / SOIL CLASSIFICATION	RECOVERY / P.I.D.(ppm)
28 —	Sample: OU-1C-13B(27.7-28.5')9:15	
29 —		
30 —		
31 —	End of Geo-Probe at 30 Feet	
32 —		
33 —		
34 —		
35 —		
36 —		
37 —		
38 —		
39 —		
40 —		
41 —		
42 —		

Time:

SESI CONSULTING ENGINEERS, PC

Fig.

PROJECT NO. <u>7180</u>	PROJECT <u>Concord</u>	GEOPROBE NO. <u>OU-1C-14</u>
LOCATION <u>SEE FIGURE 1</u>	APPROX. ELEV. <u>1484.23</u>	INSPECTED BY <u>JZ/GP</u>
WATER OBSERVATION <u>Not Encountered</u>		DATE EXCAVATED <u>9/3/2008</u>

DEPTH FT.	DESCRIPTION / SOIL CLASSIFICATION	RECOVERY / P.I.D.(ppm)
0 —	0-3":	R = 36"
—	3"-3': Red brown Silt, some fine Sand, little Gravel	P.I.D = 0.0 (0-5')
1 —		
—	Sample OU-1C-14A 8"-1.6') 15:30	
2 —		
—		
3 —		
—		
4 —		
—		
5 —		
—	5-10': Same	R = 60"
6 —		P.I.D = 0.0 (5-10')
—		
7 —		
—		
8 —		
—		
9 —		
—		
10 —		
—	10-14': Same	R = 48"
11 —		P.I.D = 0.0 (10-14)
—		
12 —		
—		
13 —		
—		
14 —		

Time: 3:30 pm - 5:45 pm

SESI CONSULTING ENGINEERS, PC

Fig.

1 of 3

PROJECT NO. <u>7180</u>	PROJECT <u>Concord</u>	GEOPROBE NO. <u>OU-1C-14</u>
LOCATION <u>SEE FIGURE 1</u>	APPROX. ELEV. <u>1484.23</u>	INSPECTED BY <u>JZ/GP</u>
WATER OBSERVATION <u>Not Encountered</u>		DATE EXCAVATED <u>9/3/2008</u>

DEPTH FT.	DESCRIPTION / SOIL CLASSIFICATION	RECOVERY / P.I.D.(ppm)
14	14-18': Same	R = 42" P.I.D = 0.0 (14-18')
15		
16		
17		
18		
19	18-23': Same	R = 52" P.I.D = 0.0 (18-23')
20		
21		
22		
23		
24	23-28': Same	R = 58" P.I.D = 0.0 (23-28')
25		
26		
27		
28		

Time: 3:30 pm - 5:45 pm

SESI CONSULTING ENGINEERS, PC

Fig.

2 of 3

PROJECT NO. <u>7180</u>	PROJECT <u>Concord</u>	GEOPROBE NO. <u>OU-1C-14</u>
LOCATION <u>SEE FIGURE 1</u>	APPROX. ELEV. <u>1484.23</u>	INSPECTED BY <u>JZ/GP</u>
WATER OBSERVATION <u>Not Encountered</u>		DATE EXCAVATED <u>9/3/2008</u>

DEPTH FT.	DESCRIPTION / SOIL CLASSIFICATION	RECOVERY / P.I.D.(ppm)
28 — — 29 — — 30 — — 31 — — 32 — — 33 —	28-33': Same	R = 48" P.I.D = 0.0 (28-33')
— 34 — — 35 — — 36 — — 37 —	33-37': Same	R = 49" P.I.D = 0.0 (33-37')
— 38 — — 39 — — 40 —	37-40': Same Sample: OU-1C-14B(38-39.75') 15:45	R = 60"
— 41 — — 42 —	End of Geoprobe at 40 Feet REFUSAL AT 40 FEET	

Time: 3:30 pm - 5:45 pm

SESI CONSULTING ENGINEERS, PC

Fig.

3 of 3

PROJECT NO. <u>7180</u>	PROJECT <u>Concord</u>	GEOPROBE NO. <u>OU-1C-15</u>
LOCATION <u>SEE FIGURE 1</u>	APPROX. ELEV. <u>1483.08</u>	INSPECTED BY <u>JZ/GP</u>
WATER OBSERVATION <u>Not Encountered</u>		DATE EXCAVATED <u>9/3/2008</u>

DEPTH FT.	DESCRIPTION / SOIL CLASSIFICATION	RECOVERY / P.I.D.(ppm)
0 —	0-1.1': Gray -brown Silt, some fine Sand, little Gravel	R = 21"
—	1.1-1.75': Red brown Silt, some fine Sand, little gravel	P.I.D = 0.0 (0-5')
1 —		
—		
2 —	Sample: OU-1C-15A(0-1.75') 1400	
—		
3 —		
—		
4 —		
—		
5 —		
—	5-10': Red brown Sand and Silt, little Gravel	R = 60"
6 —		P.I.D = 0.0 (5-10')
—		
7 —		
—		
8 —		
—		
9 —		
—		
10 —		
—	10-15': Same	R = 60"
11 —		P.I.D = 0.0 (10-15')
—		
12 —		
—		
13 —		
—		
14 —		

Time: 2:00 pm - 3:00 pm

SESI CONSULTING ENGINEERS, PC

Fig.

1 of 3

PROJECT NO. <u>7180</u>	PROJECT <u>Concord</u>	GEOPROBE NO. <u>OU-1C-15</u>
LOCATION <u>SEE FIGURE 1</u>	APPROX. ELEV. <u>1483.08</u>	INSPECTED BY <u>JZ/GP</u>
WATER OBSERVATION <u>Not Encountered</u>		DATE EXCAVATED <u>9/3/2008</u>

DEPTH FT.	DESCRIPTION / SOIL CLASSIFICATION	RECOVERY / P.I.D.(ppm)
14 — —		
15 — —	15-19.25': Same	R = 51" P.I.D = 0.0 (15-20')
16 — —		
17 — —		
18 — —		
19 — —		
20 — —	20-23.5': Same	R = 43" P.I.D = 0.0 (20-25')
21 — —		
22 — —		
23 — —		
24 — —		
25 — —	15-28.1': Same	R = 38" P.I.D = 0.0 (25-30')
26 — —		
27 — —		
28 — —		

Time: 2:00 pm - 3:00 pm

SESI CONSULTING ENGINEERS, PC

Fig.

2 of 3

PROJECT NO. <u>7180</u>	PROJECT <u>Concord</u>	GEOPROBE NO. <u>OU-1C-15</u>
LOCATION <u>SEE FIGURE 1</u>	APPROX. ELEV. <u>1483.08</u>	INSPECTED BY <u>JZ/GP</u>
WATER OBSERVATION <u>Not Encountered</u>		DATE EXCAVATED <u>9/3/2008</u>

DEPTH FT.	DESCRIPTION / SOIL CLASSIFICATION	RECOVERY / P.I.D.(ppm)
28 —		
29 —		
30 —		
31 —	30-34.5': Same	R = 24"
32 —	Sample: OU-1C-15B(31-32') 15:00	P.I.D = 0.0 (30-34.5')
33 —		
34 —		
35 —	End of Geoprobe @ 34 Feet 6 Inches	
36 —	REFUSAL @ 34 FEET 6 INCHES	
37 —		
38 —		
39 —		
40 —		
41 —		
42 —		

Time: 2:00p.m. - 3:00 p.m.

SESI CONSULTING ENGINEERS, PC

Fig. #

3 of 3

PROJECT NO.	7180	PROJECT	Concord	GEOPROBE NO.	OU-1A-10
LOCATION	SEE FIGURE 1	APPROX. ELEV.	1498.62±	INSPECTED BY	RF/CDM
WATER OBSERVATION	Not Encountered			DATE EXCAVATED	8/19/2008

DEPTH FT.	DESCRIPTION / SOIL CLASSIFICATION	RECOVERY / P.I.D.(ppm)
0—	0-4": Fill: Concrete Debris	R=30"
—	4"-2.5': Red-brown medium to fine SAND, little Silt, little Gravel w/	P.I.D. = 0.0 (0-5')
1—	Weathered Sandstone from 1.7-2.5'	
—		
2—	Sample: OU-1A-10A (4"-1') 14:45	
—		
3—		
—		
4—		
—		
5—		
—	5-7.25': Red-brown SILT, little medium to fine Sand, little Gravel w/	R=43"
6—	Weathered Sandstone from 6.4-6.9', 7.2-7.25'	P.I.D. = 0.0 (5-7.25')
—	7.25-8.6': Brown coarse to fine SAND, little Silt, little Gravel w/ oil present	P.I.D. = 147.0 (7.7')
7—		P.I.D. = 103.0 (8.1')
—	Sample: OU-1A-10B (5.8-6.4') 15:10	P.I.D. = 43.1 (8.6')
8—	Sample: OU-1A-10C (7.5-8.2') 15:15	
—		
9—		
—		
10—		
—	10-10.3': Red-brown coarse to fine SAND, little Silt, little Gravel	R=52"
11—	10.3-14.1': Red-brown Silt, some mottled medium to fine Sand w/oil	P.I.D. = 108.0 (10.2')
—	present w/ Weathered Sandstone from 14.1-14.6'	P.I.D. = 58.0 (10.7')
12—		P.I.D. = 23.0 (11.2')
—	Sample: OU-1A-10D (12.8-13.8') 15:20	P.I.D. = 17.0 (11.7')
13—		P.I.D. = 2.3 (12.5')
—		P.I.D. = 0.2 (14.3')
14—		

Time: 2:30p.m.-3:25p.m.

SESI CONSULTING ENGINEERS, PC

Fig.

1 of 2

PROJECT NO. <u>7180</u>	PROJECT <u>Concord</u>	GEOPROBE NO. <u>OU-1A-10</u>
LOCATION <u>SEE FIGURE 1</u>	APPROX. ELEV. <u>1498.62±</u>	INSPECTED BY <u>RF/CDM</u>
WATER OBSERVATION <u>Not Encountered</u>		DATE EXCAVATED <u>8/19/2008</u>

DEPTH FT.	DESCRIPTION / SOIL CLASSIFICATION	RECOVERY / P.I.D.(ppm)
14		
15	End of Geo-Probe at 14 Feet 9 Inches REFUSAL ON ROCK AT 14 FEET 9 INCHES	
16		
17		
18		
19		
20		
21		
22		
23		
24		
25		
26		
27		
28		

Time: 2:30p.m.-3:25p.m.

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Fig.

2 of 2

PROJECT NO. <u>7180</u>	PROJECT <u>Concord</u>	GEOPROBE NO. <u>OU-1A-11</u>
LOCATION <u>SEE FIGURE 1</u>	APPROX. ELEV. <u>1497.51±</u>	INSPECTED BY <u>RF/CDM</u>
WATER OBSERVATION <u>Not Encountered</u>		DATE EXCAVATED <u>8/19/2008</u>

DEPTH FT.	DESCRIPTION / SOIL CLASSIFICATION	RECOVERY / P.I.D.(ppm)
0 —	0-1': Fill: Concrete and Asphalt Debris w/ Weathered Sandstone	R=25"
—	1-2.1': Black-brown coarse to fine SAND, little Silt, little Gravel, oil present	P.I.D. = 0.0 (0-5')
1 —		
—		
2 —	Sample: OU-1A-11A (1.3-2.1') 17:20	
—		
3 —		
—		
4 —		
—		
5 —		
—	5-5.6': Same	R=45"
6 —	5.6-7.5': Light-brown/Brown medium to fine SAND, little Silt, little Gravel	P.I.D. = 0.0 (5-10')
—	7.5-8.75': Red-brown coarse to fine SAND, little Silt, little Gravel w/	
7 —	Weathered Sandstone from 7.1-7.3' and Weathered Siltstone from 8.4-8.75'	
—		
8 —		
—		
9 —		
—		
10 —		
—	10-11.7': Same w/ Weathered Sandstone from 11-11.4'	R=24"
11 —	11.7-12': Red-brown SILT, little medium to fine Sand, trace Gravel	P.I.D. = 0.0 (10-12')
—		
12 —	Sample: OU-1A-11B (11.7-12') 17:25	
—		
13 —	End of Geo-Probe at 12 Feet 9 Inches	
—	REFUSAL ON ROCK AT 12 FEET 9 INCHES	
14 —		

Time:

4:45p.m.-5:25p.m.

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Fig.

1 of 1

PROJECT NO. <u>7180</u>	PROJECT <u>Concord</u>	GEOPROBE NO. <u>I17-1</u>
LOCATION <u>SEE FIGURE 1</u>	APPROX. ELEV. <u>1496.46'+</u>	INSPECTED BY <u>JZ/RF/CDM</u>
WATER OBSERVATION <u>Not Encountered</u>		DATE EXCAVATED <u>8/11/2008</u>

DEPTH FT.	DESCRIPTION / SOIL CLASSIFICATION	RECOVERY / P.I.D.(ppm)
0 — —	0-.3': Concrete and Misc. Debris	R = 52" P.I.D. = 0.0(0.0'-5.0')
1 — —	.3'-2.9': Red-Brown coarse to fine SAND, little Silt, little Gravel Sandstone at (1.8'-2.0') and (2.3'-2.4')	
2 — —		
3 — —	2.9'-4.3': Brown SILT, little medium to fine SAND, trace Gravel Sandstone at (4.2'-4.3')	
4 — —	I17-1A (.3'-1.2') 11:37 I17-1B (3.8'-4.3') 11:50	
5 — —		
6 — —	5-5.3': Concrete and Misc. Debris 5.3'-5.5': Black Weathered Sandstone 5.5'-5.7': Gray Weathered Sandstone	R = 60" P.I.D. = 0.0(5'-10')
7 — —	5.7'-6.0': Light Brown coarse to fine SAND, little Gravel, trace Silt 6.0'-8.0': Red-Brown medium to fine SAND, little Silt, little Gravel	
8 — —		
9 — —	8.0'-8.5': Brown coarse to fine SAND, some Gravel, little Silt 8.5'-10.0': Red-Brown medium to fine SAND, little Silt, little Gravel	
10 — —		
11 — —	10.0'-12.9': Brown medium to fine SAND, little Silt, little Gravel 12.9'-14.5': Gray.Red-Brown mottled coarse to fine SAND, little Silt, little Gravel with weathered Sandstone and petroleum odor I1701C (13.4'-14.4') 12:07	R = 58" P.I.D. = 0.0(10'-14')
12 — —		
13 — —	GEOPROBE REFUSAL AT 14.5 FEET	
14 —	GEOPROBE COMPLETE AT 14.5 FEET	

Time: 11:00a.m.-12:10p.m.

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Fig. #

1 of 1

PROJECT NO. <u>7180</u>	PROJECT <u>Concord</u>	GEOPROBE NO. <u>K-18-1</u>
LOCATION <u>SEE FIGURE 1</u>	APPROX. ELEV. <u>1487.94 ±</u>	INSPECTED BY _____
WATER OBSERVATION <u>Not Encountered</u>		DATE EXCAVATED <u>9/5/2008</u>

DEPTH FT.	DESCRIPTION / SOIL CLASSIFICATION	RECOVERY / P.I.D.(ppm)
0 —	0-.9': CONCRETE .9'-3.25': Brown medium to fine SAND, little Gravel, little Silt Sample: K-18-1A (.9'-1.83') 9:15 Sample: K-18-1B (1.83'-2.9') 9:30	R = 39" P.I.D = 0.0 (0-5')
—		
1 —		
—		
2 —		
—	5'-5.6': Same 5.6'-8': Tan weathered SANDSTONE (coarse grained) Sample: K-18-1C (5.6'-6.2') 9:45	R = 50" P.I.D = 0.0 (5'-10')
3 —		
—		
4 —		
5 —		
—	End of Geo-Probe at 8 Feet REFUSAL AT 8 FEET	
6 —		
—		
7 —		
—		
8 —		
—		
9 —		
—		
10 —		
—		
11 —		
—		
12 —		
—		
13 —		
—		
14 —		

Time: 9:10 - 9:45

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Fig.

1 of 1

PROJECT NO. <u>7180</u>	PROJECT <u>Concord</u>	TEST PIT NO. <u>L-18-1</u>
LOCATION <u>SEE FIGURE 1</u>	APPROX. ELEV. <u>1491.89</u>	INSPECTED BY <u>JZ/MZ</u>
WATER OBSERVATION <u>Not Encountered</u>		DATE EXCAVATED <u>9/4/2008</u>

DEPTH FT.	DESCRIPTION / SOIL CLASSIFICATION	RECOVERY / P.I.D.
0 —		R = 37"
—	0-.3': Fill: CONCRETE and Misc. Debris	P.I.D. = 0.0(0-5')
1 —	.3'-2.2': Brown coarse to fine SAND, little Gravel, little Silt	
—		
2 —		
—	2.2'-3.1': Light Brown medium to fine SAND, trace Gravel, trace Silt	
3 —		
—	Sample: L-18-1A (.5'-1.25') 15:30	
4 —	Sample: L-18-1B (2.1'-2.9') 15:45	
—		
5 —		
—		R = 33"
6 —	5'-5.67': Same	P.I.D. = 0.0(5'-10')
—	5.67'-6.67': Brown SILT, trace Gravel, trace Silt	
7 —	6.67'-7.75': Brown medium to fine SAND, little Gravel, trace Silt	
—		
8 —		
—		
9 —		
—		
10 —		
—	0-11.9': Brown SILT, little Gravel, trace Silt with Boulders	R = 23"
11 —		P.I.D. = 0.0(10'-12')
—	Sample: L-18-1C (11.25'-11.9') 16:00	
12 —		
—	End of Geo-Probe at 12 Feet	
13 —	REFUSAL AT 12 FEET	
—		
14 —		

Time: 15:30 - 16:00

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Fig.

PROJECT NO. <u>7180</u>	PROJECT <u>Concord</u>	GEOPROBE NO. <u>L19</u>
LOCATION <u>SEE FIGURE 1</u>	APPROX. ELEV. _____	INSPECTED BY <u>JZ</u>
WATER OBSERVATION <u>Not Encountered</u>		DATE EXCAVATED <u>8/21/2008</u>

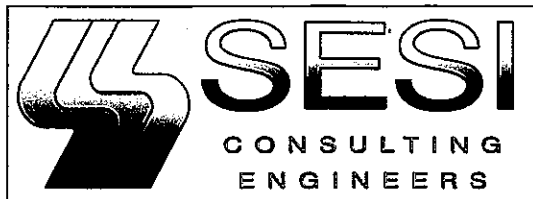
DEPTH FT.	DESCRIPTION / SOIL CLASSIFICATION	RECOVERY / P.I.D.(ppm)
0 —	0-3.6': Fill: Brown coarse to fine SAND, some medium to fine Gravel, little Silt,	R = 44"
—	w/ weathered Sandstone, Concrete, Brick	P.I.D = 0.0
1 —		
—		
2 —	L-19-1A (6"-1.5') 9:30 am	
—	L-19-1B (3-3.6') 9:45 am	
3 —		
—		
4 —		
—		
5 —		
—	5-6': Same as above	R = 30"
6 —	6-6.6': Gray coarse to fine Sand, some coarse to fine weathered Sandstone, trace Silt	P.I.D = 0.0
—	6.6-7.5': Fractured/ weathered Sandstone	
7 —	L-19-1C(5-6') 10:00 am	
—		
8 —		
—		
9 —		
—		
10 —		
—		
11 —		
—		
12 —		
—		
13 —		
—		
14 —		

Time: 8:30 am - 10:00 am

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Fig.

1 of 1



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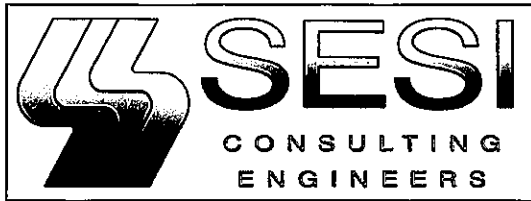
DATE: 9/12/2008	JOB NO. 5485
PROJECT: Concord	
LOCATION: Lake Kiamesha, NY	
CONTRACTOR: Fuller	
OWNER: Capelli	
WEATHER: Sunny	TEMP: Low 60's
PRESENT AT SITE	
Oscar Nordstrom/Brian Tetor-Fuller	
George Rivera-Skyline Concrete	
Eric Beck-Tano	

CONTRACTOR, LABOR & EQUIPMENT	
QUANTITY	DESCRIPTION
1	Crew for Excavation-Tano
1	Crew for Footings-Skyline Concrete

CHARGABLE EQUIPMENT USED
Nuclear Density Gauge

GEOTECH INSPECTION REPORT

- Excavated 2 to 3 feet below existing finished floor subgrade to a firm natural footing subgrade or solid bedrock for Casino Footings C724, C339, C717, C350 to C356 and compacted with double-drum sheepsfoot roller. Footings C717, C350, C353 to C355 needed rock removal to achieve subgrade. Visual observation and probing of the footing subgrades verified a footing subgrade of 4 TSF bearing capacity.
- Excavated material from footings was taken to the roadway north of Sediment Basin #2 and one 12-inch thick loose lift were placed from elevation $\pm 13'$ above former tennis court elevation in the southern part of the north of Sediment Basin #2 roadway, and compacted with single smooth drum 10-ton roller and single drum sheepsfoot 10-ton roller. Nuclear density testing, visual observation and probing of the subgrade verified the lift was placed in accordance with design specifications.
- Casino Footings C210 to C212, C221 to C223, C232 to C234, C245 to C246, C255 to C258, C264 to C270, C277 to C283, C900, C288 to C294, C891, C304 to C305, C880 were backfilled with two 12-inch thick lifts of 1-½-inch crushed bedrock and recycled concrete aggregate. Casino Footings C231, C241 to C244, C253 to C254, C262, to C263, C273 to C276, C284 to C287 were backfilled with one 12-inch thick lift of 1-½-inch crushed bedrock and recycled concrete aggregate and will be completed on 9/15/08. Footings C210 to C212, C221 to C223, C231 to C232, C241 to C244 had soft material at bottom of excavation removed before placement of fill materials. Nuclear density testing, visual observation and probing of the subgrade verified the lifts were placed in accordance with design specifications. Pending a proof-roll of 2nd lift footings backfill is approved.



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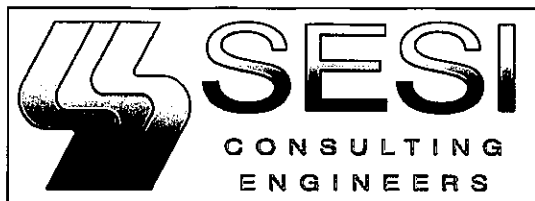
CONTRACTOR, LABOR & EQUIPMENT	
QUANTITY	DESCRIPTION
1	Crew for Excavation-Tano
1	Crew for Footings-Skyline Concrete

DATE: 9/3/2008	JOB NO. 5485
PROJECT: Concord	
LOCATION: Lake Kiamesha, NY	
CONTRACTOR: Fuller	
OWNER: Capelli	
WEATHER: Sunny	TEMP: Mid 80's
PRESENT AT SITE	
Oscar Nordstrom/Al Destefano-Fuller	
George Rivera-Skyline Concrete	
Eric Beck-Tano	
Mark Skellet-MHE (Township Representative)	

CHARGABLE EQUIPMENT USED
Nuclear Density Gauge

GEOTECH INSPECTION REPORT

- Excavated 2 to 3 feet below existing finished floor subgrade to a firm natural footing subgrade or previously approved compacted fill footing subgrade for Casino Footings C246 to C250, C927, C235 to C239, C933, C224 to C228, C938, C213 to C217, C941 and compacted with doubledrum sheepsfoot roller. Visual observation and probing of the footing subgrades verified a footing subgrade of 4 TSF bearing capacity.
- The excavated material from the footings was taken to the node line 13.5 to 14.5 from J to H and two 12-inch thick loose lifts of natural on-site soil was placed and compacted with single drum sheepsfoot 10-ton roller and a single smooth drum 10-ton roller. Nuclear density testing, visual observation and probing of the subgrade verified the lift was placed in accordance with design specifications. Some excavated material was placed in a stockpile nearby the footings approved and poured 9/2/08 for backfill tomorrow.
- Soil excavated during the construction of Sediment Basin #1 was placed in a 12-inch loose lift of on-site fill material in roadway to the north of Sediment Basin #2, but when placed noticeable amounts of large debris was visible. Lift was not approved. Al Beck informed us the soil would be removed and sorted before use.



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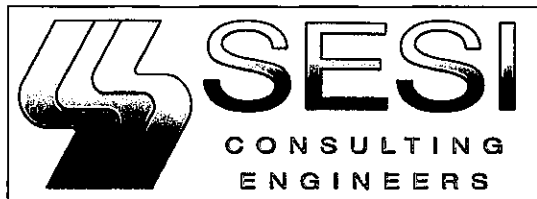
DATE: 9/15/2008	JOB NO. 5485
PROJECT: Concord	
LOCATION: Lake Kiamesha, NY	
CONTRACTOR: Fuller	
OWNER: Capelli	
WEATHER: Sunny	TEMP: Mid 70's
PRESENT AT SITE	
Oscar Nordstrom/Brian Tetor-Fuller	
George Rivera-Skyline Concrete	
Eric Beck-Tano	

CONTRACTOR, LABOR & EQUIPMENT	
QUANTITY	DESCRIPTION
1	Crew for Excavation-Tano
1	Crew for Footings-Skyline Concrete

CHARGABLE EQUIPMENT USED
Nuclear Density Gauge

GEOTECH INSPECTION REPORT

- Excavated 2 to 3 feet below existing finished floor subgrade to a firm natural footing subgrade for Casino Footings C711, C361, C362, C370, C371, C817 and compacted with double-drum sheepsfoot roller. Footing C371, C817 was over-excavated 6 to 12-inches due to slight pumping and backfilled with recycled concrete aggregate. Visual observation and probing of the footing subgrades verified a footing subgrade of 4 TSF bearing capacity.
- Excavated material from footings, from node line 18 from approximate node line K to L and from node line 17 from approximate node line L to N was taken to the roadway north of Sediment Basin #2 and two 12-inch thick loose lift were placed from elevation $\pm 14'$ to $\pm 15'$ above the former tennis court elevation, on southern end of roadway section and compacted with single smooth drum 10-ton roller. Nuclear density testing, visual observation and probing of the subgrade verified the lift was placed in accordance with design specifications.
- Casino Footings C774, C251 to C252, C756, C295 to C303, C749, C306 to C312, C741, C317 to C323, C732, C328 to C333, C724, C339 to C341 were backfilled with two 12-inch thick lifts of recycled concrete aggregate. Casino Footings C779, C240 to C244, C253 to C254, C769, C262 to C263, C765, C273 to C276, C761, C284 to C287, C313, C324, C334 to C335 were backfilled with one 12-inch thick lift of recycled concrete aggregate. Nuclear density testing, visual observation and probing of the subgrade verified the lifts were placed in accordance with design specifications. Pending a proof-roll of 2nd lift the footing backfill in this area is approved.



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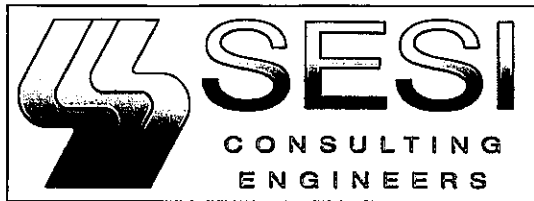
DATE: 9/9/2008	JOB NO. 5485
PROJECT: Concord	
LOCATION: Lake Kiamesha, NY	
CONTRACTOR: Fuller	
OWNER: Capelli	
WEATHER: Rain	TEMP: High 60's
PRESENT AT SITE	
Oscar Nordstrom/Brian Tetor-Fuller	
George Rivera-Skyline Concrete	
Eric Beck-Tano	

CONTRACTOR, LABOR & EQUIPMENT	
QUANTITY	DESCRIPTION
1	Crew for Excavation-Tano
1	Crew for Footings-Skyline Concrete

CHARGABLE EQUIPMENT USED

GEOTECH INSPECTION REPORT

- Excavated 2 to 3 feet below existing finished floor subgrade to a firm natural footing subgrade for Casino Footings C315 and compacted with double-drum sheepsfoot roller. Three Casino Footings C314, C302, and C303 were re-inspected from yesterday because they were not poured and it was raining at the time of pour. Footings remained approved. Visual observation and probing of the footing subgrades verified a footing subgrade of 4 TSF bearing capacity.
- A rock outcrop located at approximate node H-16 was jack hammered. All rock was brought to the crusher and crushed into 1-½-inch stone. The stone will be reused tomorrow during backfill of footings if the bottoms are moist.



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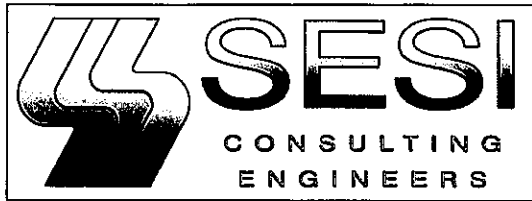
CONTRACTOR, LABOR & EQUIPMENT	
QUANTITY	DESCRIPTION
1	Crew for Excavation-Tano
1	Crew for Footings-Skyline Concrete

DATE: 9/8/2008	JOB NO. 5485
PROJECT: Concord	
LOCATION: Lake Kiamesha, NY	
CONTRACTOR: Fuller	
OWNER: Capelli	
WEATHER: Sunny	TEMP: Mid 70's
PRESENT AT SITE	
Oscar Nordstrom/Brian Tetor-Fuller	
George Rivera-Skyline Concrete	
Eric Beck-Tano	

CHARGABLE EQUIPMENT USED
Nuclear Density Gauge

GEOTECH INSPECTION REPORT

- Excavated 2 to 3 feet below existing finished floor subgrade to a firm natural footing subgrade for Casino Footings C749, C306 to C314, C756, C295 to C304 and compacted with double-drum sheepsfoot roller. Three footings were over-excavated 6 to 12 inches, C296, C306, and C303 and filled with natural on-site material due to pumping during compaction. Visual observation and probing of the footing subgrades verified a footing subgrade of 4 TSF bearing capacity.
- Excavated material from footings and from node line 16 and 17 from approximate node line J to M was taken to the roadway north of Sediment Basin #2 and three 12-inch thick loose lifts were placed from elevation $\pm 13'$ to $\pm 16'$ above former tennis court elevation, and compacted with single smooth drum 10-ton roller and single drum sheepsfoot 10-ton roller. Nuclear density testing, visual observation and probing of the subgrade verified the lift was placed in accordance with design specifications.



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DATE: 9/11/2008	JOB NO. 5485
PROJECT: Concord	
LOCATION: Lake Kiamesha, NY	
CONTRACTOR: Fuller	
OWNER: Capelli	
WEATHER: Sunny	TEMP: Low 70's
PRESENT AT SITE	
Oscar Nordstrom/Brian Tetor-Fuller	
George Rivera-Skyline Concrete	
Eric Beck-Tano	

CONTRACTOR, LABOR & EQUIPMENT	
QUANTITY	DESCRIPTION
1	Crew for Excavation-Tano
1	Crew for Footings-Skyline Concrete

CHARGABLE EQUIPMENT USED
Nuclear Density Gauge

GEOTECH INSPECTION REPORT

- Excavated 2 to 3 feet below existing finished floor subgrade to a firm natural footing subgrade or solid bedrock for Casino Footings C340 to C349, C837, C357 to C360, C826 and compacted with double-drum sheepsfoot roller. Footings C343 to C345 needed rock removal to achieve subgrade. Visual observation and probing of the footing subgrades verified a footing subgrade of 4 TSF bearing capacity.
- Excavated material from footings was taken to the roadway north of Sediment Basin #2 and two 12-inch thick loose lifts were placed from elevation $\pm 11'$ to $\pm 12'$ above former tennis court elevation in the southern part of the north of Sediment Basin #2 roadway, and compacted with single smooth drum 10-ton roller and single drum sheepsfoot 10-ton roller. Nuclear density testing, visual observation and probing of the subgrade verified the lifts were placed in accordance with design specifications.
- Casino Footings C213 to C217, C941, C224 to C228, C938, C235 to C239, C933, C247 to C250, C927, C259 to C261, C919, C271 to C272, C909 were backfilled with two 12-inch thick lifts of 1-½-inch crushed bedrock and recycled concrete aggregate. Footings C249 to C250, C927, C238 to C239, C933, C224 to C228, C938, C213 to C217, C941 had soft material at bottom of excavation removed before placement of fill materials. Nuclear density testing, visual observation and probing of the subgrade verified the lifts were placed in accordance with design specifications. Pending a proof-roll of 2nd lift footings backfill is approved.



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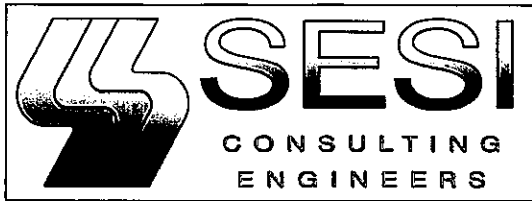
CONTRACTOR, LABOR & EQUIPMENT	
QUANTITY	DESCRIPTION
1	Crew for Excavation-Tano
1	Crew for Footings-Skyline Concrete

DATE: 9/4/2008	JOB NO. 5485
PROJECT: Concord	
LOCATION: Lake Kiamesha, NY	
CONTRACTOR: Fuller	
OWNER: Capelli	
WEATHER: Sunny	TEMP: High 80's
PRESENT AT SITE	
Oscar Nordstrom/Al Destefano-Fuller	
George Rivera-Skyline Concrete	
Eric Beck-Tano	
Mark Skellet-MHE (Township Representative)	

CHARGABLE EQUIPMENT USED
Nuclear Density Gauge

GEOTECH INSPECTION REPORT

- Excavated 2 to 3 feet below existing finished floor subgrade to a firm natural footing subgrade for Casino Footings C761, C284 to C289, C765, C273 to C278, C769, C262 to C267, C774, C251 to C256 and compacted with doubledrum sheepsfoot roller. Visual observation and probing of the footing subgrades verified a footing subgrade of 4 TSF bearing capacity. Other footings were excavated and will be expected tomorrow when forming begins.
- Excavated material from footings was placed in a 12-inch thick loose lift of natural on-site soil along the future location of footings C781, C785, C787, C203, C202, C381, C384, and C385, to achieve finished floor subgrade, and compacted with single drum sheepsfoot 10-ton roller. Nuclear density testing, visual observation and probing of the subgrade verified the lift was placed in accordance with design specifications.
- Remaining excavated material from the footings and from the recently cleared 16 node line from approximate node line J to K was taken to the south of Sediment Basin #1 and stockpiled for construction of the Sediment Basin #1 berms.
- Unapproved 12-inch thick loose lift placed yesterday at elevation $\pm 9'$ above former tennis court in Sediment Basin #2 was sorted removing all debris and compacted with a single smooth drum 10-ton roller. Nuclear density testing, visual observation and probing of the subgrade verified that lift was placed in accordance with design specifications.



12A MAPLE AVENUE
PINE BROOK, NJ 07058
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CONTRACTOR, LABOR & EQUIPMENT	
QUANTITY	DESCRIPTION
1	Crew for Excavation-Tano
1	Crew for Footings-Skyline Concrete

DATE: 9/5/2008	JOB NO. 5485
PROJECT: Concord	
LOCATION: Lake Kiamesha, NY	
CONTRACTOR: Fuller	
OWNER: Capelli	
WEATHER: Sunny	TEMP: Mid 80's
PRESENT AT SITE	
Oscar Nordstrom/Al Destefano-Fuller	
George Rivera-Skyline Concrete	
Eric Beck-Tano	
Mark Skellet-MHE (Township Representative)	

CHARGABLE EQUIPMENT USED
Nuclear Density Gauge

GEOTECH INSPECTION REPORT

- Excavated 2 to 3 feet below existing finished floor subgrade to a firm natural footing subgrade for Casino Footings C290 to C294, C891, C279 to C283, C900, C268 to C272, C909, C257 to C261, C919 and compacted with double-drum sheepsfoot roller. Visual observation and probing of the footing subgrades verified a footing subgrade of 4 TSF bearing capacity. Some footings above were previously excavated yesterday 9/4/08.
- Excavated material from cleared node lines 16 and 17 from approximate node line J to M was placed in 12-inch thick loose lifts of natural on-site soil around Casino Footings. Two lifts were placed and compacted with a double-drum sheepsfoot roller around Casino Footings C788, C207 to C210, C786, C218 to C220, C782, C229 to C231. One lift was placed and compacted with a double-drum sheepsfoot roller around Casino Footings C779, C240 to C244, C232 to C233, C221 to C222, C211. Nuclear density testing, visual observation and probing of the subgrade verified the lift was placed in accordance with design specifications.
- Remaining excavated material from node line 16 and 17 from approximate node line J to M, and the excavated footing material was taken to the roadway north of Sediment Basin #2 and three 12-inch thick loose lifts were placed from elevation $\pm 10'$ to $\pm 13'$ above former tennis court elevation, and compacted with single smooth drum 10-ton roller. Nuclear density testing, visual observation and probing of the subgrade verified the lift was placed in accordance with design specifications.



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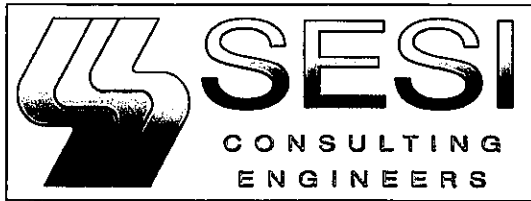
DATE: 9/10/2008	JOB NO. 5485
PROJECT: Concord	
LOCATION: Lake Kiamesha, NY	
CONTRACTOR: Fuller	
OWNER: Capelli	
WEATHER: Sunny	TEMP: Mid 60's
PRESENT AT SITE	
Oscar Nordstrom/Brian Tetor-Fuller	
George Rivera-Skyline Concrete	
Eric Beck-Tano	

CONTRACTOR, LABOR & EQUIPMENT	
QUANTITY	DESCRIPTION
1	Crew for Excavation-Tano
1	Crew for Footings-Skyline Concrete

CHARGABLE EQUIPMENT USED
Nuclear Density Gauge

GEOTECH INSPECTION REPORT

- Excavated 2 to 3 feet below existing finished floor subgrade to a firm natural footing subgrade or solid bedrock for Casino Footings C305, C880, C316, C869, C741, C317 to C327, C858, C732, C328 to C338, C847 and compacted with double-drum sheepsfoot roller. One Casino Footings C304 was re-inspected from 3/8/08 because it was not poured and it rained. Footing C316 was over-excavated 6 to 12-inches due to slight pumping and backfilled with 1-½-inch crush bedrock. Footings C741, C317, C319 and C320 were over-excavated 4 to 6-inches due to slight pumping and backfilled with on-site material and compacted with double-drum sheepsfoot roller. Footings C321 to C332 needed rock removal to achieve subgrade. Visual observation and probing of the footing subgrades verified a footing subgrade of 4 TSF bearing capacity.
- Excavated material from footings and from node line 17 and 17.5 from approximate node line L to M was taken to the roadway north of Sediment Basin #2 and three 12-inch thick loose lifts were placed from elevation $\pm 8'$ to $\pm 10'$ above former tennis court elevation in the southern part of the north of Sediment Basin #2 roadway, and compacted with single smooth drum 10-ton roller and single drum sheepsfoot 10-ton roller. Nuclear density testing, visual observation and probing of the subgrade verified the lifts were placed in accordance with design specifications.



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CONTRACTOR, LABOR & EQUIPMENT	
QUANTITY	DESCRIPTION
1	Crew for Excavation-Tano
1	Crew for Footings-Skyline Concrete

DATE: 9/2/2008	JOB NO. 5485
PROJECT: Concord	
LOCATION: Lake Kiamesha, NY	
CONTRACTOR: Fuller	
OWNER: Capelli	
WEATHER: Sunny	TEMP: Mid 80's
PRESENT AT SITE	
Oscar Nordstrom-Fuller	
George Rivera-Skyline Concrete	
Eric Beck-Tano	
Mark Skellet-MHE (Township Representative)	

CHARGABLE EQUIPMENT USED
Nuclear Density Gauge

GEOTECH INSPECTION REPORT

- Placed final 12-inch thick loose lift in area of Congress Hotel with natural on-site soil, originating from excavation of node line 16 to 17 from K to half way between M and N to finished floor subgrade, and compacted with single drum sheepsfoot 10-ton roller and single drum smooth 10-ton roller. Nuclear density testing, visual observation and probing of the subgrade verified the lift was placed in accordance with design specifications. Ganesh Persaud performed density testing.
- Excavated 2 to 3 feet below existing finished floor subgrade to a firm natural footing subgrade or previously approved compacted fill footing subgrade for Casino Footings C779, C240 to C245, C782, C229 to C234, C786, C218 to C223, C788, C207 to C212 and compacted with walk-behind double drum sheepsfoot roller. Visual observation and probing of the footing subgrades verified a footing subgrade of 4 TSF bearing capacity. Ganesh Persaud performed footing inspection.
- The additional excavated material, from the footings and excavated material from the excavation of node line 16 to 17 from K to half way between M and N, was taken to node line 13 to 14.5 from J to I. Two 12-inch thick loose lifts of natural on-site soil was placed and compacted with a single drum sheepsfoot 10-ton roller and a single smooth drum 10-ton roller. Nuclear density testing, visual observation and probing of the subgrade verified the lift was placed in accordance with design specifications. Ganesh Persaud performed density testing.



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DATE: 9/25/2008	JOB NO. 5485
PROJECT: Concord	
LOCATION: Lake Kiamesha, NY	
CONTRACTOR: Fuller	
OWNER: Cappelli	
WEATHER: Sunny	TEMP: High 60's
PRESENT AT SITE	
Oscar Nordstrom/Brian Tetor-Fuller	
George Rivera-Skyline Concrete	
Eric Beck-Tano	

CONTRACTOR, LABOR & EQUIPMENT	
QUANTITY	DESCRIPTION
1	Crew for Excavation-Tano
1	Crew for Footings-Skyline Concrete

CHARGABLE EQUIPMENT USED
Nuclear Density Gauge

GEOTECH INSPECTION REPORT

- Excavated 5 to 6 feet below existing finished floor subgrade to a firm natural footing subgrade for Casino Footing C882, however the elevation of the footing bottom changed to 2 to 3 feet below existing finished floor subgrade and two 12-inch thick lifts of recycled concrete aggregate were placed and compacted with double-drum sheepsfoot roller to bring footing to subgrade. Visual observation and probing of the footing subgrades verified a footing subgrade of 4 TSF bearing capacity.
- As part of the changed footing subgrades, Casino Footings C860 and C871 were removed.
- Excavated material from footing excavation, from hotel excavation and from node lines 17 to 18 from approximate node line L to N was taken to the tennis court area northeast of Sediment Basin #2 and two 12-inch thick loose lift were placed from elevation $\pm 1'$ to $\pm 3'$ above the former tennis court and compacted with single smooth drum 10-ton roller. Nuclear density testing, visual observation and probing of the subgrade verified the lift was placed in accordance with design specifications.
- Excavation continued for hotel towers in the northeast portion of the hotel. The excavation will be going to sound rock for a 10 TSF bearing capacity. A $\pm 8'$ cut was excavated today.
- Placement of coarse R.C.A. from crushed demolition concrete stockpile continued in previously proof-rolled area for slab-on-grade from Casino Footings C788/C207 to C217/C941 to C371/C817 to C711/C361. Placed 7-inches of R.C.A. and compacting with single smooth drum 10-ton roller.

- Excavation for UST-12 removal was backfill with three 12-inch thick lifts of natural on-site material. This completed the backfill of this excavation. Visual observation and probing of the subgrade verified the lifts were placed in accordance with design specifications.
- Excavation for UST-8 removal was backfilled today, due to approaching rain storm. No visual observation was performed on backfill operation. This excavation is located directly under the hotel tower and will be removed to bedrock at a later date.
- Casino Footings C371/C817, C360/C826, C349/C837, C338/C847, C327/C858 were excavated on their north side on 9/23/08 for an expansion of concrete. The footings were backfilled due to approaching rainstorm. Footing expansions will be excavated again at a later date.
- Casino Footings C881 and C892 were backfilled with two 12-inch thick lifts of natural on-site material to top of footings. Nuclear density testing, visual observation and probing of the subgrade verified the lifts were placed in accordance with design specifications. Pending a proof-roll of 2nd lift the footing backfill in this area is approved.



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DATE: 9/30/2008	JOB NO. 5485
PROJECT: Concord	
LOCATION: Lake Kiamesha, NY	
CONTRACTOR: Fuller	
OWNER: Cappelli	
WEATHER: P. Cloudy	TEMP: Mid 70's
PRESENT AT SITE	
Oscar Nordstrom/Brian Tetor-Fuller	
George Rivera-Skyline Concrete	
Eric Beck-Tano	

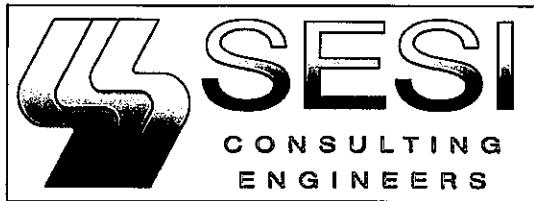
CONTRACTOR, LABOR & EQUIPMENT	
QUANTITY	DESCRIPTION
1	Crew for Excavation-Tano
1	Crew for Footings-Skyline Concrete

CHARGABLE EQUIPMENT USED
Nuclear Density Gauge

GEOTECH INSPECTION REPORT

- Excavated 2 to 3 feet below existing finished floor subgrade to a firm natural footing subgrade for Casino Footings C871 and C883 to a firm natural footing subgrade. Visual observation and probing of the footing subgrades verified a footing subgrade of 4 TSF bearing capacity. Casino Footing C882, previously approved 9/25/08, was re-approved.
- As part of the changed footing subgrades, Casino Footings C872 was removed and excavated 6 to 7 feet below existing finished floor subgrade to rock subgrade. Groundwater is present at the bottom 2 to 3 feet. A sump pump was set up to allow for forming and pouring. Footing subgrade will be approved prior to pour.
- During excavation of Casino Footing C860, contamination was found at footing subgrade elevation. Environmental testing was performed and footing approval will remain pending until results are attained. All excavated material after contamination was found was brought to the Temporary Contaminated Stockpile.
- Excavated material from footing excavations and from node lines 16 to 18 from approximate node line L to N was taken to the tennis court area east of Sediment Basin #2 and three 12-inch thick loose lift were placed from elevation $\pm 1'$ to $\pm 4'$ above the former tennis court and compacted with single smooth drum 10-ton roller. Nuclear density testing, visual observation and probing of the subgrade verified the lift was placed in accordance with design specifications.

- Casino Footings C861 and C873 were backfilled with two 12-inch thick lifts of recycled concrete aggregate and C862, C863, C874 and C875 were backfilled with one 12-inch thick lifts of recycled concrete aggregate. Nuclear density testing, visual observation and probing of the subgrade verified the lifts were placed in accordance with design specifications. Pending a proof-roll of 2nd lift the footing backfill in this area is approved.
- Excavated 2 to 3 feet below existing finished floor subgrade to a previously approved firm natural footing subgrade for footing expansion on north side of Casino Footings C371/C817, C360/C826, C349/C837, C338/C847 and C327/C858. The footings were changed and need to be expanded. Visual observation and probing of the expansion area of footing subgrades verified a footing subgrade of 4 TSF bearing capacity.



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CONTRACTOR, LABOR & EQUIPMENT	
QUANTITY	DESCRIPTION
1	Crew for Excavation-Tano
1	Crew for Footings-Skyline Concrete

DATE: 9/19/2008	JOB NO. 5485
PROJECT: Concord	
LOCATION: Lake Kiamesha, NY	
CONTRACTOR: Fuller	
OWNER: Cappelli	
WEATHER: Sunny	TEMP: Low 60's
PRESENT AT SITE	
Oscar Nordstrom/Brian Tetor-Fuller	
George Rivera-Skyline Concrete	
Eric Beck-Tano	

CHARGABLE EQUIPMENT USED
Nuclear Density Gauge

GEOTECH INSPECTION REPORT

- Excavated 2 to 3 feet below existing finished floor subgrade to a firm natural footing subgrade for Casino Footings C859 to C860, C870 to C871 and compacted with double-drum sheepsfoot roller. Visual observation and probing of the footing subgrades verified a footing subgrade of 4 TSF bearing capacity. Deo Persaud performed inspections.
- Excavation began for hotel towers in the northeast portion of the hotel. The excavation will be going to sound rock for a 10 TSF bearing capacity. A $\pm 8'$ cut was excavated today.
- Excavated material from footing excavations, from hotel excavation, from node line 18 from approximate node line K to L and from node line 17 from approximate node line L to N was taken to the roadway north of Sediment Basin #2 and one 12-inch thick loose lift were placed from elevation $\pm 18'$ to $\pm 19'$ above the former tennis court elevation on southern end of roadway section and compacted with single smooth drum 10-ton roller. This elevation is subgrade for the roadway. The remainder of material was placed on the slope of the roadway in 12-inch thick loose lifts and compacted with single smooth drum 10-ton roller. Nuclear density testing, visual observation and probing of the subgrade verified the lift was placed in accordance with design specifications. Deo Persaud and Rob Fioretti performed inspections.
- Placement of coarse R.C.A. from crushed demolition concrete stockpile continued in previously proof-rolled area for slab-on-grade from Casino Footings C788, C207 to C761, C284 to C217, C941 to C294, C891. Placed 7-inches of R.C.A. and compacting with single smooth drum 10-ton roller.



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DATE: 9/24/2008	JOB NO. 5485
PROJECT: Concord	
LOCATION: Lake Kiamesha, NY	
CONTRACTOR: Fuller	
OWNER: Cappelli	
WEATHER: Sunny	TEMP: High 60's
PRESENT AT SITE	
Oscar Nordstrom/Brian Tetor-Fuller	
George Rivera-Skyline Concrete	
Eric Beck-Tano	

CONTRACTOR, LABOR & EQUIPMENT	
QUANTITY	DESCRIPTION
1	Crew for Excavation-Tano
1	Crew for Footings-Skyline Concrete

CHARGABLE EQUIPMENT USED
Nuclear Density Gauge

GEOTECH INSPECTION REPORT

- Excavated 2 to 3 feet below existing finished floor subgrade to a firm natural footing subgrade for Casino Footings C861 to C863 and C872 to C875 and compacted with double-drum sheepsfoot roller. Visual observation and probing of the footing subgrades verified a footing subgrade of 4 TSF bearing capacity.
- Excavated material from footing excavations, from hotel excavation and from node lines 17 to 18 from approximate node line L to N was taken to the tennis court area northeast of Sediment Basin #2 and one 12-inch thick loose lift was placed from elevation $\pm 0'$ to $\pm 1'$ above the former tennis court and compacted with single smooth drum 10-ton roller. Nuclear density testing, visual observation and probing of the subgrade verified the lift was placed in accordance with design specifications.
- Excavation continued for hotel towers in the northeast portion of the hotel. The excavation will be going to sound rock for a 10 TSF bearing capacity. A $\pm 8'$ cut was excavated today.
- Placement of coarse R.C.A. from crushed demolition concrete stockpile continued in previously proof-rolled area for slab-on-grade from Casino Footings C788/C207 to C217/C941 to C371/C817 to C711/C361. Placed 7-inches of R.C.A. and compacting with single smooth drum 10-ton roller.
- Excavation for UST-12 removal was backfill with four 12-inch thick lifts of natural on-site material. Remainder will be performed tomorrow. Visual observation and probing of the subgrade verified the lifts were placed in accordance with design specifications.



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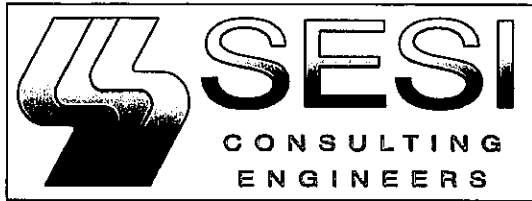
CONTRACTOR, LABOR & EQUIPMENT	
QUANTITY	DESCRIPTION
1	Crew for Excavation-Tano
1	Crew for Footings-Skyline Concrete

DATE: 9/26/2008	JOB NO. 5485
PROJECT: Concord	
LOCATION: Lake Kiamesha, NY	
CONTRACTOR: Fuller	
OWNER: Cappelli	
WEATHER: Rain	TEMP: Low 60's
PRESENT AT SITE	
Oscar Nordstrom/Brian Tetor-Fuller	
George Rivera-Skyline Concrete	
Eric Beck-Tano	

CHARGABLE EQUIPMENT USED
Nuclear Density Gauge

GEOTECH INSPECTION REPORT

- Excavated material from node lines 17 to 18 from approximate node line L to N was stockpiled at upper east tennis court area for use when condition permit.
- Placement of coarse R.C.A. from crushed demolition concrete stockpile continued in previously proof-rolled area for slab-on-grade from Casino Footings C788/C207 to C217/C941 to C371/C817 to C711/C361. Placed 7-inches of R.C.A. and compacting with single smooth drum 10-ton roller.
- No other construction work was performed due to rain.



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CONTRACTOR, LABOR & EQUIPMENT	
QUANTITY	DESCRIPTION
1	Crew for Excavation-Tano
1	Crew for Footings-Skyline Concrete

DATE: 9/16/2008	JOB NO. 5485
PROJECT: Concord	
LOCATION: Lake Kiamesha, NY	
CONTRACTOR: Fuller	
OWNER: Capelli	
WEATHER: P. Cloudy	TEMP: Mid 60's
PRESENT AT SITE	
Oscar Nordstrom/Brian Tetor-Fuller	
George Rivera-Skyline Concrete	
Eric Beck-Tano	

CHARGABLE EQUIPMENT USED
Nuclear Density Gauge

GEOTECH INSPECTION REPORT

- Excavated material from node line 18 from approximate node line K to L and from node line 17 from approximate node line L to N was taken to the roadway north of Sediment Basin #2 and two 12-inch thick loose lift were placed from elevation $\pm 17'$ to $\pm 18'$ above the former tennis court elevation on northern end of roadway section and compacted with single smooth drum 10-ton roller. Nuclear density testing, visual observation and probing of the subgrade verified the lift was placed in accordance with design specifications.
- Casino Footings C314 to C316, C869, C325 to C327, C858, C336 to C338, C847, C346 to C349, C837, C359 to C360, C826, C370 to C371, C817 were backfilled with two 12-inch thick lifts of recycled concrete aggregate. Casino Footings C313, C324, C334 to C335 were backfilled with one 12-inch thick lift of recycled concrete aggregate. Nuclear density testing, visual observation and probing of the subgrade verified the lifts were placed in accordance with design specifications. Pending a proof-roll of 2nd lift the footing backfill in this area is approved.



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DATE: 9/23/2008	JOB NO. 5485
PROJECT: Concord	
LOCATION: Lake Kiamesha, NY	
CONTRACTOR: Fuller	
OWNER: Cappelli	
WEATHER: Sunny	TEMP: High 60's
PRESENT AT SITE	
Oscar Nordstrom/Brian Tetor-Fuller	
George Rivera-Skyline Concrete	
Eric Beck-Tano	

CONTRACTOR, LABOR & EQUIPMENT	
QUANTITY	DESCRIPTION
1	Crew for Excavation-Tano
1	Crew for Footings-Skyline Concrete

CHARGABLE EQUIPMENT USED
Nuclear Density Gauge

GEOTECH INSPECTION REPORT

- Excavated 2 to 3 feet below existing finished floor subgrade to a previously approved firm natural footing subgrade for Casino Footings C371/C817, C360/C826, C349/C837, C338/C847, C327/C858 and compacted with double-drum sheepsfoot roller. The footings were changed and need to be expanded. Visual observation and probing of the expansion area of footing subgrades verified a footing subgrade of 4 TSF bearing capacity.
- Excavated material from node lines 17 to 18 from approximate node line L to N was taken to the roadway north of Sediment Basin #2 and one 12-inch thick loose lift were placed from elevation $\pm 20'$ to $\pm 22'$ above the former tennis court elevation on southern end of roadway section and compacted with single smooth drum 10-ton roller. This elevation is subgrade for this roadway section. Visual observation and probing of the subgrade verified the lift was placed in accordance with design specifications. Deo Persaud and Rob Fioretti performed inspections.
- Placement of coarse R.C.A. from crushed demolition concrete stockpile continued in previously proof-rolled area for slab-on-grade from Casino Footings C788/C207 to C217/C941 to C371/C817 to C711/C361. Placed 7-inches of R.C.A. and compacting with single smooth drum 10-ton roller.
- Casino Footings C818 to C819, C827 to C829, C838 to C841, C848 to C851, C859 to C860, C870 to C871 were backfilled with one 12-inch thick lifts of natural on-site material to top of footings. Visual observation and probing of the subgrade verified the lifts were placed in accordance with design specifications. Pending a proof-roll of 2nd lift the footing backfill in this area is approved.



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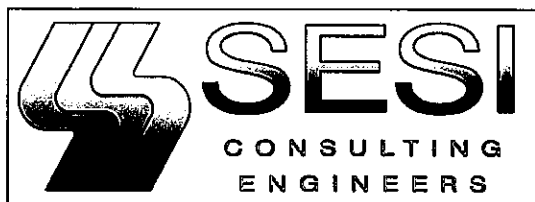
DATE: 9/18/2008	JOB NO. 5485
PROJECT: Concord	
LOCATION: Lake Kiamesha, NY	
CONTRACTOR: Fuller	
OWNER: Cappelli	
WEATHER: Sunny	TEMP: High 60's
PRESENT AT SITE	
Oscar Nordstrom/Brian Tetor-Fuller	
George Rivera-Skyline Concrete	
Eric Beck-Tano	

CONTRACTOR, LABOR & EQUIPMENT	
QUANTITY	DESCRIPTION
1	Crew for Excavation-Tano
1	Crew for Footings-Skyline Concrete

CHARGABLE EQUIPMENT USED
Nuclear Density Gauge

GEOTECH INSPECTION REPORT

- Excavated 2 to 3 feet below existing finished floor subgrade to a firm natural footing subgrade for Casino Footings C818 to C819, C827 to C829, C838 to C841, C848 to C851 and compacted with double-drum sheepsfoot roller. Footing C818 was over-excavated 2 feet below subgrade and C819 was over-excavated 3 feet below subgrade due to uncontrolled fills, and C828 was over-excavated 6 inches due to slight pumping. All over-excavated footings were backfilled with compacted lifts of recycled concrete aggregate (R.C.A.). Visual observation and probing of the footing subgrades verified a footing subgrade of 4 TSF bearing capacity. Deo Persaud and Rob Fioretti performed inspections.
- Excavated material from footing excavations, from node line 18 from approximate node line K to L and from node line 17 from approximate node line L to N was taken to the roadway north of Sediment Basin #2 and one 12-inch thick loose lift were placed from elevation $\pm 19'$ above the former tennis court elevation on northern end of roadway section and compacted with single smooth drum 10-ton roller. This elevation is subgrade for the roadway. The remainder of material was placed on the slope of the roadway in 12-inch thick loose lifts and compacted with single smooth drum 10-ton roller. Nuclear density testing, visual observation and probing of the subgrade verified the lift was placed in accordance with design specifications. Deo Persaud and Rob Fioretti performed inspections.
- Placement of coarse R.C.A. from crushed demolition concrete stockpile continued in previously proof-rolled area for slab-on-grade from Casino Footings C788, C207 to C761, C284 to C217, C941 to C294, C891. Placed 7-inches of R.C.A. and compacting with single smooth drum 10-ton roller.



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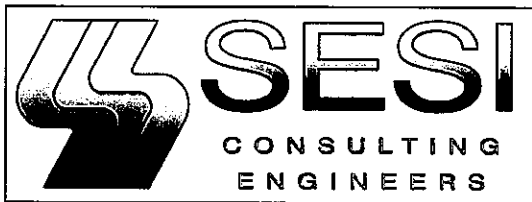
DATE: 9/29/2008	JOB NO. 5485
PROJECT: Concord	
LOCATION: Lake Kiamesha, NY	
CONTRACTOR: Fuller	
OWNER: Cappelli	
WEATHER: P. Cloudy	TEMP: Low 70's
PRESENT AT SITE	
Oscar Nordstrom/Brian Tetor-Fuller	
George Rivera-Skyline Concrete	
Eric Beck-Tano	

CONTRACTOR, LABOR & EQUIPMENT	
QUANTITY	DESCRIPTION
1	Crew for Excavation-Tano
1	Crew for Footings-Skyline Concrete

CHARGABLE EQUIPMENT USED
Nuclear Density Gauge

GEOTECH INSPECTION REPORT

- Excavated 2 to 3 feet below existing finished floor subgrade for Casino Footings C883 to C886. However, during excavation the depth of footings changed and excavation will continue tomorrow.
- Excavated material from footing excavation and from node lines 16 to 18 from approximate node line L to N was taken to the tennis court area east of Sediment Basin #2. All asphalt in tennis court was broken to allow for drainage and left in place. A 8-inch thick loose lift and a 12-inch thick loose lift was placed above the tennis court from elevation 0' to $\pm 1'$ and compacted with single smooth drum 10-ton roller. Nuclear density testing, visual observation and probing of the subgrade verified the lift was placed in accordance with design specifications.
- Casino Footings C862, C863, C874 and C875 were backfilled with one 12-inch thick lifts of recycled concrete aggregate. Visual observation and probing of the subgrade verified the lifts were placed in accordance with design specifications.
- Placement of coarse R.C.A. from crushed demolition concrete stockpile was completed in previously proof-rolled area for slab-on-grade from Casino Footings C788/C207 to C217/C941 to C371/C817 to C711/C361. Placed 7-inches of R.C.A. and compacting with single smooth drum 10-ton roller.



12A MAPLE AVENUE
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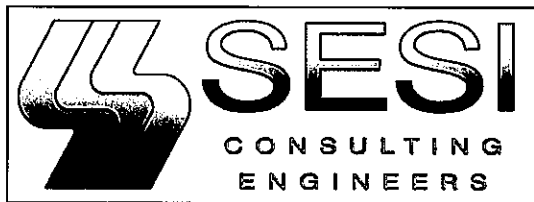
DATE: 9/22/2008	JOB NO. 5485
PROJECT: Concord	
LOCATION: Lake Kiamesha, NY	
CONTRACTOR: Fuller	
OWNER: Cappelli	
WEATHER: Sunny	TEMP: High 60's
PRESENT AT SITE	
Oscar Nordstrom/Brian Tetor-Fuller	
George Rivera-Skyline Concrete	
Eric Beck-Tano	

CONTRACTOR, LABOR & EQUIPMENT	
QUANTITY	DESCRIPTION
1	Crew for Excavation-Tano
1	Crew for Footings-Skyline Concrete

CHARGABLE EQUIPMENT USED
Nuclear Density Gauge

GEOTECH INSPECTION REPORT

- Excavated 2 to 3 feet below existing finished floor subgrade to a firm natural footing subgrade for Casino Footings C881 and C892 and compacted with double-drum sheepsfoot roller. Visual observation and probing of the footing subgrades verified a footing subgrade of 4 TSF bearing capacity.
- Excavated material from footing excavations, and from node lines 17 to 18 from approximate node line L to N was taken to the tennis court area northeast of Sediment Basin #2. All asphalt in tennis court was broken to allow for drainage and left in place. A 8-inch thick loose lift was placed above the tennis court elevation and compacted with single smooth drum 10-ton roller. Visual observation and probing of the subgrade verified the lift was placed in accordance with design specifications. Deo Persaud and Rob Fioretti performed inspections.
- Placement of coarse R.C.A. from crushed demolition concrete stockpile continued in previously proof-rolled area for slab-on-grade from Casino Footings C788, C207 to C761, C284 to C217, C941 to C294, C891. Placed 7-inches of R.C.A. and compacting with single smooth drum 10-ton roller
- Proof-rolling performed from Casino Footings C756, C295 to C305, C880 to C711, C361 to C371, C817. Area was rolled with a single smooth drum 10-ton roller. The area is approved for slab-on-grade preparation.
- Casino Footings C818 to C819, C827 to C829, C838 to C841, C848 to C851, C859 to C860, C870 to C871 were backfilled with one 12-inch thick lifts of on-site material. Nuclear density testing, visual observation and probing of the subgrade verified the lifts were placed in accordance with design specifications.



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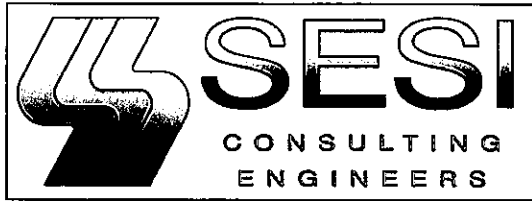
DATE: 9/17/2008	JOB NO. 5485
PROJECT: Concord	
LOCATION: Lake Kiamesha, NY	
CONTRACTOR: Fuller	
OWNER: Capelli	
WEATHER: Sunny	TEMP: High 60's
PRESENT AT SITE	
Oscar Nordstrom/Brian Tetor-Fuller	
George Rivera-Skyline Concrete	
Eric Beck-Tano	

CONTRACTOR, LABOR & EQUIPMENT	
QUANTITY	DESCRIPTION
1	Crew for Excavation-Tano
1	Crew for Footings-Skyline Concrete

CHARGABLE EQUIPMENT USED
Nuclear Density Gauge

GEOTECH INSPECTION REPORT

- Excavated material from node line 18 from approximate node line K to L and from node line 17 from approximate node line L to N was taken to the roadway north of Sediment Basin #2 and two 12-inch thick loose lift were placed from elevation $\pm 16'$ to $\pm 17'$ above the former tennis court elevation on southern end of roadway section and compacted with single smooth drum 10-ton roller. Nuclear density testing, visual observation and probing of the subgrade verified the lift was placed in accordance with design specifications. Deo Persaud and Rob Fioretti performed inspections.
- Casino Footings C342 to C344, C717, C350 to C355, C711, C361 to C362 were backfilled with two 12-inch thick lifts of recycled concrete aggregate. Nuclear density testing, visual observation and probing of the subgrade verified the lifts were placed in accordance with design specifications. Pending a proof-roll of 2nd lift the footing backfill in this area is approved. Deo Persaud and Rob Fioretti performed inspections.
- Proof-rolling performed from Casino Footings C788, C207 to C761, C284 to C217, C941 to C294, C891. Area was rolled with a single smooth drum 10-ton roller, and soft areas were removed and backfilled with recycled concrete aggregate. The area is approved for slab-on-grade preparation. Began placing 7-inches of coarse recycled concrete aggregate from crushed demolition concrete stockpile and compacting with single smooth drum 10-ton roller.



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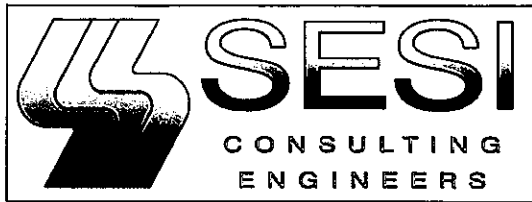
CONTRACTOR, LABOR & EQUIPMENT	
QUANTITY	DESCRIPTION
1	Excavating/Backfilling Crew - Tano

DATE: 10/9/2008	JOB NO. 5485
PROJECT: Concord	
LOCATION: Lake Kiamesha, NY	
CONTRACTOR: Fuller, JM Associates, Tano	
OWNER: Cappelli	
WEATHER: P. Cloudy	TEMP: Low 70's
PRESENT AT SITE	
Al Destefano/Oscar Nordstrom-Fuller	

CHARGABLE EQUIPMENT USED
Nuclear Density Gauge

GEOTECH FIELD REPORT

- UST-11 excavation, approximately 9 feet below existing grade, was backfilled with three feet of chipped stone bedrock and six 12-inch thick loose lifts of natural on-site material to the existing grade. The stone bedrock was placed in three lifts and compacted with the excavator bucket, and the natural on-site material was compacted with a double-drum sheepsfoot roller. Nuclear density testing, visual observation and probing of the subgrade verified the lift was placed in accordance with design specifications.



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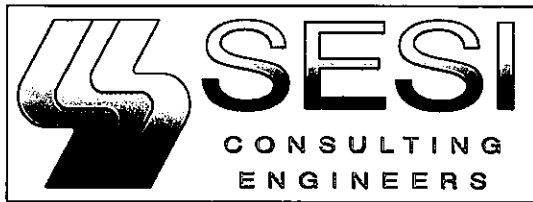
CONTRACTOR, LABOR & EQUIPMENT	
QUANTITY	DESCRIPTION
2	Crew for Excavation-Tano

DATE: 10/15/2008	JOB NO. 5485
PROJECT: Concord	
LOCATION: Lake Kiamesha, NY	
CONTRACTOR: Fuller	
OWNER: Cappelli	
WEATHER: P. Cloudy	TEMP: Mid 60's
PRESENT AT SITE	
Oscar Nordstrom-Fuller	
Eric Beck-Tano	

CHARGABLE EQUIPMENT USED
Nuclear Density Gauge

GEOTECH INSPECTION REPORT

- Placed two 12-inch thick loose lift of natural on-site material in existing excavation for hotel and compacted with a single smooth drum 10-ton roller. Due to design changes the existing hotel excavation is being filled, because the designed location of the hotel has moved. Nuclear density testing, visual observation and probing of the subgrade verified the lifts were placed in accordance with design specifications.
- Due to design changes of the slab-on-grade elevation of the Casino Area the previously approved slab-on-grade subgrade will be raised. Placed one 12-inch thick loose lift of R.C.A. from Casino Footings C771/C361 to C363 to C357 to C302 to C756/C259 and compacted with a single smooth drum 10-ton roller. Pending a proof-roll in this area the slab subgrade will be approved visually.



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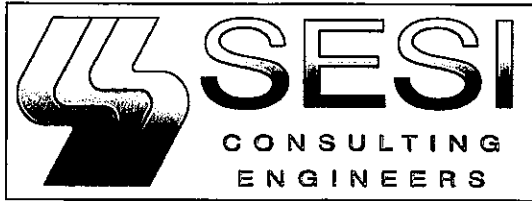
DATE: 10/10/2008	JOB NO. 5485
PROJECT: Concord	
LOCATION: Lake Kiamesha, NY	
CONTRACTOR: Fuller	
OWNER: Cappelli	
WEATHER: Sunny	TEMP: Mid 60's
PRESENT AT SITE	
Al Destefano/Oscar Nordstrom-Fuller	

CONTRACTOR, LABOR & EQUIPMENT	
QUANTITY	DESCRIPTION
1	Excavating/Backfilling Crew - Tano

CHARGABLE EQUIPMENT USED
Nuclear Density Gauge

GEOTECH FIELD REPORT

- UST-16 excavation, approximately 5 feet below existing grade, was backfilled with five 12-inch thick loose lifts of natural on-site material to the existing grade and compacted with a double-drum sheepsfoot roller. Nuclear density testing, visual observation and probing of the subgrade verified the lift was placed in accordance with design specifications.



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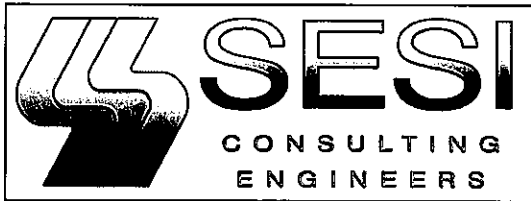
DATE: 10/8/2008	JOB NO. 5485
PROJECT: Concord	
LOCATION: Lake Kiamesha, NY	
CONTRACTOR: Fuller, JM Associates, Tano	
OWNER: Cappelli	
WEATHER: P. Cloudy	TEMP: Low 60's
PRESENT AT SITE	
Al Destefano/Oscar Nordstrom-Fuller	
Erick Beck-Tano	

CONTRACTOR, LABOR & EQUIPMENT	
QUANTITY	DESCRIPTION
1	Excavating/Backfilling Crew - Tano

CHARGABLE EQUIPMENT USED
Nuclear Density Gauge

GEOTECH FIELD REPORT

- Remainder of node excavation M-18, approximately 3 feet below existing grade and 30 feet diameter, was backfilled with three 12-inch thick loose lifts of natural on-site material to the existing grade and compacted with a double-drum sheepsfoot roller. Nuclear density testing, visual observation and probing of the subgrade verified the lift was placed in accordance with design specifications.
- Node excavation O-18, approximately 5 feet below existing grade and 30 feet diameter, was excavated one additional foot to remove all soft material to a firm natural subgrade, and was backfilled with six 12-inch thick loose lifts of natural on-site material to the existing grade and compacted with a double-drum sheepsfoot roller. Nuclear density testing, visual observation and probing of the subgrade verified the lift was placed in accordance with design specifications.



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DATE: 10/2/2008	JOB NO. 5485
PROJECT: Concord	
LOCATION: Lake Kiamesha, NY	
CONTRACTOR: Fuller	
OWNER: Cappelli	
WEATHER: P. Cloudy/Windy	TEMP: High 50's
PRESENT AT SITE	
Oscar Nordstrom/Brian Tetor-Fuller	
George Rivera-Skyline Concrete	
Eric Beck-Tano	

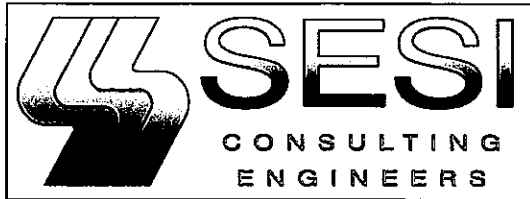
CONTRACTOR, LABOR & EQUIPMENT	
QUANTITY	DESCRIPTION
1	Crew for Excavation-Tano
1	Crew for Footings-Skyline Concrete

CHARGABLE EQUIPMENT USED
Nuclear Density Gauge

GEOTECH INSPECTION REPORT

- Excavated 6 to 7 feet below existing finished floor subgrade to a bedrock footing subgrade for Casino Footings C884 to C886. Groundwater is present at the 6 to 7 feet below existing finished floor subgrade. Sump pumps were set up to allow for forming and pouring. Visual observation and probing of the footing subgrades verified a footing subgrade of 4 TSF bearing capacity.
- Excavated material from footing excavations and from node lines 15 to 18 from approximate node line L to N was taken to the tennis court area northeast and east of Sediment Basin #2 and one 12-inch thick loose lift were placed at elevation $\pm 7'$ above the former tennis court and compacted with single smooth drum 10-ton roller. Nuclear density testing, visual observation and probing of the subgrade verified the lift was placed in accordance with design specifications.
- Casino Footings C872 was backfilled with two 12-inch thick lifts of recycled concrete aggregate and compacted with a double-drum sheepsfoot roller. Nuclear density testing, visual observation and probing of the subgrade verified the lifts were placed in accordance with design specifications.
- Placement of 1 to 2-inches of fine R.C.A. from crushed demolition concrete stockpile was performed on low areas of previously graded slab subgrade and compacted with a single smooth drum 10-ton roller.
- Under slab drainage pipe trenches were excavated 2 to 5 feet below existing slab subgrade in general areas between Casino Footings C219 to C221 to C265 to C263 and C329 to C331 to C353 to C351.

- Excavated 2 to 3 feet below existing finished floor subgrade to a previously approved firm natural footing subgrade for footing expansion on north side of Casino Footings C724/C339, C732/C328, C741/C317, C749/C306, C756/C259, C761/C284, C765/C273, C769/C262, C779/C240, C782/C229, C786/C218. The footings were changed and need to be expanded. Visual observation and probing of the expansion area of footing subgrades verified a footing subgrade of 4 TSF bearing capacity.



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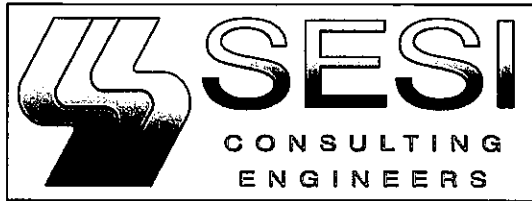
DATE: 10/6/2008	JOB NO. 5485
PROJECT: Concord	
LOCATION: Lake Kiamesha, NY	
CONTRACTOR: Fuller	
OWNER: Cappelli	
WEATHER: P. Cloudy/Windy	TEMP: Mid 50's
PRESENT AT SITE	
Oscar Nordstrom-Fuller	
George Rivera-Skyline Concrete	
Eric Beck-Tano	

CONTRACTOR, LABOR & EQUIPMENT	
QUANTITY	DESCRIPTION
1	Crew for Excavation-Tano
1	Crew for Footings-Skyline Concrete

CHARGABLE EQUIPMENT USED
Nuclear Density Gauge

GEOTECH INSPECTION REPORT

- Excavated 4 to 5 feet below existing finished floor subgrade to a bedrock footing subgrade for Casino Footings C901, C910, C920, C928, and C934. Visual observation and probing of the footing subgrades verified a footing subgrade of 4 TSF bearing capacity. However, before footings were poured work was stopped on site due to major changes to design.
- Stockpiled material in upper east tennis court area from 9/26/08 (from node lines 17 to 18 from approximate node line L to N) was taken to the tennis court area northeast and east of Sediment Basin #2 and one 12-inch thick lift was placed and compacted with single smooth drum 10-ton roller. Additional material from stockpile was taken to the Hotel excavation and two 12-inch thick lifts were placed on the west side of the excavation and compacted with single smooth drum 10-ton roller. Nuclear density testing, visual observation and probing of the subgrade verified the lift was placed in accordance with design specifications.
- Under slab drainage pipe trenches were excavated 2 to 5 feet below existing slab subgrade in general areas between Casino Footings C302 to C304 to C359 to C357.



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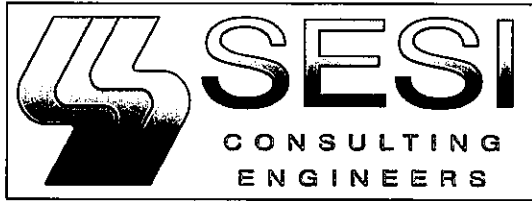
CONTRACTOR, LABOR & EQUIPMENT	
QUANTITY	DESCRIPTION
2	Crew for Excavation-Tano

DATE: 10/14/2008	JOB NO. 5485
PROJECT: Concord	
LOCATION: Lake Kiamesha, NY	
CONTRACTOR: Fuller	
OWNER: Cappelli	
WEATHER: P. Cloudy	TEMP: Low 70's
PRESENT AT SITE	
Oscar Nordstrom-Fuller	
Eric Beck-Tano	

CHARGABLE EQUIPMENT USED
Nuclear Density Gauge

GEOTECH INSPECTION REPORT

- Placed one 12-inch thick loose lift of natural on-site material in existing excavation for old hotel location and compacted with a single smooth drum 10-ton roller. Due to design changes the existing hotel excavation is being filled with natural on-site material, because the designed location of the hotel has moved. Nuclear density testing, visual observation and probing of the subgrade verified the lift was placed in accordance with design specifications.
- An excavation created during footing demolition from node lines J to K and 17 to 18 approximately 8 feet below existing grade was pumped of water. Two 12-inch thick loose lifts of chipped stone were placed and compacted with the excavator bucket, and six 12-inch thick loose lifts of natural on-site material were placed and compacted with a single smooth drum 10-ton roller. Nuclear density testing, visual observation and probing of the subgrade verified the lifts were placed in accordance with design specifications.



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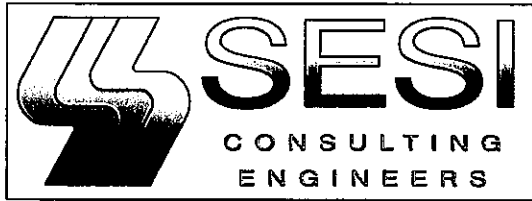
CONTRACTOR, LABOR & EQUIPMENT	
QUANTITY	DESCRIPTION
1	Excavating/Backfilling Crew - Tano

DATE: 10/7/2008	JOB NO. 5485
PROJECT: Concord	
LOCATION: Lake Kiamesha, NY	
CONTRACTOR: Fuller, JM Associates, Tano	
OWNER: Cappelli	
WEATHER: Sunny	TEMP: Mid 60's
PRESENT AT SITE	
Al Destefano/Oscar Nordstrom-Fuller	
Erick Beck-Tano	

CHARGABLE EQUIPMENT USED
Nuclear Density Gauge

GEOTECH FIELD REPORT

- Node excavation M-18, approximately 10 feet below existing grade and 30 feet diameter, was backfilled with seven lifts. Five 12-inch thick loose lifts of natural on-site material and two lifts of crushed sandstone bedrock that was excavated from the southern most point of the parking garage excavation (Approx. grid node lines 16 to 18 and along gridline N). The soil was compacted with a double-drum sheeps-foot roller. Nuclear density testing, visual observation and probing of the subgrade verified the lift was placed in accordance with design specifications.



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DATE: 10/3/2008	JOB NO. 5485
PROJECT: Concord	
LOCATION: Lake Kiamesha, NY	
CONTRACTOR: Fuller	
OWNER: Cappelli	
WEATHER: P. Cloudy/Windy	TEMP: Mid 50's
PRESENT AT SITE	
Oscar Nordstrom/Brian Tetor-Fuller	
George Rivera-Skyline Concrete	
Eric Beck-Tano	

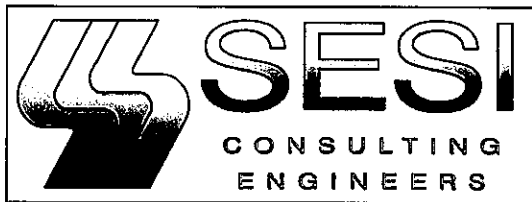
CONTRACTOR, LABOR & EQUIPMENT	
QUANTITY	DESCRIPTION
1	Crew for Excavation-Tano
1	Crew for Footings-Skyline Concrete

CHARGABLE EQUIPMENT USED
Nuclear Density Gauge

GEOTECH INSPECTION REPORT

- Excavated 4 to 5 feet below existing finished floor subgrade to a bedrock footing subgrade for Casino Footings C893 to C897. Visual observation and probing of the footing subgrades verified a footing subgrade of 4 TSF bearing capacity.
- Excavated material from footing excavations and from node lines 15 to 18 from approximate node line L to N was taken to the tennis court area north of Sediment Basin #2 and one 6-inch thick lift of chipped bedrock was placed on slopes and compacted with single smooth drum 10-ton roller. Visual observation and probing of the subgrade verified the lift was placed in accordance with design specifications.
- Expansion Casino Footings C724/C339, C732/C328, C741/C317, C749/C306, C756/C259, C761/C284, C765/C273, C769/C262, C774/C251, C782/C229, C786/C218, C371/C817, C360/C826, C349/C837, C338/C847, C327/C858, C316/C869, C305/C880, C294/C891, C283/C900, C272/C909 were backfilled with two 12-inch thick lifts, one of recycled concrete aggregate and one of natural on-site material, and compacted with a double-drum sheepsfoot roller. Nuclear density testing, visual observation and probing of the subgrade verified the lifts were placed in accordance with design specifications. It was discovered that C774/C251 was expanded instead of C779/C240. Report dated 10/2/08 shows C779/C240 approval and it should be noted that the expansion really took place on C774/C251. Site supervisors were notified of mistake.
- Under slab drainage pipe trenches were excavated 2 to 5 feet below existing slab subgrade in general areas between Casino Footings C219 to C221 to C265 to C263 and C329 to C331 to C353 to C351.

- Stockpile of on-site natural material (from previous excavations at node line 16 from K to L) located to the southeast of Sediment Basin #1 was used to place two 12-inch thick loose lifts to the southeast of Sediment Basin #1. The area was proofrolled before the lifts were placed and upon placement compacted with a single smooth drum 10-ton roller. Nuclear density testing, visual observation and probing of the subgrade verified the lifts were placed in accordance with design specifications.
- Excavated five test pits approximately 25 feet below existing grade in racetrack area. Test pits excavated to verify depth to rock.



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DATE: 10/1/2008	JOB NO. 5485
PROJECT: Concord	
LOCATION: Lake Kiamesha, NY	
CONTRACTOR: Fuller	
OWNER: Cappelli	
WEATHER: P. Cloudy	TEMP: Mid 60's
PRESENT AT SITE	
Oscar Nordstrom/Brian Tetor-Fuller	
George Rivera-Skyline Concrete	
Eric Beck-Tano	

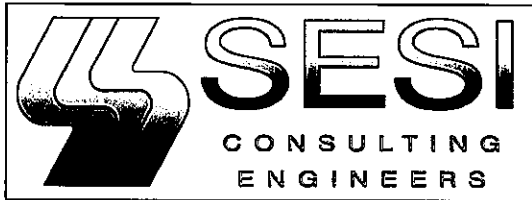
CONTRACTOR, LABOR & EQUIPMENT	
QUANTITY	DESCRIPTION
1	Crew for Excavation-Tano
1	Crew for Footings-Skyline Concrete

CHARGABLE EQUIPMENT USED
Nuclear Density Gauge

GEOTECH INSPECTION REPORT

- Previously excavated 6 to 7 feet below existing finished floor subgrade to a bedrock footing subgrade for Casino Footings C872. Groundwater is present at the 4 to 5 feet below existing finished floor subgrade. A sump pump was set up to allow for forming and pouring. Visual observation and probing of the footing subgrades verified a footing subgrade of 4 TSF bearing capacity.
- Excavated material from footing excavations and from node lines 16 to 18 from approximate node line L to N was taken to the tennis court area northeast and east of Sediment Basin #2 and two 12-inch thick loose lift were placed from elevation $\pm 4'$ to $\pm 6'$ above the former tennis court and compacted with single smooth drum 10-ton roller. Nuclear density testing, visual observation and probing of the subgrade verified the lift was placed in accordance with design specifications.
- Casino Footings C871, C882 and C883 were backfilled with two 12-inch thick lifts of recycled concrete aggregate and compacted with a double-drum sheepsfoot roller. Nuclear density testing, visual observation and probing of the subgrade verified the lifts were placed in accordance with design specifications. Pending a proof-roll of 2nd lift the footing backfill in this area is approved.
- Placement of 1 to 2-inches of fine R.C.A. from crushed demolition concrete stockpile was performed on low areas of previously graded slab subgrade and compacted with a single smooth drum 10-ton roller.

- Excavated 2 to 3 feet below existing finished floor subgrade to a previously approved firm natural footing subgrade for footing expansion on north side of Casino Footings C316/C869, C305/C880, C294/C891, C283/C900 and C372/C909. The footings were changed and need to be expanded. Visual observation and probing of the expansion area of footing subgrades verified a footing subgrade of 4 TSF bearing capacity.



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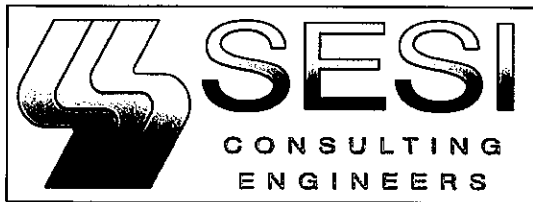
DATE: 10/23/2008	JOB NO. 5485
PROJECT: Concord	
LOCATION: Lake Kiamesha, NY	
CONTRACTOR: Fuller	
OWNER: Cappelli	
WEATHER: Sunny	TEMP: Low 50's
PRESENT AT SITE	
Oscar Nordstrom-Fuller	
Eric Beck-Tano	

CONTRACTOR, LABOR & EQUIPMENT	
QUANTITY	DESCRIPTION
2	Crew for Excavation-Tano

CHARGABLE EQUIPMENT USED
Nuclear Density Gauge

GEOTECH INSPECTION REPORT

- Placed five 12-thick loose lifts of fine grained R.C.A. from approximate elevation $\pm 1481'$ to $\pm 1485'$ on northern 1/3 of hotel footprint (including 25' east of footprint) and two 12-thick lifts from approximate elevation $\pm 1481'$ to $\pm 1482'$ on the remaining southern 2/3 of hotel footprint (including 25' east of footprint) and compacted with a single smooth drum 10-ton roller. Nuclear density testing, visual observation and probing of the subgrade verified the lifts were placed in accordance with design specifications.
- One 12-thick loose lift of 6-minus R.C.A. to approximate elevation $\pm 1478'$ on northern 1/3, 25 feet west of hotel footprint placed 10/22/08 was compacted with a single smooth drum 10-ton roller. Placed five 12-thick loose lifts of fine grained and 4-inch minus R.C.A. from approximate elevation $\pm 1479'$ to $\pm 1483'$ on northern 1/3, 25 feet west of hotel footprint and compacted with a single smooth drum 10-ton roller. Placed two 12-thick loose lifts of fine grained and 4-inch minus R.C.A. from approximate elevation $\pm 1478'$ to $\pm 1479'$ on southern 2/3, 25 feet west of hotel footprint and compacted with a single smooth drum 10-ton roller. Nuclear density testing, visual observation and probing of the subgrade verified the lifts were placed in accordance with design specifications.
- One 12-thick loose lift of on-site natural material, from previously compacted material north of Sediment Basin #2, from 25 to 50 feet east and west of hotel footprint and compacted with a single smooth drum 10-ton roller on 10/22/08. Placed three 12-thick loose lifts of on-site natural material, from excavation southwestern most tennis court south of Sediment Basin #2, from 25 to 50 feet east and west of hotel footprint and compacted with a single smooth drum 10-ton roller. The lifts are sloped away from the building footprint at an approximate 1:2 ratio. Nuclear density testing, visual observation and probing of the subgrade verified the lifts were placed in accordance with design specifications.



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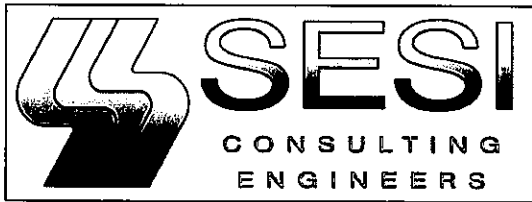
DATE: 10/24/2008	JOB NO. 5485
PROJECT: Concord	
LOCATION: Lake Kiamesha, NY	
CONTRACTOR: Fuller	
OWNER: Cappelli	
WEATHER: P. Cloudy	TEMP: High 50's
PRESENT AT SITE	
Oscar Nordstrom-Fuller	
Eric Beck-Tano	

CONTRACTOR, LABOR & EQUIPMENT	
QUANTITY	DESCRIPTION
2	Crew for Excavation-Tano

CHARGABLE EQUIPMENT USED
Nuclear Density Gauge

GEOTECH INSPECTION REPORT

- Placed two 12-thick loose lifts of fine grained R.C.A. from approximate elevation $\pm 1486'$ to $\pm 1487'$ on northern 1/3 of hotel footprint (including 25' east of footprint) and compacted with a single smooth drum 10-ton roller. Nuclear density testing, visual observation and probing of the subgrade verified the lifts were placed in accordance with design specifications.
- Placed four 12-thick loose lifts of fine grained and 4-inch minus R.C.A. from approximate elevation $\pm 1484'$ to $\pm 1487'$ on northern 1/3, 25 feet west of hotel footprint and compacted with a single smooth drum 10-ton roller. Placed one 12-thick loose lift of fine grained and 4-inch minus R.C.A. to approximate elevation $\pm 1480'$ on southern 2/3, 25 feet west of hotel footprint and compacted with a single smooth drum 10-ton roller. Nuclear density testing, visual observation and probing of the subgrade verified the lifts were placed in accordance with design specifications.
- Placed three 12-thick loose lifts of on-site natural material, from excavation southwestern most tennis court south of Sediment Basin #2, from 25 to 50 feet east and west of hotel footprint and compacted with a single smooth drum 10-ton roller. The lifts are sloped away from the building footprint at an approximate 1:2 ratio. Nuclear density testing, visual observation and probing of the subgrade verified the lifts were placed in accordance with design specifications.



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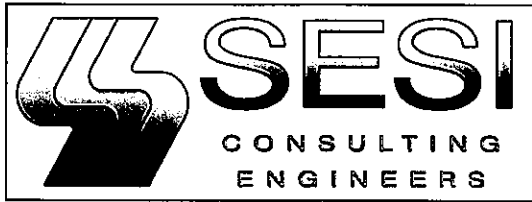
DATE: 11/13/2008	JOB NO. 5485
PROJECT: Concord	
LOCATION: Lake Kiamesha, NY	
CONTRACTOR: Fuller	
OWNER: Cappelli	
WEATHER: Rain	TEMP: Mid 40's
PRESENT AT SITE	
Oscar Nordstrom-Fuller	
Eric Beck-Tano	

CONTRACTOR, LABOR & EQUIPMENT	
QUANTITY	DESCRIPTION
1	Crew for Backfill-Tano

CHARGABLE EQUIPMENT USED

GEOTECH INSPECTION REPORT

- Earthwork had been previously stopped after 10/27/08 because SWPPP program was being revised and installed.
- Due to design changes of the slab-on-grade elevation of the Casino Area previously approved slab-on-grade subgrade will be raised. Placed one 12-inch thick loose lift of crushed bedrock, from parking garage #1 excavation, from Casino Footings 253 to C231 to C235 to C213 to C210 and compacted with a single smooth drum 10-ton roller. Work was performed in accordance with our recommendations, and upon a proof-roll in this area the slab subgrade will be given final approved.



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CONTRACTOR, LABOR & EQUIPMENT	
QUANTITY	DESCRIPTION
2	Crew for Excavation-Tano

DATE: 10/22/2008	JOB NO. 5485
PROJECT: Concord	
LOCATION: Lake Kiamesha, NY	
CONTRACTOR: Fuller	
OWNER: Cappelli	
WEATHER: P. Cloudy/Windy	TEMP: Low 40's
PRESENT AT SITE	
Oscar Nordstrom-Fuller	
Eric Beck-Tano	

CHARGABLE EQUIPMENT USED
Nuclear Density Gauge

GEOTECH INSPECTION REPORT

- Placed three 12-thick loose lifts of fine grained R.C.A. from approximate elevation $\pm 1478'$ to $\pm 1480'$ over entire hotel footprint (including 25' east of footprint) and compacted with a single smooth drum 10-ton roller. Nuclear density testing, visual observation and probing of the subgrade verified the lifts were placed in accordance with design specifications.
- Placed one 12-thick loose lift of 6-minus R.C.A., 25 feet west of hotel footprint, however the lift was not compacted. Placed one 12-thick loose lift of on-site natural material, from previously compacted material north of Sediment Basin #2, from 25 to 50 feet east and west of hotel footprint and compacted with a single smooth drum 10-ton roller. Tests will be performed tomorrow.



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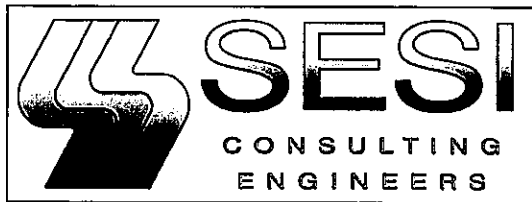
CONTRACTOR, LABOR & EQUIPMENT	
QUANTITY	DESCRIPTION
1	Crew for Backfill-Tano

DATE: 11/17/2008	JOB NO. 5485
PROJECT: Concord	
LOCATION: Lake Kiamesha, NY	
CONTRACTOR: Fuller	
OWNER: Cappelli	
WEATHER: Sunny	TEMP: High 30's
PRESENT AT SITE	
Oscar Nordstrom-Fuller	
Eric Beck-Tano	

CHARGABLE EQUIPMENT USED

GEOTECH INSPECTION REPORT

- No Geotechnical Work performed on 11/14/08.
- Due to design changes of the slab-on-grade elevation of the Casino Area the previously approved slab-on-grade subgrade will be raised. Placed one 12-inch thick loose lift of crushed bedrock, from parking garage #1 excavation, from Casino Footings 2774/C251 to C252 to C219 to C220 to C209 to C786/C207 and compacted with a single smooth drum 10-ton roller. Work was performed in accordance with our recommendations, and upon a proof-roll in this area the slab subgrade will be given final approved.



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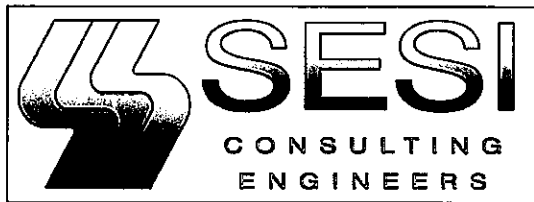
CONTRACTOR, LABOR & EQUIPMENT	
QUANTITY	DESCRIPTION
2	Crew for Excavation-Tano

DATE: 10/17/2008	JOB NO. 5485
PROJECT: Concord	
LOCATION: Lake Kiamesha, NY	
CONTRACTOR: Fuller	
OWNER: Cappelli	
WEATHER: Sunny	TEMP: High 60's
PRESENT AT SITE	
Oscar Nordstrom-Fuller	
Eric Beck-Tano	

CHARGABLE EQUIPMENT USED
Nuclear Density Gauge

GEOTECH INSPECTION REPORT

- Due to design changes of the slab-on-grade elevation of the Casino Area the previously approved slab-on-grade subgrade will be raised. Placed one 12-inch thick loose lift of natural on-site material (from soil previously placed in compacted lifts north of Sediment Basin #2) from Casino Footings C357 to C358 to C325 to C327/C858 to C261/C919 to C257 and compacted with a double drum sheepsfoot roller. Upon end of the day, the area was compacted with a single smooth drum 10-ton roller. Pending a proof-roll in this area the slab subgrade will be approved.
- Due to design changes the hotel location has been moved. Excavated approximately 15 feet in the northeast corner of the building footprint and proceeded south, because the contractor initially thought they needed to go to bedrock. However, due to further revision hotel will be placed on compacted lifts of R.C.A. material. A 2 foot by 2 foot by 2 foot excavation at the bottom of the excavation was excavated to locate bedrock. This excavation was backfilled with two 12-inch thick loose lifts of R.C.A. material and compacted with the excavator bucket. Nuclear density testing, visual observation and probing of the subgrade verified the lifts were placed in accordance with design specifications.



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CONTRACTOR, LABOR & EQUIPMENT	
QUANTITY	DESCRIPTION
2	Crew for Excavation-Tano

DATE: 10/21/2008	JOB NO. 5485
PROJECT: Concord	
LOCATION: Lake Kiamesha, NY	
CONTRACTOR: Fuller	
OWNER: Cappelli	
WEATHER: P. Cloudy/Showers	TEMP: Mid 40's
PRESENT AT SITE	
Oscar Nordstrom-Fuller	
Eric Beck-Tano	

CHARGABLE EQUIPMENT USED
Nuclear Density Gauge

GEOTECH INSPECTION REPORT

- Surveyed location of UST-8 that was not excavated and compacted previously on 10/20/08 was excavated 5 feet below existing grade to a firm natural material and proofrolled with a single drum 10-ton roller. Placed five 12-inch thick loose lifts of 6-inch minus R.C.A. material and compacted with a single smooth drum 10-ton roller. Contractor then placed proofrolled firm natural material in south end of hotel footprint and placed three 12-inch thick loose lifts of 6-inch minus R.C.A. material and compacted with a single smooth drum 10-ton roller. Finally, placed one 12-inch thick loose lift of 6-inch minus R.C.A. material over entire hotel footprint to approximate elevation $\pm 1477'$ and compacted with a single smooth drum 10-ton roller. Nuclear density testing, visual observation and probing of the subgrade verified the lift was placed in accordance with design specifications.



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DATE: 11/13/2008	JOB NO. 5485
PROJECT: Concord	
LOCATION: Lake Kiamesha, NY	
CONTRACTOR: Fuller	
OWNER: Cappelli	
WEATHER: Rain	TEMP: Mid 40's
PRESENT AT SITE	
Oscar Nordstrom-Fuller	
Eric Beck-Tano	

CONTRACTOR, LABOR & EQUIPMENT	
QUANTITY	DESCRIPTION
1	Crew for Backfill-Tano

CHARGABLE EQUIPMENT USED

GEOTECH INSPECTION REPORT

- Placed one 12-thick loose lift of fine grained R.C.A. from approximate elevation $\pm 1482'$ to $\pm 1483'$ on southern 2/3 of hotel footprint (including 25' east of footprint) and compacted with a single smooth drum 10-ton roller. Placed one 12-thick loose lift of fine grained R.C.A. from approximate elevation $\pm 1480'$ to $\pm 1481'$ on southern 2/3 of hotel footprint, 25 feet west of hotel footprint and compacted with a single smooth drum 10-ton roller. Nuclear density testing, visual observation and probing of the subgrade verified the lifts were placed in accordance with design specifications.
- Began demolition of the upper-most tennis courts. The courts are located on top of a 4-in. to 6-in. thick concrete pad. The first ± 100 -ft. (from south to north) of concrete contains rebar, post-tension rods and electrical conduit piping running throughout. The remaining concrete appears to just have a wire mat that is located in the bottom 1-in. to 2-in. of the pad.



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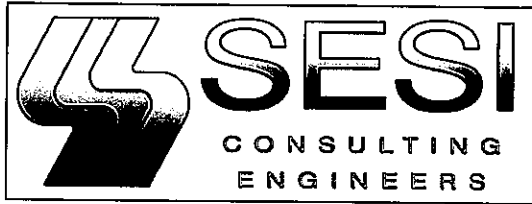
DATE: 10/16/2008	JOB NO. 5485
PROJECT: Concord	
LOCATION: Lake Kiamesha, NY	
CONTRACTOR: Fuller	
OWNER: Cappelli	
WEATHER: P. Cloudy	TEMP: Mid 60's
PRESENT AT SITE	
Oscar Nordstrom-Fuller	
Eric Beck-Tano	

CONTRACTOR, LABOR & EQUIPMENT	
QUANTITY	DESCRIPTION
2	Crew for Excavation-Tano

CHARGABLE EQUIPMENT USED

GEOTECH INSPECTION REPORT

- Minor grading with bulldozer occurred in area north of footing grid line CO8. Area was rolled firm with single smooth drum 10-ton roller.
- Due to design changes of the slab-on-grade elevation of the Casino Area the previously approved slab-on-grade subgrade will be raised. Placed one 12-inch thick loose lift of R.C.A. from Casino Footings C756/C259 to C302 to C269 to C268 to C246 to C243 to C769/C262 and compacted with a single smooth drum 10-ton roller. Pending a proof-roll in this area the slab subgrade will be approved.



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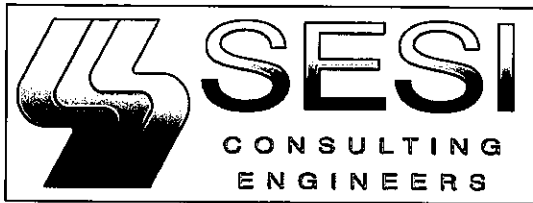
DATE: 10/20/2008	JOB NO. 5485
PROJECT: Concord	
LOCATION: Lake Kiamesha, NY	
CONTRACTOR: Fuller	
OWNER: Cappelli	
WEATHER: Sunny	TEMP: Low 50's
PRESENT AT SITE	
Oscar Nordstrom-Fuller	
Eric Beck-Tano	

CONTRACTOR, LABOR & EQUIPMENT	
QUANTITY	DESCRIPTION
2	Crew for Excavation-Tano

CHARGABLE EQUIPMENT USED
Nuclear Density Gauge

GEOTECH INSPECTION REPORT

- Due to design changes of the slab-on-grade elevation of the Casino Area the previously approved slab-on-grade subgrade will be raised. Placed one 12-inch thick loose lift of natural on-site material (from soil previously placed in compacted lifts north of Sediment Basin #2) from Casino Footings C257 to C261/C919 to C217/C941 to C213 and compacted with a single smooth drum 10-ton roller. Pending a proof-roll in this area the slab subgrade is approved.
- Due to design changes the hotel location has been moved. Previously excavated northern side of the building footprint (approx. location of UST-8) was proofrolled on firm natural material. Placed eight 12-inch thick loose lifts of 6-inch minus R.C.A material and compacted with a single smooth drum 10-ton roller. Nuclear density testing, visual observation and probing of the subgrade verified the lifts were placed in accordance with design specifications. Geod surveyed the exact location of UST-8 and five feet to the north and five feet to the west needs to be excavated and proofrolled before any compacted lifts are placed. This area requires excavation as it was filled previously with uncontrolled backfill with the thought that the building subgrade would be excavated to bedrock.



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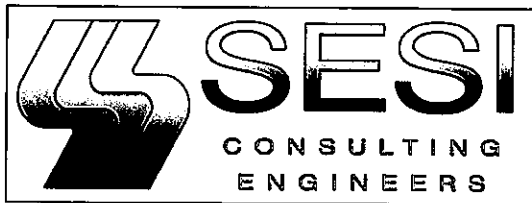
DATE: 11/24/2008	JOB NO. 5485
PROJECT: Concord	
LOCATION: Lake Kiamesha, NY	
CONTRACTOR: Fuller	
OWNER: Cappelli	
WEATHER: Windy	TEMP: High 30's
PRESENT AT SITE	
Oscar Nordstrom-Fuller	
Eric Beck-Tano	

CONTRACTOR, LABOR & EQUIPMENT	
QUANTITY	DESCRIPTION
1	Crew for Backfill-Tano

CHARGABLE EQUIPMENT USED
Nuclear Density Gauge

GEOTECH INSPECTION REPORT

- Ten inches of frost present in morning. Contractor removed frost and placed in frost stockpile on tennis court prior to placement of fill. Placed three 12-inch thick loose lift of 3-inch minus R.C.A. and 3-inch minus crushed bedrock (from parking garage #1 excavation) from approximate elevation $\pm 1475'$ to $\pm 1477'$ along entire hotel footprint width including 20 feet outside of footprint and from column line T-2.5 to T-4.5 and compacted with a single smooth drum 10-ton roller. Nuclear density testing, visual observation and probing of the subgrade verified the lifts were placed in accordance with design specifications. Deo Persaud performed inspection.



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DATE: 11/20/2008	JOB NO. 5485
PROJECT: Concord	
LOCATION: Lake Kiamesha, NY	
CONTRACTOR: Fuller	
OWNER: Cappelli	
WEATHER: P. Cloudy	TEMP: High 20's
PRESENT AT SITE	
Oscar Nordstrom-Fuller	
Eric Beck-Tano	

CONTRACTOR, LABOR & EQUIPMENT	
QUANTITY	DESCRIPTION
1	Crew for Backfill-Tano

CHARGABLE EQUIPMENT USED
Nuclear Density Gauge

GEOTECH INSPECTION REPORT

- Two to three inches of frost present in morning. Contractor used dozer to track and break the frost and compacted with a single smooth drum 10-ton roller. Placed three 12-inch thick loose lift of 3-inch minus R.C.A. from approximate elevation $\pm 1462'$ to $\pm 1464'$ along entire hotel footprint width including 20 feet outside of footprint and from 20 feet south of footprint to column line T-2.5 and compacted with a single smooth drum 10-ton roller. Nuclear density testing, visual observation and probing of the subgrade verified the lifts were placed in accordance with design specifications. Deo Persaud performed inspection.
- Two to three inches of frost present in morning. Contractor used dozer to track and break the frost and compacted with a single smooth drum 10-ton roller. Placed five 12-inch thick loose lifts of natural on-site material on subgrade of Casino Footings C385 to C996 to C990 to C984 approximately 7 feet below slab-on-grade elevation and compacted with a single smooth drum 10-ton roller. Natural on-site material came from the excavation of Sediment Basin #4 and parking garage #1 excavation. This backfills the former hotel location that had been excavated and is being filled with natural on-site material because the designed location of the hotel has been moved. Nuclear density testing, visual observation and probing of the subgrade verified the lift was placed in accordance with design specifications. Deo Persaud performed inspection.



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DATE: 11/19/2008	JOB NO. 5485
PROJECT: Concord	
LOCATION: Lake Kiamesha, NY	
CONTRACTOR: Fuller	
OWNER: Cappelli	
WEATHER: Windy	TEMP: High 20's
PRESENT AT SITE	
Oscar Nordstrom-Fuller	
Eric Beck-Tano	

CONTRACTOR, LABOR & EQUIPMENT	
QUANTITY	DESCRIPTION
1	Crew for Backfill-Tano

CHARGABLE EQUIPMENT USED
Nuclear Density Gauge

GEOTECH INSPECTION REPORT

- Veteran's Day Holiday 11/18/08. No work performed.
- A coffer dam was constructed across Sediment Basin #1 and the contractor pumped dry eastern half of Sediment Basin #1. Bottom of basin was mucked of all high moist silts and material was taken to the temporary topsoil stockpile. Placed 18 to 24-inch thick loose lift of 12-inch minus R.C.A. and heeled with excavator bucket. Another 18 to 24-inch thick loose lift of 12-inch minus R.C.A. was placed and compacted with a single smooth drum 10-ton roller. This achieved top of Sediment Basin#1 berm subgrade.
- Contractor removed onsite natural soil berm at southern end of hotel backfill pad due to design changes extending the hotel to the south. The soil was removed to a firm natural material and compacted with a single smooth drum 10-ton roller. Placed one 12-inch thick loose lift of 3-inch minus R.C.A. along entire hotel footprint width including 20 feet outside of footprint and from 20 feet south of footprint to column line T-2.5 and compacted with a single smooth drum 10-ton roller. Nuclear density testing, visual observation and probing of the subgrade verified the lifts were placed in accordance with design specifications.
- Two to three inches of frost present in morning. Contractor used dozer to track and break the frost and compacted with a single smooth drum 10-ton roller. Placed two 12-inch thick loose lifts of natural on-site material on subgrade of Casino Footings C385 to C996 to C990 to C984 approximately 9 feet below slab-on-grade elevation and compacted with a single smooth drum 10-ton roller. Natural on-site material came from the excavation of Sediment Basin #4 and parking garage #1 excavation. This backfills the former hotel location that had been excavated and is being filled with natural on-site material because the

the subgrade verified the lift was placed in accordance with design specifications.



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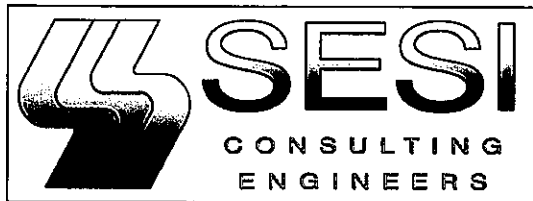
DATE: 11/25/2008	JOB NO. 5485
PROJECT: Concord	
LOCATION: Lake Kiamesha, NY	
CONTRACTOR: Fuller	
OWNER: Cappelli	
WEATHER: SNOW	TEMP: Mid 30's
PRESENT AT SITE	
Oscar Nordstrom-Fuller	
Eric Beck-Tano	

CONTRACTOR, LABOR & EQUIPMENT	
QUANTITY	DESCRIPTION
2	Crew for Backfill-Tano

CHARGABLE EQUIPMENT USED
Nuclear Density Gauge

GEOTECH INSPECTION REPORT

- Five inches of frost present in morning. Contractor removed frost and placed in frost stockpile on tennis court prior to placement of fill. Placed four 12-inch thick loose lift of 3-inch minus R.C.A. and 3-inch minus crushed bedrock (from parking garage #1 excavation) from approximate elevation $\pm 1478'$ to $\pm 1481'$ along entire hotel footprint width including 20 feet outside of footprint and from column line T-2.5 to T-4.5 and compacted with a single smooth drum 10-ton roller. Nuclear density testing, visual observation and probing of the subgrade verified the lifts were placed in accordance with design specifications. Deo Persaud performed inspection.
- Working mat of concrete approximately 3 to 9-inches thick was poured from the south end of the hotel pad to column line T-2.5.
- Utility trench in north to south direct in second tier tennis court was excavated to a firm natural subgrade to the north of the future hotel pad. Utility trench from east to west in second tier tennis court was excavated to a firm natural subgrade to west of the above utility trench. Placed four 12-inch thick loose lifts of 6-inch minus crushed bedrock and compacted with a double drum sheepsfoot roller. Visual observation and probing of the subgrade verified the lifts were placed in accordance with design specifications. Deo Persaud performed inspection.



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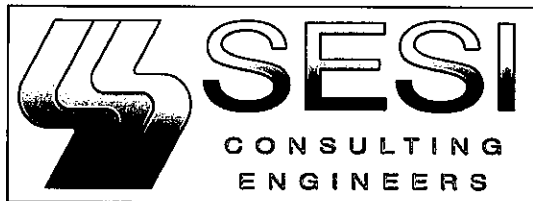
DATE: 12/1/2008	JOB NO. 5485
PROJECT: Concord	
LOCATION: Lake Kiamesha, NY	
CONTRACTOR: Fuller	
OWNER: Cappelli	
WEATHER: P. Cloudy	TEMP: High 40's
PRESENT AT SITE	
Oscar Nordstrom-Fuller	
Eric Beck-Tano	

CONTRACTOR, LABOR & EQUIPMENT	
QUANTITY	DESCRIPTION
2	Crew for Backfill-Tano

CHARGABLE EQUIPMENT USED

GEOTECH INSPECTION REPORT

- Two to three inches of frost present in morning. Contractor used dozer to track and break the frost and compacted with a single smooth drum 10-ton roller. Placed one 12-inch thick loose lift of 3-inch minus crushed bedrock (from parking garage #1 excavation) from approximate elevation $\pm 1478'$ to $\pm 1479'$ along entire hotel footprint width including 20 feet outside of footprint to the east and from column line T-5 to T-7 and compacted with a single smooth drum 10-ton roller. Visual observation and probing of the subgrade verified the lifts were placed in accordance with design specifications.
- Contractor excavated approximately 8 feet of natural soil placed from 25 feet to 50 feet outside of former hotel footprint from column line T-4 to T-8 to a firm natural soil subgrade and compacted with a single smooth drum 10-ton roller. This location is approximately from the west wall of the new hotel footprint to 20 feet west of the hotel footprint. Placed two 12-inch thick loose lifts of 12-inch minus R.C.A. and from approximate elevation $\pm 1471'$ to $\pm 1472'$ and compacted with a single smooth drum 10-ton roller. Placed three 12-inch thick loose lifts of 3-inch minus crushed bedrock from approximate elevation $\pm 1473'$ to $\pm 1475'$ and compacted with a single smooth drum 10-ton roller. Visual observation and probing of the subgrade verified the lifts were placed in accordance with design specifications.



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DATE: 12/2/2008	JOB NO. 5485
PROJECT: Concord	
LOCATION: Lake Kiamesha, NY	
CONTRACTOR: Fuller	
OWNER: Cappelli	
WEATHER: Sunny	TEMP: Mid 30's
PRESENT AT SITE	
Oscar Nordstrom-Fuller	
Eric Beck-Tano	

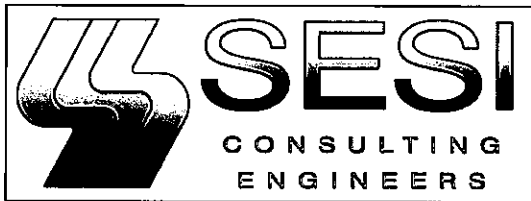
CONTRACTOR, LABOR & EQUIPMENT	
QUANTITY	DESCRIPTION
2	Crew for Backfill-Tano

CHARGABLE EQUIPMENT USED
Nuclear Density Gauge

GEOTECH INSPECTION REPORT

- No frost present today. Placed two 12-inch thick loose lift of 3-inch minus R.C.A. from approximate elevation $\pm 1482'$ to $\pm 1484'$ along entire hotel footprint width including 20 feet outside of footprint from column line T-5 to T-8 and compacted with a single smooth drum 10-ton roller. Nuclear density testing, visual observation and probing of the subgrade verified the lifts were placed in accordance with design specifications.
- No frost present today. Placed three 12-inch thick loose lifts of 3-inch minus crushed concrete from 20' to 60' west of hotel footprint in area of indoor pool structure from approximate elevation $\pm 1475'$ to $\pm 1478'$ and compacted with a single smooth drum 10-ton roller. No density testing performed on crushed bedrock due to lack amount of gravel in material. Visual observation and probing of the subgrade verified the lifts were placed in accordance with design specifications.
- Contractor excavated approximately 10 feet of undisturbed existing grade fill material to a firm natural soil subgrade to elevation $\pm 1463'$. This was performed from 60' to 90' west of hotel footprint in area of indoor pool structure. The soil was stockpiled in the frost stockpile on the tennis courts as it was highly moist. In area 75' to 90 west of hotel footprint concrete slab from tennis court was broken and a manhole and footing drain pipe were discovered. The manhole and footing drain were removed.

- Area in Casino footprint (to the east of the former ice rink in former building footprint) for approximate Casino Footings C12 to C15 to C772 to C53 was previously cut 3 to 4 feet to a firm natural soil subgrade to elevation $\pm 1475'$ and proofrolled with a single smooth drum 10-ton roller. Placed three 12-inch thick loose lifts of natural onsite material, from excavation 20 to 100 feet south of OU-1B in racetrack area, from approximate elevation $\pm 1475'$ to $\pm 1478'$ and compacted with a single smooth drum 10-ton roller. Nuclear density testing, visual observation and probing of the subgrade verified the lifts were placed in accordance with design specifications.



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CONTRACTOR, LABOR & EQUIPMENT	
QUANTITY	DESCRIPTION
2	Crew for Backfill-Tano

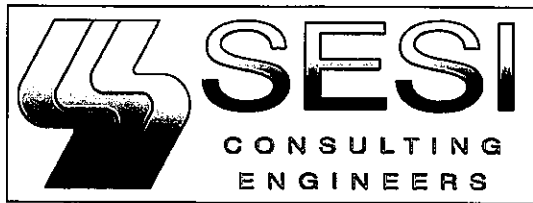
DATE: 12/1/2008	JOB NO. 5485
PROJECT: Concord	
LOCATION: Lake Kiamesha, NY	
CONTRACTOR: Fuller	
OWNER: Cappelli	
WEATHER: P. Cloudy	TEMP: High 40's
PRESENT AT SITE	
Oscar Nordstrom-Fuller	
Eric Beck-Tano	

CHARGABLE EQUIPMENT USED
Nuclear Density Gauge

GEOTECH INSPECTION REPORT

- No frost present today. Area previously filled 11/28/08 from 25 feet to 50 feet outside of former hotel footprint from column line T-4 to T-8 (approx. from 5 feet inside to 20 feet outside of the west wall of new hotel). Placed four 12-inch thick loose lifts of 3-inch minus crushed bedrock, from parking garage #1 excavation, from approximate elevation $\pm 1475'$ to $\pm 1479'$ and compacted with a single smooth drum 10-ton roller. This brought area up to grade of the remaining areas on the hotel footprint fill. No density testing performed on crushed bedrock due to size of particles in material. Visual observation and probing of the subgrade verified the lifts were placed in accordance with design specifications.
- No frost present today. Placed three 12-inch thick loose lift of 3-inch minus R.C.A. and 3-inch minus crushed bedrock, from parking garage #1 excavation, from approximate elevation $\pm 1479'$ to $\pm 1482'$ along entire hotel footprint width including 20 feet outside of footprint from column line T-5 to T-8 and compacted with a single smooth drum 10-ton roller. Nuclear density testing, visual observation and probing of the subgrade verified the lifts were placed in accordance with design specifications.
- Chris Zwingle had a meeting onsite to discuss frost issues. It was decided that the remainder of the hotel footprint will have 22-inches of $\frac{3}{4}$ -inch clean crushed stone placed on it to subgrade to avoid the effects of frost. The areas already with a mud mat of concrete need to be covered and heated when temperature overnight are below freezing. A letter will be issued by SESI regarding these procedures.

- Contractor excavated approximately 10 feet of previous fill material to a firm natural soil subgrade to elevation $\pm 1470'$. This was performed from 20' to 60' west of hotel footprint in area of indoor pool structure. The soil was stockpiled in a location close by for reuse. Placed five 12-inch thick loose lifts of natural material from stripped material and racetrack excavations (of the new tracking pad approximately 250 feet southwest of the gas station and swale excavation southwest of sediment basin #3T) from approximate elevation $\pm 1470'$ to $\pm 1475'$ and compacted with a single smooth drum 10-ton roller. Nuclear density testing, visual observation and probing of the subgrade verified the lifts were placed in accordance with design specifications.



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DATE: 11/22/2008	JOB NO. 5485
PROJECT: Concord	
LOCATION: Lake Kiamesha, NY	
CONTRACTOR: Fuller	
OWNER: Cappelli	
WEATHER: Windy	TEMP: Low 20's
PRESENT AT SITE	
Oscar Nordstrom-Fuller	
Eric Beck-Tano	

CONTRACTOR, LABOR & EQUIPMENT	
QUANTITY	DESCRIPTION
1	Crew for Backfill-Tano

CHARGABLE EQUIPMENT USED
Nuclear Density Gauge

GEOTECH INSPECTION REPORT

- Six inches of frost present in morning. Contractor removed frost and placed in frost stockpile on tennis court prior to placement of fill. Placed three 12-inch thick loose lift of 3-inch minus R.C.A. and 3-inch minus crushed bedrock (from parking garage #1 excavation) from approximate elevation $\pm 1472'$ to $\pm 1474'$ along entire hotel footprint width including 20 feet outside of footprint and from 20 feet south of footprint to column line T-2.5 and compacted with a single smooth drum 10-ton roller. Nuclear density testing, visual observation and probing of the subgrade verified the lifts were placed in accordance with design specifications.



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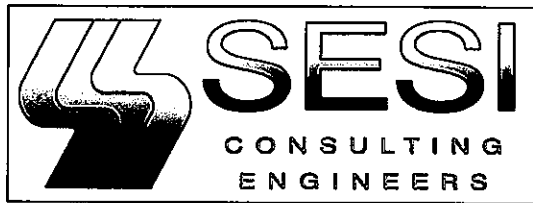
CONTRACTOR, LABOR & EQUIPMENT	
QUANTITY	DESCRIPTION
2	Crew for Backfill-Tano

DATE: 11/26/2008	JOB NO. 5485
PROJECT: Concord	
LOCATION: Lake Kiamesha, NY	
CONTRACTOR: Fuller	
OWNER: Cappelli	
WEATHER: P. Cloudy	TEMP: Mid 30's
PRESENT AT SITE	
Oscar Nordstrom-Fuller	
Eric Beck-Tano	

CHARGABLE EQUIPMENT USED

GEOTECH INSPECTION REPORT

- Two to three inches of frost present in morning. Contractor used dozer to track and break the frost and compacted with a single smooth drum 10-ton roller. Placed four 12-inch thick loose lift of 3-inch minus crushed bedrock (from parking garage #1 excavation) from approximate elevation $\pm 1482'$ to $\pm 1485'$ along entire hotel footprint width including 20 feet outside of footprint and from column line T-3.5 to T-4.5 and compacted with a single smooth drum 10-ton roller. Visual observation and probing of the subgrade verified the lifts were placed in accordance with design specifications. Deo Persaud performed inspection.
- Utility trench from north to south in second tier tennis court was excavated to a firm natural subgrade to in the future hotel pad. Placed four 12-inch thick loose lifts of 6-inch minus crushed bedrock and compacted with a double drum sheepsfoot roller. Visual observation and probing of the subgrade verified the lifts were placed in accordance with design specifications. Deo Persaud performed inspection.



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DATE: 11/21/2008	JOB NO. 5485
PROJECT: Concord	
LOCATION: Lake Kiamesha, NY	
CONTRACTOR: Fuller	
OWNER: Cappelli	
WEATHER: P. Cloudy	TEMP: High 20's
PRESENT AT SITE	
Oscar Nordstrom-Fuller	
Eric Beck-Tano	

CONTRACTOR, LABOR & EQUIPMENT	
QUANTITY	DESCRIPTION
1	Crew for Backfill-Tano

CHARGABLE EQUIPMENT USED
Nuclear Density Gauge

GEOTECH INSPECTION REPORT

- Two to three inches of frost present in morning. Contractor used dozer to track and break the frost and compacted with a single smooth drum 10-ton roller. Placed six 12-inch thick loose lift of 3-inch minus R.C.A. and 3-inch minus crushed bedrock (from parking garage #1 excavation) from approximate elevation $\pm 1465'$ to $\pm 1471'$ along entire hotel footprint width including 20 feet outside of footprint and from 20 feet south of footprint to column line T-2.5 and compacted with a single smooth drum 10-ton roller. Nuclear density testing, visual observation and probing of the subgrade verified the lifts were placed in accordance with design specifications. Deo Persaud performed inspection.



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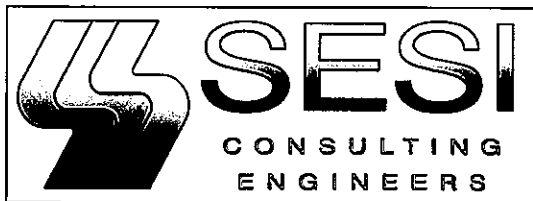
DATE: 12/8/2008	JOB NO. 5485
PROJECT: Concord	
LOCATION: Lake Kiamesha, NY	
CONTRACTOR: Fuller	
OWNER: Cappelli	
WEATHER: Sunny	TEMP: Low 20's
PRESENT AT SITE	
Oscar Nordstrom-Fuller	
Eric Beck-Tano	

CONTRACTOR, LABOR & EQUIPMENT	
QUANTITY	DESCRIPTION
3	Crew for Backfill-Tano

CHARGABLE EQUIPMENT USED
Nuclear Density Gauge

GEOTECH INSPECTION REPORT

- Twelve inches of frost present in morning. Contractor removed frost and placed in frost stockpile on to the north of Sediment Basin #1 prior to placement of fill. Placed four 12-inch thick loose lifts of natural onsite material, from racetrack excavation 20 to 100 feet south of OU-1B, from 20' to 60' west of hotel footprint in area of indoor pool structure, from column lines T-1 to T-7, from approximate elevation $\pm 1478'$ to $\pm 1482'$ and compacted with a single smooth drum 10-ton roller. Nuclear density testing, visual observation and probing of the subgrade verified the lifts were placed in accordance with design specifications. Deo Persaud performed inspection.
- Rat slab poured in hotel footprint from column line T-3.5 to T-7 except for elevator pit area which is completed.
- Twelve inches of frost present in morning. Contractor removed frost and placed in frost stockpile on to the north of second tier tennis court prior to placement of fill. Placed two 12-inch thick loose lifts of natural onsite material, from racetrack excavation 20 to 100 feet south of OU-1B, from approximate elevation $\pm 1465'$ to $\pm 1467'$, inside of Ballroom footprint on the north side of second tier tennis court and compacted with a single smooth drum 10-ton roller. Nuclear density testing, visual observation and probing of the subgrade verified the lifts were placed in accordance with design specifications. Deo Persaud performed inspection.
- Twelve inches of frost present in morning, eight inches removed. Contractor removed frost and took it to the temporary topsoil stockpile. Placed stabilization fabric between Casino column line CF to CD and C17 to C19. Placed 8-inches of $\frac{3}{4}$ -inch clean crushed stone as working surface for upper level construction. Area will be proofrolled in spring prior to slab-on-grade approval.



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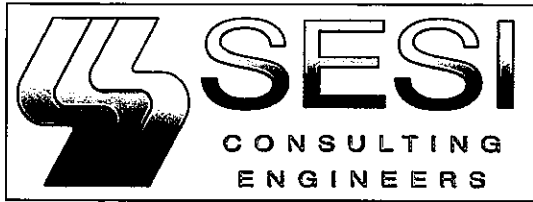
CONTRACTOR, LABOR & EQUIPMENT	
QUANTITY	DESCRIPTION
1	Crew for Backfill-Tano

DATE: 12/12/2008	JOB NO. 5485
PROJECT: Concord	
LOCATION: Lake Kiamesha, NY	
CONTRACTOR: Fuller	
OWNER: Cappelli	
WEATHER: P. Cloudy	TEMP: Mid 30's
PRESENT AT SITE	
Oscar Nordstrom-Fuller	
Eric Beck-Tano	

CHARGABLE EQUIPMENT USED

GEOTECH INSPECTION REPORT

- No frost present in morning. Contractor removed 8-inches of crushed bedrock material and stockpiled for future use. Placed stabilization fabric between Casino column line CF to CB and C14 to C15. Placed 8-inches of ¾-inch clean crushed stone as working surface for upper level construction. Area will be proofrolled in spring prior to slab-on-grade approval.



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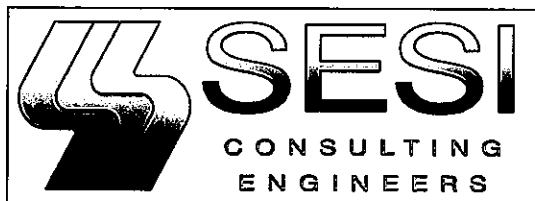
DATE: 12/6/2008	JOB NO. 5485
PROJECT: Concord	
LOCATION: Lake Kiamesha, NY	
CONTRACTOR: Fuller	
OWNER: Cappelli	
WEATHER: P. Cloudy	TEMP: High 20's
PRESENT AT SITE	
Oscar Nordstrom-Fuller	
Eric Beck-Tano	

CONTRACTOR, LABOR & EQUIPMENT	
QUANTITY	DESCRIPTION
3	Crew for Backfill-Tano

CHARGABLE EQUIPMENT USED

GEOTECH INSPECTION REPORT

- No frost present in areas to be worked this morning. 18-inches of $\frac{3}{4}$ -inch clean crushed stone placed on hotel footprint from T-3.5 to T-12 except for elevator pit area which is already complete. Stone was compacted with a single drum 10-ton roller.



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CONTRACTOR, LABOR & EQUIPMENT	
QUANTITY	DESCRIPTION
3	Crew for Backfill-Tano

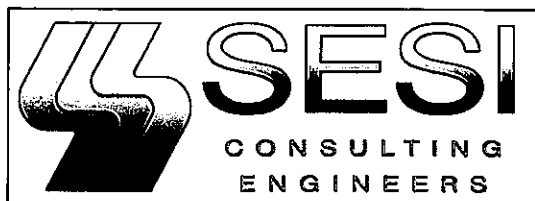
DATE: 12/4/2008	JOB NO. 5485
PROJECT: Concord	
LOCATION: Lake Kiamesha, NY	
CONTRACTOR: Fuller	
OWNER: Cappelli	
WEATHER: Rain	TEMP: Mid 30's
PRESENT AT SITE	
Oscar Nordstrom-Fuller	
Eric Beck-Tano	

CHARGABLE EQUIPMENT USED
Nuclear Density Gauge

GEOTECH INSPECTION REPORT

- No frost present this morning. Placed eight 12-inch thick loose lifts of 3-inch minus crushed bedrock, in hotel elevator pit area, from the west foundation wall to column line TA1.5 and column line T-9 to T-12 from approximate elevation $\pm 1473'$ to $\pm 1481'$ and compacted with a single smooth drum 10-ton roller. Area previously excavated approximately 8 feet to a firm natural soil subgrade to approximate elevation $\pm 1473'$. 22-inches of $\frac{3}{4}$ -inch clean crushed stone placed from the west foundation wall to column line TA2 and column line T-9 to T-12 to approximate elevation $\pm 1481'$. No density testing performed on crushed bedrock due to amount of gravel in material. Visual observation and probing of the subgrade verified the lifts were placed in accordance with design specifications. Deo Persaud performed inspection.
- No frost present this morning. Placed 12 to 18-inches of $\frac{3}{4}$ -inch clean crushed stone over entire hotel footprint area from column lines T-3.5 to T-12 and compacted with a single smooth drum 10-ton roller. Area was either at approximate elevation $\pm 1485'$ or was stripped, if necessary, 12 to 18-inches to approximate elevation $\pm 1485'$. This was done to allow for 22-inches of $\frac{3}{4}$ -inch clean crushed stone and a 2-inch concrete rat slab before achieving elevation $\pm 1487'$ which is footing subgrade. Placement of $\frac{3}{4}$ -inch clean crushed stone will continue as required to achieve 22-inches. No density testing performed on crushed bedrock due to amount of gravel in material. Visual observation and probing of the subgrade verified the lifts were placed in accordance with design specifications. Deo Persaud performed inspection.

- No frost present this morning. Placed three 12-inch thick loose lifts of natural onsite material, from racetrack excavation 20 to 100 feet south of OU-1B, from approximate elevation $\pm 1463'$ to $\pm 1466'$, inside of Ballroom footprint on the north side of second tier tennis court and compacted with a single smooth drum 10-ton roller. Tennis court concrete was previously broken, tracked with a dozer and left in place with large pieces of metal removed. Nuclear density testing, visual observation and probing of the subgrade verified the lifts were placed in accordance with design specifications. Deo Persaud performed inspection.



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DATE: 12/15/2008	JOB NO. 5485
PROJECT: Concord	
LOCATION: Lake Kiamesha, NY	
CONTRACTOR: Fuller	
OWNER: Cappelli	
WEATHER: P. Cloudy	TEMP: Mid 50's
PRESENT AT SITE	
Oscar Nordstrom-Fuller	
Eric Beck-Tano	

CONTRACTOR, LABOR & EQUIPMENT	
QUANTITY	DESCRIPTION
1	Crew for Backfill-Tano

CHARGABLE EQUIPMENT USED
Nuclear Density Gauge

GEOTECH INSPECTION REPORT

- No frost present in morning. Placed stabilization fabric between Casino column line CF to CD and C12.5 to C14. Placed 8-inches of ¾-inch clean crushed stone as working surface for upper level construction. Area will be proofrolled in spring prior to slab-on-grade approval.
- No frost present in morning. Placed one 12-inch thick loose lifts of natural onsite material, from racetrack sediment basin C excavation, from approximate elevation $\pm 1463'$ to $\pm 1464'$, inside of Ballroom footprint on the north side of second tier tennis court on the east side of the haul road in the center of the tennis court and compacted with a single smooth drum 10-ton roller. Nuclear density testing, visual observation and probing of the subgrade verified the lifts were placed in accordance with design specifications.
- No frost present in morning. Placed two 12-inch thick loose lifts of R.C.A. material from 0' to 20' west of hotel footprint in area of indoor pool structure, from column lines T-3.5 to T-11, from approximate elevation $\pm 1482'$ to $\pm 1484'$ and compacted with a single smooth drum 10-ton roller. Nuclear density testing, visual observation and probing of the subgrade verified the lifts were placed in accordance with design specifications.
- No frost present in morning. Contractor removed 6-inches of mud and brought it to the temporary topsoil stockpile. Placed two 12-inch thick loose lifts of natural onsite material, from racetrack sediment basin C excavation, from approximate elevation $\pm 1482'$ to $\pm 1484'$, approximately 0' to 20' east of hotel footprint in area for roadway and compacted with a single smooth drum 10-ton roller. Nuclear density testing, visual observation and probing of the subgrade verified the lifts were placed in accordance with design specifications.

- Excavated 3 to 4 feet below existing subgrade to a firm previously backfilled and approved footing subgrade for Casino Footings C387, C392, C977 and compacted with double-drum sheepsfoot roller. Excavated 3 to 4 feet below existing subgrade to bedrock for Casino Footings C997/C992, C968/C993 and C980/C994. It was discovered later in the day that due to contractors working on footing subgrade and water the subgrade for C387 and C392 deteriorated. The footings were over excavated approximately 1 foot and ¾-inch clean crushed stone was placed. Visual observation and probing of the footing subgrades verified a footing subgrade of 4.5 TSF bearing capacity. All footings were poured today except for C387 and C392. No frost is expected and footing subgrade will be checked again tomorrow.



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DATE: 12/18/2008	JOB NO. 5485
PROJECT: Concord	
LOCATION: Lake Kiamesha, NY	
CONTRACTOR: Fuller	
OWNER: Cappelli	
WEATHER: Sunny	TEMP: Mid 30's
PRESENT AT SITE	
Oscar Nordstrom-Fuller	
Eric Beck-Tano	

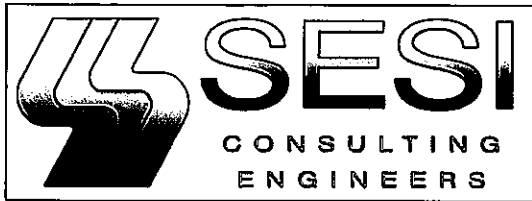
CONTRACTOR, LABOR & EQUIPMENT	
QUANTITY	DESCRIPTION
1	Crew for Backfill-Tano

CHARGABLE EQUIPMENT USED
Nuclear Density Gauge

GEOTECH INSPECTION REPORT

- Placed 12-inches $\frac{3}{4}$ -inch clean crushed stone, from column lines T-3.5 to T-11, from 0' to 20' west of hotel footprint in area of indoor pool structure, to approximate elevation $\pm 1489'$. Now 24-inches of $\frac{3}{4}$ -inch clean crushed stone in this area. Deo Persaud performed inspection.
- Two to three inches of frost present in morning. Contractor used dozer to track and break the frost and compacted with a single smooth drum 10-ton roller. Placed two 12-inch thick loose lifts of natural onsite material, from racetrack sediment basin C excavation, from 20' to 40' west of hotel footprint in area of indoor pool structure, from column lines T-3.5 to T-11, from 0' to 40' west of hotel footprint, from approximate elevation $\pm 1486'$ to $\pm 1488'$, from column line T-2 to T-3.5, from approximate elevation $\pm 1481'$ to $\pm 1483'$, from column line T-1 to T-2, from approximate elevation $\pm 1476'$ to $\pm 1478'$. Nuclear density testing, visual observation and probing of the subgrade verified the lifts were placed in accordance with design specifications. Deo Persaud performed inspection.
- Two to three inches of frost present in morning. Contractor removed 2 feet of material from column line C11 to C13 from CW to CZ, that was deteriorated due to high traffic from concrete pour yesterday. Placed one 12-inch thick loose lift of 12-inch minus R.C.A. and one 12-inch thick loose lift of 3-inch minus R.C.A., from approximate elevation $\pm 1488'$ to $\pm 1490'$ and compacted with a single smooth drum 10-ton roller. No density testing performed due to high gravel content. Visual observation and probing of the subgrade verified the lifts were placed in accordance with design specifications. Deo Persaud performed inspection.

- Footing subgrade for Casino Footing C377, C965, and C975, column line C14 from CW to CZ as shown above was reapproved to elevation $\pm 1490'$. Footings were covered with blankets to protect the subgrade from frost overnight. Excavated 2 to 3 feet below existing subgrade to a firm previously backfilled and approved footing subgrade for Casino Footings C988, C989 and C990 and to bedrock for Casino Footing C991/C995. Footings C988 and C990 were compacted with a double drum sheepsfoot roller and C989 was over-excavated 12-inches due to moist material and 3-inch minus R.C.A. was placed and compacted with a double drum sheepsfoot roller. Casino Footing C387 previously rejected was removed $\frac{3}{4}$ -inch stone was added to areas where stone was previously removed. Visual observation and probing of the footing subgrades verified a footing subgrade of 4.5 TSF bearing capacity. All footings were poured today. All pours were covered with blankets to protect the subgrade and footings.
- MW-BSEA-5 was removed today in area on north of second tier tennis court. The monitoring well was backfilled with bentonite pellets and the pvc pipe and steel casing removed.



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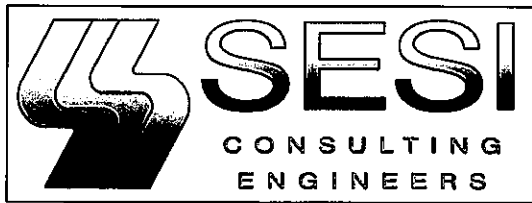
DATE: 12/10/2008	JOB NO. 5485
PROJECT: Concord	
LOCATION: Lake Kiamesha, NY	
CONTRACTOR: Fuller	
OWNER: Cappelli	
WEATHER: Rain	TEMP: Low 50's
PRESENT AT SITE	
Oscar Nordstrom-Fuller	
Eric Beck-Tano	

CONTRACTOR, LABOR & EQUIPMENT	
QUANTITY	DESCRIPTION
1	Crew for Backfill-Tano

CHARGABLE EQUIPMENT USED

GEOTECH INSPECTION REPORT

- No frost present in morning. Contractor removed 8-inches of crushed bedrock material and stockpiled for future use. Placed stabilization fabric between Casino column line CF to CB and C16 to C17. Placed 8-inches of ¾-inch clean crushed stone as working surface for upper level construction. Area will be proofrolled in spring prior to slab-on-grade approval. Deo Persaud performed inspection.



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CONTRACTOR, LABOR & EQUIPMENT	
QUANTITY	DESCRIPTION
1	Crew for Backfill-Tano

DATE: 12/19/2008	JOB NO. 5485
PROJECT: Concord	
LOCATION: Lake Kiamesha, NY	
CONTRACTOR: Fuller	
OWNER: Cappelli	
WEATHER: P.Cloudy/Snow	TEMP: Mid 20's
PRESENT AT SITE	
Oscar Nordstrom-Fuller	
Eric Beck-Tano	

CHARGABLE EQUIPMENT USED
Nuclear Density Gauge

GEOTECH INSPECTION REPORT

- It was determined that work would be stopping for two weeks due to weather and holidays. Four to five inches of frost present in morning. Contractor did not remove frost due to pending work stoppage informed Sesi representative that any unsatisfactory areas would be addressed later. Placed four 12-inch thick loose lifts of natural onsite material, from racetrack sediment basin C excavation around Casino Footings C377, C965, and C975 which were covered overnight and had no frost. Placed two 12-inch thick loose lifts of natural onsite material, from racetrack sediment basin C excavation from approximate elevation $\pm 1492'$ to $\pm 1494'$ from column lines C14 to C15 from CW to CZ, and one 12-inch thick loose from elevation $\pm 1492'$ to $\pm 1494'$ C15 to C19 from CW to CZ. Nuclear density testing, visual observation and probing of the subgrade verified the lifts were placed in accordance with design specifications. Due to frosty slab-on-grade subgrade that will need to be addressed prior to pour.



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DATE: 12/16/2008	JOB NO. 5485
PROJECT: Concord	
LOCATION: Lake Kiamesha, NY	
CONTRACTOR: Fuller	
OWNER: Cappelli	
WEATHER: P. Cloudy	TEMP: Mid 40's
PRESENT AT SITE	
Oscar Nordstrom-Fuller	
Eric Beck-Tano	

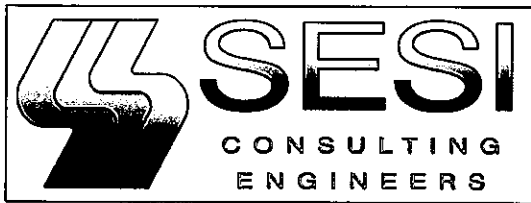
CONTRACTOR, LABOR & EQUIPMENT	
QUANTITY	DESCRIPTION
1	Crew for Backfill-Tano

CHARGABLE EQUIPMENT USED
Nuclear Density Gauge

GEOTECH INSPECTION REPORT

- No frost present in morning. Placed stabilization fabric between Casino column line CF to CD and C11 to C112.5. Placed 8-inches of ¾-inch clean crushed stone as working surface for upper level construction. Area will be proofrolled in spring prior to slab-on-grade approval.
- No frost present in morning. Placed two 12-inch thick loose lifts of natural onsite material, from racetrack sediment basin C excavation, from approximate elevation $\pm 1464'$ to $\pm 1466'$, inside of Ballroom footprint on the north side of second tier tennis court on the east side of the haul road in the center of the tennis court and compacted with a single smooth drum 10-ton roller. Nuclear density testing, visual observation and probing of the subgrade verified the lifts were placed in accordance with design specifications.
- No frost present in morning. Placed three 12-inch thick loose lifts of natural onsite material, from racetrack sediment basin C excavation, from approximate elevation $\pm 1476'$ to $\pm 1479'$, around the first tier of the hotel on the southern end backfilling the foundation and protecting it from frost. The lifts were compacted with a single smooth drum 10-ton roller. Nuclear density testing, visual observation and probing of the subgrade verified the lifts were placed in accordance with design specifications.
- Excavated 3 to 4 feet below existing subgrade to a firm previously backfilled and approved footing subgrade for Casino Footings C386, C966, C976 and compacted with double-drum sheepsfoot roller. Footing subgrade for Casino Footings C387 and C392 was approved yesterday and was approved again today. Footing C217/C941 was removed due to design changes and subgrade remained approved. Visual observation and probing of the footing subgrades verified a footing subgrade of 4.5 TSF bearing capacity. All footings were poured today. No frost is expected and footing subgrade will be checked again tomorrow.

- No frost present in morning. Contractor removed 12-inches of mud and brought it to the temporary topsoil stockpile. Placed one 12-inch thick loose lifts of 12-inch minus R.C.A. material, from approximate elevation $\pm 1481'$ to $\pm 1482'$, approximately 60' to 100' east of hotel footprint in area for roadway and compacted with a single smooth drum 10-ton roller. No density testing performed on crushed bedrock due to size of material. Visual observation and probing of the subgrade verified the lifts were placed in accordance with design specifications.



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DATE: 12/3/2008	JOB NO. 5485
PROJECT: Concord	
LOCATION: Lake Kiamesha, NY	
CONTRACTOR: Fuller	
OWNER: Cappelli	
WEATHER: Sunny	TEMP: Mid 30's
PRESENT AT SITE	
Oscar Nordstrom-Fuller	
Eric Beck-Tano	

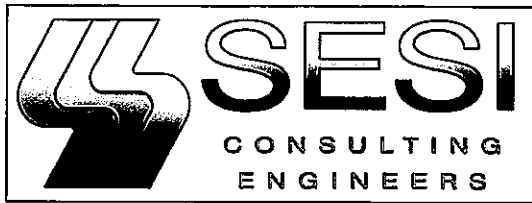
CONTRACTOR, LABOR & EQUIPMENT	
QUANTITY	DESCRIPTION
3	Crew for Backfill-Tano

CHARGABLE EQUIPMENT USED

GEOTECH INSPECTION REPORT

- Four inches of frost present in morning. Contractor removed frost and placed in frost stockpile on tennis court prior to placement of fill. Placed one 12-inch thick loose lift of 3-inch minus crushed bedrock, from parking garage #1 excavation, from approximate elevation $\pm 1484'$ to $\pm 1485'$ along entire hotel footprint width including 20 feet outside of footprint from column line T-5 to T-6 and compacted with a single smooth drum 10-ton roller. No density testing performed on crushed bedrock due to amount of gravel in material. Visual observation and probing of the subgrade verified the lifts were placed in accordance with design specifications.
- Area for hotel elevation pit from column lines TA1.5 to TA2 and column line T-9 to T-12 was excavated approximately 3 feet from slab-on-grade subgrade. Area from the west foundation wall to column line TA1.5 and column line T-9 to T-12 was excavated approximately 8 feet to a firm natural soil subgrade to approximate elevation $\pm 1473'$ because area was previously backfilled with soil from former hotel pad. Area needs to be filled to achieve elevator pit subgrade elevation.
- Four inches of frost present in morning. Contractor removed frost and placed in frost stockpile on tennis court prior to placement of fill. Placed one 12-inch thick loose lifts of 3-inch minus crushed bedrock, from parking garage #1 excavation, from 20' to 60' west of hotel footprint in area of indoor pool structure from approximate elevation $\pm 1478'$ to $\pm 1479'$ and compacted with a single smooth drum 10-ton roller. No density testing performed on crushed bedrock due to amount of gravel in material. Visual observation and probing of the subgrade verified the lifts were placed in accordance with design specifications.

- Four inches of frost present in morning. Contractor removed frost and placed in frost stockpile on tennis court prior to placement of fill. Placed three 12-inch thick loose lifts of 12-inch minus crushed concrete, from 60' to 90' west of hotel footprint in area of indoor pool structure from approximate elevation $\pm 1463'$ to $\pm 1466'$ and compacted with a single smooth drum 10-ton roller. Area in which manhole was removed was filled with approximately four 12-inch thick loose lifts of 12-inch minus crushed concrete and heeled with bucket. No density testing performed on crushed bedrock due to size of material. Visual observation and probing of the subgrade verified the lifts were placed in accordance with design specifications.
- Six inches of frost present in natural onsite material this morning. Contractor removed frost and placed in frost stockpile on tennis court prior to placement of fill. Area in outside of Casino footprint to the north of second tier tennis court was previously cut 1 foot to a firm natural soil subgrade to elevation $\pm 1460'$ and proofrolled with a single smooth drum 10-ton roller. Placed three 12-inch thick loose lifts of natural onsite material (from stripped material and from racetrack excavation 20 to 100 feet south of OU-1B) to approximate elevation $\pm 1460'$ to $\pm 1463'$ and compacted with a single smooth drum 10-ton roller. Visual observation and probing of the subgrade verified the lifts were placed in accordance with design specifications.



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DATE: 12/5/2008	JOB NO. 5485
PROJECT: Concord	
LOCATION: Lake Kiamesha, NY	
CONTRACTOR: Fuller	
OWNER: Cappelli	
WEATHER: P. Cloudy	TEMP: High 20's
PRESENT AT SITE	
Oscar Nordstrom-Fuller	
Eric Beck-Tano	

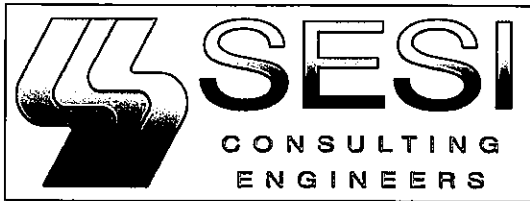
CONTRACTOR, LABOR & EQUIPMENT	
QUANTITY	DESCRIPTION
3	Crew for Backfill-Tano

CHARGABLE EQUIPMENT USED
Nuclear Density Gauge

GEOTECH INSPECTION REPORT

- No frost present this morning. Rat slab poured in hotel elevator pit area. 2 to 3-inches of concrete poured. 18-inches of ¾-inch clean crushed stone placed on hotel footprint from T-3.5 to T-12 except for elevator pit area. At least 22-inches of ¾-inch clean crushed stone will be placed and a 2 to 3 inch mud mat of concrete will be poured to prevent frost of the subgrade. Deo Persaud performed inspection.
- Frost stockpile on second tier tennis court was moved to temporary topsoil stockpile.
- No frost present this morning. Placed three 12-inch thick loose lifts of natural onsite material, from racetrack excavation 20 to 100 feet south of OU-1B, from 20' to 60' west of hotel footprint in area of indoor pool structure, from column lines T-7 to T-12, from approximate elevation $\pm 1479'$ to $\pm 1482'$ and compacted with a single smooth drum 10-ton roller. Density testing, visual observation and probing of the subgrade verified the lifts were placed in accordance with design specifications. Deo Persaud performed inspection.
- No frost present this morning. Placed three 12-inch thick loose lifts of natural onsite material, from racetrack excavation 20 to 100 feet south of OU-1B, from approximate elevation $\pm 1463'$ to $\pm 1466'$, outside of Casino footprint to the north of second tier tennis court and compacted with a single smooth drum 10-ton roller. Nuclear density testing, visual observation and probing of the subgrade verified the lifts were placed in accordance with design specifications. Deo Persaud performed inspection.

- Contractor removed 3 to 4 feet of wet unsuitable material to approximately 10 feet below subgrade and brought it to the temporary topsoil stockpile in area of Casino footings C792/C203 to C384 to C984 to C795/C981. Placed four 12-inch thick loose lifts of natural onsite material, from racetrack excavation 20 to 100 feet south of OU-1B, from approximate elevation $\pm 1484'$ to $\pm 1488'$. Nuclear density testing, visual observation and probing of the subgrade verified the lifts were placed in accordance with design specifications. Deo Persaud performed inspection.



12A MAPLE AVENUE
PINE BROOK, NJ 07058
(973) 808-9050
www.SESI.org

DATE: 12/9/2008	JOB NO. 5485
PROJECT: Concord	
LOCATION: Lake Kiamesha, NY	
CONTRACTOR: Fuller	
OWNER: Cappelli	
WEATHER: P. Cloudy	TEMP: High 40's
PRESENT AT SITE	
Oscar Nordstrom-Fuller	
Eric Beck-Tano	

CONTRACTOR, LABOR & EQUIPMENT	
QUANTITY	DESCRIPTION
3	Crew for Backfill-Tano

CHARGABLE EQUIPMENT USED
Nuclear Density Gauge

GEOTECH INSPECTION REPORT

- Six to twelve inches of frost present in morning. Contractor removed frost and placed in frost stockpile on to the north of second tier tennis court prior to placement of fill. Placed five 12-inch thick loose lifts of natural onsite material, from racetrack excavation 20 to 100 feet south of OU-1B, from approximate elevation $\pm 1466'$ to $\pm 1471'$, inside of Ballroom footprint on the north side of second tier tennis court and approximately 100 feet north of the Ballroom and compacted with a single smooth drum 10-ton roller. Nuclear density testing, visual observation and probing of the subgrade verified the lifts were placed in accordance with design specifications. Deo Persaud performed inspection.
- Six to twelve inches of frost present in morning. Contractor removed frost and placed in frost stockpile on to the north of second tier tennis court prior to placement of fill. Placed three 12-inch thick loose lifts of natural onsite material in area of Casino footings C792/C203 to C384 to C984 to C795/C981, from racetrack excavation 20 to 100 feet south of OU-1B, from approximate elevation $\pm 1487'$ to $\pm 1490'$. Nuclear density testing, visual observation and probing of the subgrade verified the lifts were placed in accordance with design specifications. Deo Persaud performed inspection.
- Twelve inches of frost present in morning, eight inches removed. Contractor removed frost and took it to the temporary topsoil stockpile. Placed stabilization fabric between Casino column line CD to CB and C17 to C19. Placed 8-inches of $\frac{3}{4}$ -inch clean crushed stone as working surface for upper level construction. Area will be proofrolled in spring prior to slab-on-grade approval.

- Excavated 3 to 4 feet below existing subgrade to a firm previously backfilled and approved footing subgrade for Casino Footings C388, C397, C978, C996, C967, and C979 and compacted with double-drum sheepsfoot roller. Visual observation and probing of the footing subgrades verified a footing subgrade of 4.5 TSF bearing capacity. All footings were poured today. Deo Persaud performed inspection.
- Placed one 12-inch thick loose lifts of natural onsite material from racetrack excavation 20 to 100 feet south of OU-1B on area of hotel footing on northeast side of footprint to avoid frost getting under the footing. Told this was a temporary placement and was not compacted.



12A MAPLE AVENUE
PINE BROOK, NJ 07058
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www.SESI.org

DATE: 12/17/2008	JOB NO. 5485
PROJECT: Concord	
LOCATION: Lake Kiamesha, NY	
CONTRACTOR: Fuller	
OWNER: Cappelli	
WEATHER: P. Cloudy	TEMP: Mid 30's
PRESENT AT SITE	
Oscar Nordstrom-Fuller	
Eric Beck-Tano	

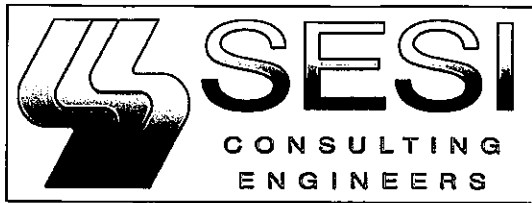
CONTRACTOR, LABOR & EQUIPMENT	
QUANTITY	DESCRIPTION
1	Crew for Backfill-Tano

CHARGABLE EQUIPMENT USED
Nuclear Density Gauge

GEOTECH INSPECTION REPORT

- No frost present this morning. Placed one 12-inch thick loose lift of 12-minus R.C.A. material, from approximate elevation $\pm 1463'$ to $\pm 1464'$. Placed three 12-inch thick loose lifts of natural onsite material, from racetrack sediment basin C excavation, from approximate elevation $\pm 1464'$ to $\pm 1467'$, inside of Ballroom footprint on the far north side of second tier tennis court on the east side of the haul road in the center of the tennis court and compacted with a single smooth drum 10-ton roller. Nuclear density testing, visual observation and probing of the subgrade verified the lifts were placed in accordance with design specifications. Deo Persaud performed inspection.
- No frost present this morning. Contractor removed 2 feet of extremely moist material and brought it to the temporary topsoil stockpile. Placed two 12-inch thick loose lifts of natural onsite material, from racetrack sediment basin C excavation, from column line C12 to C14 from CW to CZ, from approximate elevation $\pm 1488'$ to $\pm 1490'$ and compacted with a single smooth drum 10-ton roller. Visual observation and probing of the subgrade verified the lifts were placed in accordance with design specifications. Deo Persaud performed inspection.
- Footing subgrade for Casino Footing C377, C965, and C975, column line C14 from CW to CZ as shown above was approved to elevation $\pm 1490'$. Contractor placed 8 to 12-inches of $\frac{3}{4}$ -inch clean crushed stone for C377 and C965, and 8 to 12-inches of R.C.A. material for C975 and compacted with double-drum sheepsfoot roller. Visual observation and probing of the footing subgrades verified a footing subgrade of 4.5 TSF bearing capacity. Footings were not poured today and footing subgrade will be checked again tomorrow. The footings were covered with blankets to protect the subgrade from frost. Deo Persaud performed inspection.

- Casino Footings from column line C15 to C19 and CW to CZ were backfilled with 3 or 4 12-inch thick loose lifts of R.C.A. material depending upon depth to footing bottom. Loose moist material was removed to a firm material prior to backfill. MHE rejected footings C387, C978 and C979 due to presence of water and backfill procedures. Michael St. Pierre (MSP) inspected all 3 footings and found them to be in good condition. C978 and C979 had not had water, ice and muck removed from around footing and C387 had become undermined in a small area on 1 side of the footing. MSP discussed the conditions with Michael Lamorueax(ML) and ML agreed with MSP that footings C978 and C979 could remain provided that Mark Skellett inspect the mucking out of the footings. ML did not agree with MSP on footing C387 and required it to be removed. MSP was present during the mucking out of footing C978 and found the footing subgrade to be in a firm condition. The majority of the muck was removed and approximately 6 inches of $\frac{3}{4}$ inch clean crushed stone was placed in the bottom of the excavation. Footing backfill with RCA continued from there. Visual observation and probing of the subgrade verified the lifts were placed in accordance with design specifications.
- No frost present in morning. Placed three 12-inch thick loose lifts of R.C.A. material from 0' to 20' west of hotel footprint in area of indoor pool structure, from column lines T-3.5 to T-11, from approximate elevation $\pm 1484'$ to $\pm 1487'$ and compacted with a single smooth drum 10-ton roller. Placed 12-inches of $\frac{3}{4}$ -inch clean crushed stone. Nuclear density testing, visual observation and probing of the subgrade verified the lifts were placed in accordance with design specifications. Deo Persaud performed inspection.
- Placed 24-inch thick loose lifts of 12-inch minus R.C.A. material, from approximate elevation $\pm 1463'$ to $\pm 1465'$, in future hotel footprint in area on east side of second tier tennis court area. Area was not compacted as laborers were taking out any loose metal material. Deo Persaud performed inspection.



12A MAPLE AVENUE
PINE BROOK, NJ 07058
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DATE: 12/11/2008	JOB NO. 5485
PROJECT: Concord	
LOCATION: Lake Kiamesha, NY	
CONTRACTOR: Fuller	
OWNER: Cappelli	
WEATHER: Rain	TEMP: Mid 30's
PRESENT AT SITE	
Oscar Nordstrom-Fuller	
Eric Beck-Tano	

CONTRACTOR, LABOR & EQUIPMENT	
QUANTITY	DESCRIPTION
1	Crew for Backfill-Tano

CHARGABLE EQUIPMENT USED

GEOTECH INSPECTION REPORT

- No frost present in morning. Contractor removed 8-inches of crushed bedrock material and stockpiled for future use. Placed stabilization fabric between Casino column line CF to CB and C15 to C16. Placed 8-inches of ¾-inch clean crushed stone as working surface for upper level construction. Area will be proofrolled in spring prior to slab-on-grade approval. Deo Persaud performed inspection.